



Engineering the Future

HERVEY BAY CITY COUNCIL

URANGAN DRAINAGE STUDY - ELIZABETH ST PIER STREET PILOT STREET GUARD STREET

ULTIMATE DEVELOPMENT SCENARIO

Revised Final Report

**JULY 2003
JOB NO. 2919/31**

URANGAN DRAINAGE STUDY REVISED FINAL REPORT

TABLE OF CONTENTS

EXECUTIVE SUMMARY	1
1.0 INTRODUCTION.....	3
1.1 <i>Overview</i>	3
1.2 <i>Modelling Package</i>	3
2.0 HYDROLOGY	5
2.1 <i>Overview</i>	5
2.2 <i>Selection of Hydrograph Derivation Method</i>	5
2.2.1 Available Options.....	5
2.2.2 Laurenson Non Linear Method	6
2.2.3 Time Area Method	8
2.2.4 Rational Method/ User Defined Inflows.....	8
2.3 <i>Derivation of Hydrologic Parameters</i>	11
2.3.1 Time of Concentration	11
2.3.2 Rainfall Intensity	12
2.3.3 Fraction Impervious Values and Runoff Coefficients.....	12
3.0 HYDRAULIC MODELLING.....	14
3.1 <i>General</i>	14
3.2 <i>Surface Network</i>	15
3.3 <i>Underground Network</i>	17
3.3.1 General.....	17
3.3.2 Manhole Losses	17
3.3.3 Gross Pollutant Traps.....	18
3.4 <i>Stormwater Inlet Pits</i>	19
3.4.1 General.....	19
3.4.2 Sag Inlets	20
3.4.3 On Grade Inlets	21
3.5 <i>Tailwater Levels</i>	21
3.6 <i>Model Parameters</i>	21

4.0	MODELLING RESULTS	23
4.1	<i>General.....</i>	23
4.2	<i>Minimum Floor Level</i>	26
4.3	<i>Elizabeth Street Catchment (A).....</i>	27
4.4	<i>Pier Street Catchment (B).....</i>	28
4.5	<i>Pilot Street Catchment (C).....</i>	30
4.6	<i>Guard Street Catchment (D).....</i>	31
4.7	<i>Estimated Cost of Works</i>	33
5.0	CONCLUSION.....	35
6.0	REFERENCES.....	36

APPENDIX A- Hydrologic Data
 APPENDIX B- Hydraulic Data
 APPENDIX C- Model Results
 APPENDIX D- Estimated Construction Costs

List of Tables

TABLE 1	Design Rainfall Intensities
TABLE 2	Adopted Fraction Impervious Values and Runoff Coefficients
TABLE 3	Inflow Rating Curves- Weir Correction Parameters
TABLE 4	Estimated Cost of Construction

List of Figures

Figure 1-	Locality Plan
Figure 2-	Pilot Street- 20 Year, Pipe Sizes and Fill Level- Ultimate Development
Figure 3-	Pilot Street- 50 Year, Pipe Sizes and Fill Level- Ultimate Development
Figure 4-	Pilot Street- 100 Year, Pipe Sizes and Fill Level- Ultimate Development

EXECUTIVE SUMMARY

The majority of the Urangan Coastal Strip between Elizabeth Street and Pulgul Street is low lying, with ground elevations in some areas less than RL 2.0 m AHD. Large areas have ground levels less than RL 4.0 m AHD. In comparison, the level of the Highest Astronomic Tide is RL 2.1 m AHD. Due to existing low ground levels and the fact that a natural ridge exists along the foreshore, properties within the Urangan catchment are regularly inundated during storm events. The problem is presently exacerbated by an undersized stormwater drainage system.

It has been determined that the cost of constructing relief drainage works to conventional design standards would be prohibitively expensive. The cost of the drainage works, which would need to be apportioned between landowners within the catchment, would act as a significant deterrent to redevelopment in the future.

However, a subsequent Concept Design Phase Investigation identified an innovative and cost effective solution for the region. The solution comprises the following elements:

- Upgrading the main drainage lines in the catchment to convey the minor event flow (2 year or 10 year depending on the requirements for each subcatchment),
- Allowing water to pond in road reserves, particularly during major storm events such as the 20, 50, and 100 year event, and
- Setting fill levels or habitable floor levels which take into account the ponding of water in road reserves.

Allowing water to pond within the catchment during significant flood events minimises the pipe drainage requirements for the catchment. Providing main drain lines capable of conveying the minor event flow minimises the extent of ponding or nuisance flooding during minor storm events which, by definition, occur more frequently than relatively rare major event storms. In addition, the upgraded stormwater drains provide sufficient capacity to ensure that ponded water depths, while a key element of the drainage design, do not become excessive and pose a significant risk to public safety.

Due to the complex nature of the proposed works, detailed modelling of the works was necessary. The detailed modelling, the results of which are presented in this report, confirmed the viability of the drainage scheme and allowed the refinement of the pipe sizes developed during the Concept Design Phase Investigation.

The overall cost of the drainage scheme, including the construction of gross pollutant traps at the downstream end of each of the four main drainage lines, is M\$3.62.

It is important to note the following with regard to the derived drainage scheme and this report:

- By taking advantage of the storage available in the road reserve, the drainage scheme does not rigorously comply with the requirements of the *Queensland Urban Drainage Manual* and the *Hervey Bay City Council Development Manual* for new developments.

However, the proposed works are relief drainage works, and the *Queensland Urban Drainage Manual* permits the relaxation of design standards for relief works in order that cost effective solutions can be implemented. It is estimated that drainage works to achieve full compatibility to the requirements of the *Queensland Urban Drainage Manual* for the Urangan Coastal Strip would cost well in excess of M\$10 (Gutteridge Haskins and Davey 1996, Drainage Strategy No. 41). Such a cost is likely to compromise the viability of future development in the catchment. In comparison, the cost of the relief drainage works proposed for the coastal strip are considered to be within reason.

- The main report presents calculated flood levels for the 20, 50, and 100 year major storm events. The choice of immunity level for buildings does not affect the cost of relief drainage works as the pipe sizes adopted for the catchment are largely governed by minor (2 and 10 year) event storms. For example, the adoption of a 50 year immunity level only allows a reduction in the minimum habitable floor level or fill level compared to that required for the 100 year event.
- The modelling has considered the ultimate extent of development within the catchment and has assumed that private properties have been filled to above the major event storm level. Council can decide whether to impose a fill requirement or to simply accept a minimum floor level for each property as development or redevelopment applications are assessed.
- The original final report was issued in February 2000 and was revised in 2003 to reflect a reduction in catchment area afforded by the development of a site at the corner of Elizabeth Street and Dayman Street and the redirection of runoff from the site to the adjacent Lowlands catchment.

In summary, detailed modelling has confirmed the viability of a cost effective relief drainage scheme for the Urangan Coastal Strip between Elizabeth Street and Pulgul Street. Required pipe sizes and development levels are shown on Figures 2, 3, and 4 for the 20, 50, and 100 year events respectively.

1.0 INTRODUCTION

1.1 Overview

The majority of the Urangan Coastal Strip between Elizabeth Street and Pulgul Street is relatively low lying, with ground elevations in some areas less than RL 2.0 m AHD. Large areas have ground levels less than RL 4.0 m AHD. In comparison, the level of Highest Astronomic Tide is RL 2.1 m AHD (Hervey Bay City Council 1997, p 16-14). Due to existing ground levels and the fact that a natural ridge exists along the foreshore, properties within the catchment are regularly inundated during storm events. The problem is presently exacerbated by an undersized stormwater drainage system.

Cardno and Davies was commissioned by Council on 19 October 1998 to conduct a stormwater drainage investigation to determine a cost effective drainage solution which would allow future development within the catchment.

The resultant Concept Design Phase investigation produced a solution which involved a combination of pipe drainage works to convey minor event flows, road regrading to redefine catchment boundaries, and the filling of properties to allow the road reserve to store water during major events without inundating residences.

Due to the reliance of the solution upon storage within the catchment, it was necessary to confirm the viability of the solution using a detailed hydraulic model. Cardno & Davies was commissioned by Hervey Bay City Council in February 1999 to complete the required modelling of the ultimate development scenario.

A final report was prepared in relation to the modelling in February 2000. The report has been revised in 2003 to reflect a reduction in catchment area afforded by the development of a site at the corner of Elizabeth Street and Dayman Street. As a result of the redirection of runoff from the site to the adjacent Lowlands catchment, the size of relief drainage works in Elizabeth Street and Pier Street has been reduced.

Section 2 details the derivation of hydrologic inputs for the model while Section 3 describes the formulation of the hydraulic model. Section 4 presents the results of the analysis and recommended drainage works for each part of the catchment.

1.2 Modelling Package

The catchment was analysed using Version 8.5 of the XP Software XP-STORM package (XP Software 2002). XP-STORM replaces the former XP-UDD and EXTRAN hydraulic models.

XP-STORM comprises Runoff and Hydraulic (UDD/EXTRAN) modes to allow the simulation and hydraulic routing of rainfall. The Hydraulic (UDD/EXTRAN) mode of the package is an hydraulic flow routing model for both open channel and closed conduits in dendritic and looped networks. The Hydraulic mode receives hydrograph input at specific nodal locations directly from the output of the Runoff mode of the program. The model uses a combination of implicit and explicit finite difference formulations to dynamically route runoff throughout the modelled drainage system.

The XP-STORM model used for the analysis had the following capabilities:

- Maximum Nodes: 500
- Maximum Links: 500
- Maximum Texts: 40
- Maximum Pictures: 10
- Maximum Cards: 150,000

License options for the model included:

- Profile Plotting
- Pumps/ Orifices
- Full Equations (rather than simple kinematic wave approach for conduits)
- All conduit shapes

A major feature of the program is that data can be input to XP-STORM models entirely via ASCII text files. Extensive use of this facility was made during the investigation to improve the quality of the model and to facilitate data entry. Data was input to EXCEL spreadsheets, output in a suitable text file format and then imported to the XP-STORM model.

2.0 HYDROLOGY

2.1 Overview

To correctly represent the runoff characteristics of the study area, the 68.34 hectare catchment was divided into a total of 72 subcatchments with an average area of 0.95 hectares, as follows:

- Elizabeth Street (A) 25 subcatchments, total area 20.84 hectares
- Pier Street (B) 14 subcatchments, total area 14.06 hectares
- Pilot Street (C) 12 subcatchments, total area 10.49 hectares
- Guard Street (D) 21 subcatchments, total area 22.96 hectares

The adopted sub catchment layout is shown on **Figure A1- Catchment Definition of Appendix A- Hydrologic Data**. Data pertinent to each subcatchment is presented in **Table A1- Catchment Areas, Weighted Runoff Coefficients and Times of Concentration** in Appendix A.

The division of the catchment was based upon internal ridge lines, existing inlets to the stormwater drainage system, and proposed stormwater drainage inlets and road regrading works. Each subcatchment area was assigned to a node in the XP-STORM model.

Hydrographs were derived for each subcatchment for storm durations ranging from fifteen minutes to two hours for events with recurrence intervals of 2, 10, 20, 50, and 100 years. The standard storm durations presented in *Australian Rainfall and Runoff* (Institution of Engineers 1987) were considered applicable to the catchment area. Consequently, storm durations of 15 minutes, 20 minutes, 25 minutes, 30 minutes, 45 minutes, 1 hour, 90 minutes, and 2 hours were considered.

In order to facilitate future modelling of intermediate development scenarios and to provide maximum consistency with the *Queensland Urban Drainage Manual* (QUDM) (Neville Jones & Associates et al 1992) and the *Hervey Bay City Council Development Manual* (Hervey Bay City Council 1997), simplistic runoff hydrographs were derived based on the Rational Method.

2.2 Selection of Hydrograph Derivation Method

2.2.1 Available Options

The Runoff mode of XP-STORM provides the following methods of hydrograph generation (XP-Software 1998, pp 164-174):

- SWMM Runoff Non-Linear Reservoir Method,
- Kinematic Wave Method,
- Laurenson Non-Linear Method (RAFTS),
- SCS Unit Hydrograph Method,
- Other Unit Hydrograph Methods including:
 - Nash,
 - Santa Barbara Urban Hydrograph,
 - Snyder,
 - Snyder (Alameda Modified),
 - Time Area (ILSAX), and
 - Rational Formula,
- User Defined Hydrograph.

The following sections describe the strengths and weaknesses of the available hydrograph derivation techniques and provide the rationale for the adoption of a Rational Method based approach to the derivation of hydrographs and the use of User Defined hydrographs for the importation of the hydrographs to the hydraulic model.

2.2.2 Laurenson Non Linear Method

The Laurenson non-linear method (as applied in the XP Software RAFTS computer model) uses Laurenson's 1964 non-linear runoff routing procedure with modifications by Aitken to allow greater flexibility to model both rural and urban catchments (XP Software 1998, p 167).

Routing of each local subarea is performed using either Muskingum Method or more simplistic lagging by a specified time period, with the storage of each subcatchment being obtained via the following equation:

$$s = B q^{(n+1)}$$

Where: s = volume of storage (hrs.m³/s)
 q = instantaneous rate of runoff (m³/s)
 n = non-linearity exponent (default value: -0.285)
 B = storage delay time coefficient

To provide a suitable value of the B parameter for Australian conditions, Aitken derived the following relationship from on a study of six urban Australian catchments (Aitken 1975, as quoted in XP Software 1998, p 167):

$$B = 0.285 A^{0.52} (1+U)^{-1.97} S_c^{-0.50}$$

Where: A = subcatchment area (km²)
 U = urbanisation factor
 S_c = main drainage slope of catchment (%)

The Urbanisation factor is calculated from the fraction impervious value of a catchment and varies between zero for a completely pervious catchment, to unity for a catchment with fifty percent impervious area, and to two for a completely impervious catchment area (XP Software 1998, p 168). Subareas within each catchment can be treated as a single area (with an appropriate fraction imperviousness) or as two areas (pervious and impervious) for which individual hydrographs are derived and then combined. Due to the dependence of the B parameter upon the urbanisation factor, the choice of single area or dual area definition for each subarea impacts significantly upon the magnitude of the B parameter and therefore the peak flow calculated for the catchment.

The B parameter is also modified by a multiplication factor (PERN) to account for differences in subcatchment roughness. To differentiate between local catchments with the same degree of urbanisation but different roughnesses the Laurenson (RAFTS) method provides an empirical relationship between the average subcatchment Manning's 'n' and a lag multiplication factor (PERN), which varies between 0.5 for completely impervious areas, to 3.0 for forested catchments (XP Software 1998, p 168).

On catchments where calibration data is available, the procedure typically employed is to adjust loss rates, routing characteristics, PERN values, catchment slopes, and the BX parameter (which allows for the multiplication of the B values derived for each subcatchment by a uniform value) until a satisfactory calibration is achieved.

However, on urban catchments such as the one under investigation in this case, there is usually no data available for model calibration. In order to derive peak flows which can be applied with confidence to the hydraulic model, it is common practice to calibrate the peak flow predicted by the Laurenson (RAFTS) model to the peak flow calculated by the Rational Method according to the *Queensland Urban Drainage Manual* (QUDM) (Neville Jones & Associates et al 1992, Section 5).

To achieve the required calibration, rainfall loss rates, subcatchment slopes, and the BX parameter are typically adjusted within reasonable limits until an acceptable agreement is achieved with QUDM with respect to time of concentration and peak flow rates for each recurrence interval under investigation. The peak flows derived via the Laurenson (RAFTS) method need to agree with those derived according to the QUDM Rational Method for each point of inflow to the hydraulic model. For stormwater drainage analysis, there are normally dozens of inflow points for which hydrographs are required.

Similar studies which have used the Laurenson (RAFTS) method have adopted the following proportional rainfall loss rates in order to achieve acceptable calibration across the range of recurrence intervals considered (Cardno & Davies 1998, Appendix A):

• 2 Year Event	Pervious (first) area	0.4
	Impervious (second) area	0.23
▪ 50 Year Event	Pervious (first) area	0.19
	Impervious (second) area	0.0

Consequently, for a typical urban subarea with a fraction impervious value of 0.5, in order to achieve peak flows consistent with the Rational method, proportional loss rates equivalent to about thirty percent and ten percent of the rainfall incident upon the catchment are required for the 2 year and 50 year events respectively.

Given the formulation of the supporting equations and non-linear nature of the Laurenson (RAFTS) method, it is not possible to exactly match the peak flow and time of concentration predicted by the Rational Method for each subarea and the range of recurrence intervals requiring analysis. The quality of the agreement is also potentially affected when intermediate stages of development are considered due to the method of inclusion of the fraction impervious value in the equation for the derivation of the B parameter.

Of particular importance in this case is the likely future development of the catchment. As the catchment is largely developed with individual lots in private ownership, filling or other redevelopment of blocks is unlikely to occur at a single time or with any significant degree of coordination between groups of land holders. Consequently, development will be staged and require the analysis of a number of intermediate development scenarios. For each intermediate scenario, reliable estimates of the peak flow from various parts of the catchment will be required.

Further, as the results of the analysis will be used as part of the development approval process as redevelopment of the catchment proceeds, to minimise the potential for unnecessary disputes between Council and developers, it is important in the long term that the flows calculated for the ultimate and any intermediate development scenario be derived in accordance with a commonly accepted and understood standard such as the *Queensland Urban Drainage Manual*.

Given the above, it was considered that a hydrograph derivation method which provided the best possible consistency with the QUDM Rational Method was required. For this reason, the Laurenson (RAFTS) method for hydrograph derivation was not used in the study in favour of a method which provided direct compatibility with the Rational Method (refer Section 2.2.4).

2.2.3 Time Area Method

The time area method is a unit hydrograph approach which forms the basis of the ILSAX stormwater drainage program (O'Loughlin, 1988). The time area method assumes that each subarea of a catchment will respond in a linear manner to rainfall according to a user defined time of concentration (XP Software 1998, p 173).

Using this model, it is possible to derive hydrographs with peak flow rates which agree exactly with those calculated via the Rational Method. The time of concentration of each subcatchment can be defined according to the *Queensland Urban Drainage Manual* (QUDM) (Neville Jones & Associates et al 1992, pp 5-12 to 5-20). Due to the assumed linear time area relationship, the method produces either triangular (when the storm duration is less than or equal to the time of concentration of the sub area under consideration) or trapezoidal (when the storm duration is greater than the time of concentration of the sub area) shaped hydrographs, with a time to peak (and time to zero following the cessation of rainfall) equivalent to the nominated time of concentration of each subcatchment.

The only drawback with the method is that it is necessary to set separate proportional loss rates for each subarea in order to produce peak flow rate estimates which agree with those of the Rational Method. As each subcatchment differs slightly with respect to fraction impervious and therefore runoff coefficient, it would be necessary to create separate rainfall loss models for each subcatchment. Further, different sets of loss models would be required for each recurrence interval to be investigated in order to allow for the variation in runoff coefficient with recurrence interval.

In the XP-STORM package, rainfall loss rates are defined as individual events and then assigned to each subarea as appropriate. In the case of the study area, a total of 216 separate rainfall loss scenarios would have had to have been defined (three for each subarea) and then assigned to the subareas of the model.

Given this, it was decided not to adopt the Time Area Method in favour of a strictly Rational Method based approach (refer Section 2.2.4).

2.2.4 Rational Method/ User Defined Inflows

The XP-STORM package presently supports the generation of hydrographs by the derivation of unit hydrographs according to the Rational Method (XP Software 1998, p173).

For each subarea of a catchment, the time of concentration and runoff coefficient of the subarea is entered and a hydrograph either triangular (when the storm duration is less than or equal to the time of concentration of the sub area under consideration) or trapezoidal (when the storm duration is greater than the time of concentration of the sub area) in shape produced. The hydrograph should have a time to peak (and time to zero following the cessation of rainfall) equivalent to the nominated time of concentration of the subarea concerned.

The method permits the direct specification of times of concentration and runoff coefficients in accordance with the *Queensland Urban Drainage Manual* (QUDM) (Neville Jones & Associates 1992). Consequently, hydrographs with peak flow rates and hydrograph shapes which are entirely consistent with QUDM can be readily produced for any recurrence interval or intermediate development scenario (refer Section 2.2.2). Given this, a Rational Method based approach was considered the most appropriate and adopted for use in the study.

However, during the investigation it was discovered that the time to peak (i.e. time of concentration) of the hydrographs produced by the method was a third greater than the specified time of concentration. When this software error was reported to XP Software, the company indicated that it was aware of the problem and recommended that, due to the error, the method should not be used. Further, the company indicated that it intended to remove the Rational Method generation technique from future releases of the XP-Storm package (XP Software 1999).

As the consideration of available methods had indicated that a Rational Method based approach was optimal for the investigation (refer Section 2.2.2), a solution to the time of concentration problem could have been achieved by the adjustment of specified time of concentration values to provide hydrographs with the required time to peak. However, as it was likely that the hydrograph derivation technique would not be available at a later date, it was decided that such an approach was not acceptable.

Consequently, in order to provide the desired hydrographs based on the Rational Method, recourse was made to the option available in XP-STORM to directly input "User Defined" hydrographs to the model (XP Software 1998, p 148). It can be noted that this technique differs from the other methods of hydrograph generation in that hydrographs are directly input to the model rather than being generated by the Runoff mode of the XP-STORM package. When the Runoff mode of the package is used, the program calculates hydrographs and then generates an interface file which is read at the start of the Hydraulic or EXTRAN mode run. The Runoff and Hydraulic modes of the program are otherwise entirely separate models.

The use of user defined hydrographs obviated the need to use the Runoff mode of the package. Due to the relatively simplistic nature of the hydrographs input to the model, the hydrographs were able to be produced using a spreadsheet. The spreadsheet was formatted in a manner which allowed the output of text files in a format which could be directly input to the model using the XPX file importation option of XP-STORM (XP Software 2002, pp 110-115).

The runoff hydrograph spreadsheet featured a master worksheet containing the following information for each subarea of the catchment:

- Name of nodes in the model to which inflow was to be attached,
- Time of concentration of the subarea
- Total area of the subcatchment
- Breakdown of area into the various zones present within the catchment (eg Foreshore Residential)
- Calculated runoff coefficients for the 2, 10, and 100 year events weighted according to the composition of the subcatchment.

Underlying worksheets took the information presented in the master worksheet and provided tables of hydrographs in XPX file format. For each recurrence interval considered, worksheets were produced for each storm duration requiring investigation. The only information input to the underlying worksheets was the rainfall intensity and storm duration represented by the worksheet.

The contents of each worksheet were output as text files and then imported to the Hydraulic (EXTRAN) mode XP-STORM model as required. The importation of a data file for each storm duration and recurrence interval under investigation was similar in terms of user effort to specifying the interface file which would have been produced if the Runoff mode of the package had been used to derive hydrographs.

Overall, it was considered that the spreadsheet based approach facilitated analysis of the catchment and will provide a superior method for the consideration of intermediate development scenarios or alternate recurrence interval events in the future. Entry of data to a spreadsheet is straightforward (and therefore less prone to human error) compared to the relatively tortuous requirements for data entry directly to the XP-STORM model itself, particularly for users unfamiliar with the model or for those who only need to use the model on an infrequent basis. Obtaining summary printouts of adopted data for particular runs is also facilitated, thereby contributing to quality assurance for the analysis.

In order to provide an acceptable coverage of the range of storm durations which could potentially cause peak flood levels and flows within the catchment, the standard storm durations presented in *Australian Rainfall and Runoff* (Institution of Engineers Australia 1987) were adopted for analysis. It was considered that the time difference between the standard storm durations for the short duration (15 to 20 minute) storms likely to produce peak conditions for minor events was sufficiently small to negate the requirement for the assessment of intermediate durations.

The only potential concern in relation to the use of a hydrographs based on the Rational method was with respect to the runoff volume produced by the hydrograph. As the XP-STORM model was being used in the investigation specifically to account for storage effects, it was important that the model incorporated the likely volume of runoff from the catchment.

The acceptability of the use of the QUDM runoff coefficient as a volumetric runoff coefficient was therefore investigated. Based on the runoff coefficients adopted for each of the various land uses within the study area (refer Section 2.3.2), the following runoff coefficients were found to be typical for the catchment:

- Minor Events
 - 2 Year: 0.71 (i.e. loss of 29% of total runoff volume via infiltration)
 - 10 Year: 0.85 (i.e. loss of 15% of total runoff volume via infiltration)

- Major Events
 - 20 Year: 0.89 (i.e. loss of 11% of total runoff volume via infiltration)
 - 50 Year: 0.97 (i.e. loss of 3% of total runoff volume via infiltration)
 - 100 Year: 0.99 (i.e. loss of 1% of total runoff volume via infiltration)

For the major storm event according to the Rational Method, virtually all of the rainfall is converted to runoff. This is consistent with the general practice of assuming catchment saturation at the commencement of a major storm event such as the 100 year event. Consequently, the use of hydrographs based on the Rational Method is acceptable with respect to the volume of runoff for major storm events.

For minor storm events, it is likely that the catchment will not be saturated at the commencement of the event and that therefore some loss of rainfall to infiltration could be expected. The Rational Method runoff coefficient used as a volumetric runoff coefficient predicts a rainfall loss of between 29 percent (2 year event) and 15 percent (10 Year event). In comparison, a similar loss rate was adopted for modelling minor storm events using the Laurenson (RAFTS) method in order to provide flows compatible with QUDM (refer Section 2.2.2).

Consideration of typical soil loss rates and the effect of antecedent weather conditions according to the method of the United States Department of Agriculture (USDA method, as quoted in Chow and Queensland Water Resources 1980) indicated that, for the short storm durations under consideration, loss rates of the order of thirty percent were not unreasonable for the soil types present within the study area.

Therefore, it was concluded that the use of Rational Method peak runoff coefficients as volumetric runoff coefficients provided overall runoff volumes equivalent to those which would be produced via the other commonly used method for hydrograph derivation (i.e. Laurenson (RAFTS)) and which were acceptable with respect to likely infiltration during actual rainfall events.

In summary, it was found that the optimal method for hydrograph derivation given the nature of the catchment and its likely future development was provided by a direct application of the Rational Method. This provided inflow hydrographs entirely consistent with the *Hervey Bay City Council Development Manual* (Hervey Bay City Council 1997) and the *Queensland Urban Drainage Manual*.

2.3 Derivation of Hydrologic Parameters

2.3.1 Time of Concentration

Section 5.05.4 of the *Queensland Urban Drainage Manual* (QUDM)(Neville Jones & Associates 1992, p 5-15) notes that:

“The use of standard inlet times for developed catchments is recommended because of the uncertainty related to the calculation of time of overland flow. The standard inlet times should be adopted except where subcatchment characteristics indicate that detailed overland flow calculations are justified.”

Section 16.3.3 of the *Hervey Bay City Council Development Manual* (Hervey Bay City Council 1997, p 16-6) allows the use of the standard inlet times presented in QUDM.

Given the above and the transparency of calculation offered by the use of standard inlet times, the standard inlet times presented in Table 5.05.1 of QUDM were adopted for the analysis. Due to the relatively low grades within the study area, the majority of the subareas were assigned a time of concentration of 15 minutes. As the inlet time is defined as **“the combined time for overland flow and channel flow to the gully inlet under consideration”** (Neville Jones & Associates et al 1992, p 5-15), the time of concentration at nodes not directly connected to the underground stormwater drainage network was reduced to account for the likely channel (i.e. road) travel time to the closest inlet to the inflow point.

The time of concentration adopted for each subcatchment is shown in **Table A1-Catchment Areas, Weighted Runoff Coefficients and Times of Concentration in Appendix A- Hydrologic Data.**

2.3.2 Rainfall Intensity

Rainfall intensities for the catchment were derived in accordance with Volumes 1 and 2 of *Australian Rainfall and Runoff* (Institution of Engineers Australia 1987). The derived rainfall intensities are listed in **Table 1- Design Rainfall Intensities**.

TABLE 1
Design Rainfall Intensities

Storm Duration	Rainfall Intensity (mm/h)				
	2 Year	10 Year	20 Year	50 Year	100 Year
15 Min	94.8	134	154	180	200
20 Min	82.8	117	134	157	174
25 Min	74.1	105	120	140	156
30 Min	67.5	95.2	109	128	142
45 Min	54.3	76.4	87.6	102	113
1 Hour	46.2	65.0	74.4	86.8	96.3
90 Min	35.7	50.7	58.2	68.2	75.9
2 Hour	29.6	42.4	48.8	57.3	63.8

2.3.3 Fraction Impervious Values and Runoff Coefficients

Appropriate fraction impervious values for each type of landuse within the catchment were derived based on the zonings presented in the Hervey Bay City Council publication *Statement of Proposals, Hervey Bay Planning Scheme Amendment* (August 1998). The zoning plan for the Urangan precinct is reproduced as **Figure A2- Zoning Plan** in **Appendix A- Hydrologic Data**.

The majority of the study area is designated as medium density. It contains the following land use types:

- Residential Low Density
- Residential Medium Density
- Foreshore Medium Density
- Local Shopping
- Village Centre
- Open Space

The area of each type of zoning present within each of the subcatchments defined for the investigation is presented in **Table A1- Catchment Areas, Weighted Runoff Coefficients, and Times of Concentration** in **Appendix A- Hydrologic Data**.

The zonings presented in the Statement of Proposals were adopted for use in the study except for the Open Space zone indicated to the North of King Street between Pier Street and Pilot Street. For the analysis, it was assumed that this area would be developed as a Medium Density Residential area.

The fraction impervious values listed for various development categories in the *Queensland Urban Drainage Manual* (Neville Jones & Associates 1992, p 5-10) were correlated with the various land use zonings present within the study area according to the *Hervey Bay City Council Development Manual* (Hervey Bay City Council 1997, p 16-6) to provide appropriate fraction impervious values for each land use within the catchment. The adopted fraction impervious values were then applied to Table 5.04.2 of QUDM (Neville Jones & Associates et al 1992, p 5-11) to derive suitable values of the runoff coefficient for the 10 year event. The derived values were subsequently compared to those adopted for the *Toogoom Area Drainage Study* (John Wilson and Partners 1998) and any coefficients significantly different to those adopted for the Toogoom study altered appropriately.

Frequency factors of 0.85, 1.05, 1.15 and 1.20 were applied to the 10 year event coefficients to derive the coefficients applicable to the 2, 20, 50 and 100 year events respectively (Neville Jones & Associates 1992, p 5-11). As recommended by QUDM, runoff coefficient values in excess of unity were rounded down to 1.0.

The adopted values of fraction impervious and runoff coefficient are listed in **Table 2- Adopted Fraction Impervious Values and Runoff Coefficients**.

TABLE 2
Adopted Fraction Impervious Values and Runoff Coefficients

Land Use	QUDM Equivalent	Adopted Fraction Impervious	Adopted Runoff Coefficient				
			2 Year	10 Year	20 Year	50 Year	100 Year
Residential Low Density	Urban Residential High Density	0.7	0.701	0.825	0.866	0.949	0.99
Residential Medium Density	Urban Residential High Density	0.8	0.723	0.850	0.893	0.978	1.0
Foreshore Medium Density	Urban Residential High Density	0.8	0.723	0.850	0.893	0.978	1.0
Local Shopping	Central Business	1.0	0.765	0.900	0.945	1.0	1.0
Village Centre	Commercial	0.9	0.748	0.880	0.924	1.0	1.0
Road Reserve	Road	0.7	0.701	0.850	0.893	0.978	1.0
Open Space	Open Space	0.0	0.561	0.660	0.693	0.759	0.792

3.0 HYDRAULIC MODELLING

3.1 General

The XP-STORM model of the Urangan catchment developed for the study contained the following number of links and nodes:

- 269 Links, comprising:
 - 114 Surface links (natural channels)
 - 7 Weirs to represent surface overflow conditions
 - 90 Links to represent the underground drainage system (closed circular conduits)
 - 58 Special links to represent the connections between the surface and underground drainage networks.
- 199 nodes, comprising:
 - 102 Nodes to model the surface network (71 nodes receiving inflow hydrographs)
 - 97 Nodes to model the underground drainage network.
- 12 Points of outlet:
 - 10 outlet points for the existing/ proposed drainage system
 - 2 outlet points for overland flow at the downstream end of Pier and Pulgul Streets.

To facilitate the use of the model, the following colour convention was adopted:

- Surface links: Red
- Surface Nodes: Red if not receiving inflow hydrographs
Black if receiving inflow
- Underground Links: Blue
- Underground Nodes: Blue
- Surface to Underground Links: Black

The naming convention adopted in general for nodes was as follows:

Node name: N*#%/&&\$

- Where:
- N = Node
 - * = Catchment (A, B, C, or D, for Elizabeth St, Pilot St, Pier St, or Guard St respectively)
 - # = S (surface node) or U (Underground node)
 - %% = Drainage Line number (eg 01, 02 etc for each catchment)
 - && = Node number on drainage line in downstream direction (i.e. 01, 02, 03 etc)
 - \$ = Optional letter (A) to signify upstream end of short pipes discharging to main drainage line with otherwise the same node name.

For example, node NAS03/02 is a node located on the surface of catchment A (Elizabeth Street) and is the second node in the third drainage line of the catchment. In general, the underground drainage nodes were named after the corresponding surface node.

The naming convention adopted in general for links was as follows:

Link name: L*#%/%/%%&&\$@~~

Where	L	=	Link
	*#%/%/%%&&\$	=	Name of node at upstream end of link,
	@	=	Name of downstream catchment (A, B, C, or D) Optionally used if direction of link was to another catchment or contrary to direction of other links
	~~	=	Name of drainage line number at downstream end of link (eg 01) Optionally used if direction of link was to another catchment or contrary to the direction of other links.

For example, link LBU01/04 is an underground link in catchment B (Pilot Street) with upstream node NBU01/04. Link LBS01/04C02 is a surface link in catchment B with upstream link NBS01/04). The link drains to drainage line 02 on catchment C (Pier Street).

The use of the above naming format allowed the creation of a unique set of node and link names for the model.

The adopted model layout is shown in **Figures B1 to B5- Model Layout in Appendix B- Hydraulic Data.**

All of the data necessary to define the links and nodes for the model was created in spreadsheets to facilitate data entry and modification. Each spreadsheet was output as a text file in XPX format (XP Software 1998, pp 110 to 115) and subsequently imported to the XP-STORM model. Separate spreadsheets were created for the surface network, the underground drainage network, and the cross links (i.e. gully inlets) between the surface and underground networks.

3.2 Surface Network

To correctly account for the available storage within road reserves within the catchment, the road network was divided into a total of 114 channel links. The location of the links and associated nodes was based on crest and sag locations within the existing road network and proposed road regrading works.

The cross sectional information required for the model was obtained from spot levels and contours of the catchment derived from the asset data capture program undertaken by Hervey Bay City Council. The accuracy of the level information collected by the asset data capture program is understood to be ± 70 mm and therefore quite reliable for flood modelling purposes (Cardno & Davies 1998). This level information was correlated with the information collected regarding pipe inverts to provide details of surface levels within the study area. Checks were conducted upon the collected data to ensure that adopted node invert levels and minimum cross section levels were consistent with respect to the adopted direction of flow for each link.

For each link, cross sections were extracted at a point midway between the upstream and downstream nodes of the link. In all cases, the orientation of sections was assumed to be left to right looking downstream.

For the analysis, it was assumed that the following roads would be regraded (Refer **Figure 2- Pilot Street 20 Year Stormwater Truck Drainage Strategy**):

- Coral Street
- Shell Street between Elizabeth Street and Pier Street
- Sandy Street
- King Street between Pilot Street and Pier Street
- Beulah Street
- Johnson Street
- Hibiscus Street between Elizabeth Street and Pier Street
- Pier Street between Dayman Street and King Street
- Corner of Pilot Street and Charlton Esplanade

King Street is designated as a Collector Road under the *Hervey Bay Planning Scheme Amendment* (Hervey Bay City Council 1998, Road Hierarchy Plan). Accordingly, a pavement width of 7.5 metres (Hervey Bay City Council 1997, Table 13.2.1) was adopted for the section of the street to be regraded. The remainder of the streets to be regraded are designated as Residential Access Streets under the Planning Scheme. For these streets, a pavement width of 5.5 metres (Hervey Bay City Council 1997, Table 13.2.1) was adopted. A cross fall of 1 in 40 was assumed for all new road surfaces.

For all regraded roads, a mountable type M1 kerb and channel (Hervey Bay City Council Standard Drawing R-11, 1998) was assumed. In existing areas, a type barrier type B1 kerb and channel was adopted (Standard Drawing R-11).

To account for future filling works encroaching within the road reserve, it was assumed that filling would be permitted to a point one metre inside the road reserve on both sides of the road. A ground slope of 1 (V) in 6 (H) was assumed for the fill batter (Hervey Bay City Council Standard Drawing R-04, 1998).

Road gradings for each new road were based upon existing road level junction constraints. Where possible, road gradings of 0.25 % were adopted.

For each section, it was possible to define three portions and to assign different Mannings 'n' values to each portion. For the analysis, each section was divided into the left verge, the road (including kerb and channel), and the right verge. For existing roads a Mannings n roughness value of 0.018 was adopted while for new roads a value of 0.014 was considered appropriate. Verges were considered to have an 'n' value of 0.04.

The general data relating to the surface link network is listed in **Table B1- Surface Links- General Data** in **Appendix B- Hydraulic Data**. Cross sectional data input to the model is presented in **Table B2- Surface Links- Cross Sections** in **Appendix B- Hydraulic Data**. Adopted surface node invert levels are shown in **Table B3- Adopted Invert Levels for Surface Nodes** in **Appendix B- Hydraulic Data**.

In addition to the channel links, seven weir type links (modelled as multilinks in XP-STORM) were added to the model to account for situations where water levels would need to rise to a certain level prior to overland flow commencing. Weirs were input to allow for overland flow at the downstream end of Pilot Street and at the downstream end of Pulgul Street. For the Pilot Street overland flow path, it was assumed that a 20 metre wide flow path would be available to convey flow during major storm events.

Three weirs were used to define flow conditions at the junction of King Street and Pilot Street. At this location, water ponds in Pilot Street prior to discharging across the slightly higher King Street to the downstream area of Pilot Street. Weirs were used to define this flow relationship. Two additional weirs were used to allow for the transfer of flow between nodes on Dayman Street. Details of the surface weir links are listed in **Table B4- Surface Weirs of Appendix B- Hydraulic Data**.

3.3 Underground Network

3.3.1 General

The underground stormwater drainage system was modelled using closed conduit links. Information relating to the existing drainage network was collected by Council as part of its Asset Data Capture program. This information was used to recreate the existing drainage system in the XP-STORM model.

Conduits were also added to the model to represent the proposed relief drainage works. These conduits were adjusted as appropriate during the course of the investigation to provide adequate drainage capacity.

In addition to the inclusion of links to represent the main drainage lines, links were also added at points of major inflow to the stormwater system to allow the effect of pipes connecting gully pits to the main drainage line to be accounted for when considering the acceptability of calculated water levels.

A total of 97 links were added to the model to account for the underground drainage system. A Mannings 'n' value of 0.013 was adopted for all pipes. All pertinent data relating to the underground drainage network is presented in **Table B5- Stormwater Drainage Details in Appendix B- Hydraulic Data**. The table includes information on the length, diameter and number of pipes, together with node connectivity information and adopted junction loss values (refer Section 3.3.2).

Of the proposed remedial works, the Pier Street beach outfall has been designed. Details of the proposed outfall were therefore incorporated in the 2003 model. The outfall design was drawn from Cardno MBK drawings 2919/37.01-01 to 19.

Invert levels for the underground drainage system (i.e. node inverts) were adopted based on the results of the asset data capture program for the existing drainage system and calculated allowing 600 mm minimum cover to the top of pipes for new drainage lines. Adopted invert levels for the stormwater drainage system are presented in **Table B6- Adopted Invert Levels for Underground Nodes in Appendix B- Hydraulic Data**.

3.3.2 Manhole Losses

Although loss coefficients can be input to account for manhole losses, the Hydraulic (EXTRAN) mode of XP-STORM is not capable of dynamically altering manhole losses to suit flow conditions. Only a single coefficient (multiplied by the velocity head in the downstream pipe) can be input for each pipe to represent losses at junctions throughout a storm event.

In order to assign appropriate manhole loss coefficients, recourse was made to the generalised values presented by Argue in the publication *Storm Drainage Design in Small Urban Catchments- A Handbook for Australian Practice* (1986). Argue recommends single values for given junction scenarios which eliminates the need for extensive reference to charts as required under the *Queensland Urban Drainage Manual* (QUDM)(Neville Jones & Associates et al 1992, Section 5 and Volume 2). When comparing designs completed using his simplified values and designs completed using the Missouri Charts (Sangster et al 1958, as quoted in Argue 1986, p 45), Argue found that the resultant difference between the designs was minimal (Argue 1986, p 106).

The values proposed by Argue (Argue 1986, pp 46-48) were compared with values likely to be derived from the QUDM charts and a generally conservative amalgam of junction pit loss coefficient values derived. The manhole loss coefficients adopted for the investigation for various pipe configuration scenarios are listed in **Table B7- Adopted Manhole Loss Coefficients** in **Appendix B- Hydraulic Data**. The loss coefficients adopted for each junction are listed in **Table B5- Stormwater Drainage Details** in **Appendix B- Hydraulic Data**. The loss values entered for each link refer to the upstream end of the pipe concerned.

3.3.3 Gross Pollutant Traps

It is understood that Council is considering the installation of gross pollutant traps as a water quality improvement measure (Cardno & Davies 1999). To ensure that the inclusion of the traps will not compromise the design of the relief drainage works, the potential head loss associated with gross pollutant traps was included in the XP-STORM model of the catchment.

For the analysis, it was assumed that gross pollutant traps would be installed near the downstream end of all relief drainage lines. Gross pollutant traps were added to the following nodes in the model (refer **Figures B1 to B5- Model Layout** in **Appendix B- Hydraulic Data**):

(A)	Elizabeth Street Catchment	NAU01/12
(B)	Pier Street Catchment	NBU01/12
(C)	Pilot Street Catchment	NCU01/11
(D)	Guard Street Catchment	NDU01/07

No gross pollutant traps were added to existing drainage lines on the basis that the existing drainage lines are already undersized and that the additional head loss caused by adding traps to the existing lines would only exacerbate flooding problems. If Council were to install gross pollutant traps on existing drainage lines, it was considered that works to compensate for the increase in flood level would be completed in conjunction with trap installation (eg upgrading of adjacent pipes to provide the same overall head loss as previously) to provide a no-worsening solution.

CSR-Humes recommend a loss coefficient of 1.5 for its precast Humeceptor gross pollutant trap (CSR-Humes 1996). Rocla indicate that the appropriate loss coefficient for its Cleansall pollutant trap is between 0.75 and 1.8 (Rocla 1998).

Discussions were held with representatives of CDS Technologies (producer of the CDS pollutant removal unit). The representatives indicated that their units are purpose designed for each site to an allowable head loss specified by the client. Consequently, no figure can be directly applied to the modelling of losses due to CDS units.

Given the loss coefficient values quoted by CSR-Humes and Rocla, a manhole loss coefficient of 1.5 was adopted for each gross pollutant trap. This figure was added to the loss coefficient applicable to the node at which the trap was placed to provide a slightly conservative estimate of the likely head loss associated with the introduction of each gross pollutant trap.

3.4 Stormwater Inlet Pits

3.4.1 General

To model stormwater inlet pits and the interaction between the surface and underground drainage networks during surcharge conditions in the pipe system, stormwater inlet pits were modelled as weirs via the multiple conduit/ diversion option in the XP-STORM model (XP Software 2002, pp 243-244). Inlet pits were modelled using weirs rather than rating curves due to the ability of the Hydraulic (EXTRAN) mode of XP-STORM to model the drowning of weirs when the downstream water level approaches the upstream water level, thereby allowing the simulation of surcharge conditions and restricted inflow conditions during major storm events.

The required weir relationships were derived by first considering the inlet capacity of each of the various types of inlet pit present within the catchment. Hervey Bay City Council standard 1 bay, 2 bay, 3 bay and 4 bay side entry pits (Hervey Bay City Council standard drawings SD 010 and SD 011 Revision 0) were considered together with grated inlets (existing drainage system only). It can be noted that side entry pits were included for all new drainage lines analysed. For all types of inlet, separate inlet capacity curves were derived for sag and on grade conditions.

The inlet capacity curves were then converted to equivalent weir relationships by calculating the length of weir required at each defined water level to produce a flow equivalent to the inlet capacity for that level.

For weir relations, XP-STORM requires that the weir length, coefficient and discharge exponent is specified for each depth entered in the relation.

It was found that the standard value of discharge exponent (1.5) produced a significant variation in the weir length required with increasing depth. To minimise the variation in weir length with depth (and to thereby reduce the potential for instabilities in the model), a discharge exponent of 0.2 was adopted. A standard weir discharge coefficient of 1.7 was adopted for all water depths.

Further, the weir tables had to be adjusted at very small depths because the low exponent value generated appreciable flows at very small depths over the weir. This was overcome by using weir parameters for flow depths to 0.001 m (1 mm head on the weir) as listed in **Table 3- Inflow Rating Curves- Weir Correction Parameters**.

**TABLE 3
Inflow Rating Curves - Weir Correction Parameters**

Head on Weir (m)	Weir Length (m)	Exponent	Coefficient
0.0	As for head 0.08	1.5	0.1
0.001	As for head 0.08	1.0	1.0
0.08	Calculated	0.2	1.7

The adopted inlet capacity curves are presented in **Table B8- Inlet Capacity of Side Entry Pits and Grates** and **Table B9- Equivalent Weir Dimensions for Inlet Pits** in **Appendix B- Hydraulic Data**.

Due to the number of inlet pits present within the catchment, inlet pits draining to the same node on the underground drainage system were aggregated and modelled as a single inlet.

The weir relation used to define flow into the underground drainage system was also used to define flow to the surface in the event of surcharge conditions within the pipe network. Due to current limitations of the Hydraulic (EXTRAN) mode of XP-STORM, it is not possible to model one directional weirs. Such a facility would have allowed the specification of separate relations for flow into and out of the underground system. Other methods to provide the desired replication of flow conditions were not able to be justified due to the prohibitively large number of links and nodes which would be required. A review of the surcharge capacity of inlets indicated that the use of inflow capacity curves underestimated the outflow capacity to some degree.

3.4.2 Sag Inlets

The Hervey Bay City Council side entry pit capacity curves for sag conditions are presently based upon the United States Department of Transportation formula for weir flow. At higher depths above the invert level of the pit, the capacity of inlets will be limited by orifice type flow (Neville Jones & Associates et al 1992, p 5-47). Based on the available orifice area for each of the various side entry pit types (calculated from Hervey Bay City Council standard drawings), curves were produced for each type of side entry pit for weir and orifice type flow conditions and the resulting flow envelope adopted.

It can be noted that the inlet curves for side entry pits are not based on model tests of the actual Council design. Full size model testing of Brisbane City Council gully inlets by the Urban Water Resources Centre, University of South Australia (Argue 1994, as quoted in Brisbane City Council 1994, Chart QUDM-1) has indicated that the capture rate of inlet pits when the water level is about 90 mm above the top of the kerb is significantly less than that predicted by the orifice equation.

Although the orifice equation and normal hydraulic considerations indicate that the flow into an inlet should increase with increasing water level, the results of model testing led to the recommendation that the capacity of inlets not be extrapolated beyond a certain point. A similar result was obtained from testing of the Rocla 'Drainway' unit (Argue 1993, as quoted in QUDM Volume 2).

Given the experimental results, the inflow relations for inlet pits were conservatively limited at depth, to reflect the outcome of model testing of other pit types.

Based on the orifice area of the Hervey Bay side entry pits and the orifice area of the Brisbane City Council and Drainway inlets, the following limiting flow capacities were conservatively adopted for the study:

- 1 Bay 270 L/s
- 2 Bay 310 L/s
- 3 Bay 375 L/s
- 4 Bay 475 L/s

The calculated sag capacities were reduced by 20 percent to account for blockage effects (Hervey Bay City Council 1997, p 16-9, Neville Jones & Associates 1992, p 5-42).

For grated inlets to the existing drainage system, a grate with a gross area of 0.5 m² was assumed. This area was reduced by 25 percent to account for the reduction in waterway area caused by the bars of the grate. The inlet capacity of the grate was defined according to the weir and orifice equations (Neville Jones & Associates 1992, pp 5-47 to 5-48). The calculated capacity of the grate inlet was reduced by 50 percent to account for blockage effects (Hervey Bay City Council 1997, p 16-9, Neville Jones & Associates 1992, p 5-42)

3.4.3 On Grade Inlets

The inlet capacities of on grade side entry pits and grates were derived from the Hervey Bay City Council gully pit capacity charts which relate captured flow to gutter flow. In order to provide the required level-inflow capacity relation, the water depth associated with various gutter flows was determined.

The water level for a given gutter flow varies according to the longitudinal and cross slope of the road and road width. To provide typical water levels for the types of road present within the study area, roads with widths of 8 metres and 12 metres and longitudinal slopes of between 0.25 percent and five percent were analysed using the backwater program HEC-RAS. The roads were assumed to have a cross fall of 1 in 30 and to lie within a 20 metre road reserve. The results of the analysis were used to assign water levels to a range of gutter flows and thereby establish the necessary stage discharge relations for on grade side entry pits and grates.

The calculated on grade capacities were reduced by 20 percent in the case of side entry pits and 50 percent in the case of grates to account for blockage effects (Hervey Bay City Council 1997, p 16-9, Neville Jones & Associates 1992, p 5-42).

3.5 Tailwater Levels

The following tailwater levels were adopted for the analysis (Hervey Bay City Council 1997, p 16-14):

- Minor Storm Event (2 and 10 Year) RL 1.50 m AHD (MHWS + 0.3 m Greenhouse Effect).
- Major Storm Event (20, 50, and 100 Year) RL 2.40 m AHD (HAT + 0.3 m Greenhouse Effect).

3.6 Model Parameters

The Hydraulic (EXTRAN) mode of XP-STORM allows the creation of hotstart files for use in design runs. For both the Minor and Major events, hotstart files were created by defining small (0.01 cumec) flows in upstream areas of the catchment and then running the model for a sufficient period to provide stable conditions within the model. The hotstart file produced by the model was then used to define initial water levels and flows in the system for design event modelling.

Due to the difference in tailwater level (refer Section 3.5), separate hotstart files were produced for minor and major flood events.

The minor drainage design standard for the catchment varies between the 2 year and 10 year event. For this reason, both the 2 year and 10 year events were modelled to ensure the adequacy of the designed relief drainage system for minor event flooding. The 20, 50, and 100 year events were modelled to allow the assessment of system performance under major event flooding.

For each recurrence interval analysed, standard storm durations of between 15 minutes and ninety minutes were input to the model. The results of modelling were output in CSV (comma separated variable) format and imported to spreadsheets. The spreadsheets were used to determine peak flood levels, flows and velocities across the range of storm durations modelled.

For the analysis, a relatively short time step of 2 seconds was adopted due to the complexity of the model. For the analysis of major storm events, the time step was reduced to 1 second to promote stability. Model run times for major event storms were found to be considerably longer than those required for minor storm events. This was attributed to the drowning of inlet pits during major flow conditions and the model having to complete a significant number of iterations to produce a solution at each time step.

4.0 MODELLING RESULTS

4.1 General

Modelling of the Urangan system was an iterative process involving the resizing of pipe elements and the addition or deletion of inlet pits until an acceptable solution was achieved for the drainage of minor event flows. The performance of the relief drainage system under major storm events was then assessed and pipe sizes adjusted if necessary to produce acceptable conditions during major events.

For the analysis of major event storms, storms with recurrence intervals of 20, 50, and 100 years were considered. It is important to note that the same relief drainage system was adopted for all of the major events. The selection of a major event other than the 100 year event for the specification of habitable floor levels only provides a reduction in the required floor level and does not impact on the drainage works adopted for the catchment.

For the minor storm events (2 and 10 year recurrence interval) storm durations of between 15 and 25 minutes were found in general to produce peak conditions within the catchment. For the major storm events (20, 50, and 100 year), storm durations of between 15 minutes and 1 hour were found to produce peak conditions.

The results of modelling of the adopted relief drainage works are presented in the following tables in **Appendix C- Model Results**:

General:

Table C1 - Surface Network- Peak Water Levels and Depths

Minor Event Results

2 Year Event:

Table C2 - Peak Water Levels- Surface Network- 2 Year Event
Table C3 - Peak Water Levels- Underground Network- 2 Year Event
Table C4 - Peak Flows and Velocities- Surface Network- 2 Year Event
Table C5 - Peak Flows and Velocities- Underground Network- 2 Year Event

10 Year Event:

Table C6 - Peak Water Levels- Surface Network- 10 Year Event
Table C7 - Peak Water Levels- Underground Network- 10 Year Event
Table C8 - Peak Flows and Velocities- Surface Network- 10 Year Event
Table C9 - Peak Flows and Velocities- Underground Network- 10 Year Event

Major Event Results

20 Year Event:

Table C10 - Peak Water Levels- Surface Network- 10 Year Event
Table C11 - Peak Water Levels- Underground Network- 10 Year Event
Table C12 - Peak Flows and Velocities- Surface Network- 10 Year Event
Table C13 - Peak Flows and Velocities- Underground Network- 10 Year Event

50 Year Event:

Table C14	-	Peak Water Levels- Surface Network- 10 Year Event
Table C15	-	Peak Water Levels- Underground Network- 10 Year Event
Table C16	-	Peak Flows and Velocities- Surface Network- 10 Year Event
Table C17	-	Peak Flows and Velocities- Underground Network- 10 Year Event

100 Year Event:

Table C18	-	Peak Water Levels- Surface Network- 100 Year Event
Table C19	-	Peak Water Levels- Underground Network- 100 Year Event
Table C20	-	Peak Flows and Velocities- Surface Network- 100 Year Event
Table C21	-	Peak Flows and Velocities- Underground Network- 100 Year Event

KEY RESULTS OF STUDY:

The recommended drainage works and floor levels for the catchment are presented in:

Figure 2- Pilot Street - 20 Year

Figure 3- Pilot Street - 50 Year

Figure 4- Pilot Street - 100 Year

*The proposed relief drainage works are tabulated in **Table B5- Stormwater Drainage Details in Appendix B- Hydraulic Data**. This table also lists the pipes proposed between each gully pit and the main drainage lines.*

*The invert levels adopted for each drainage line are presented in **Table B6- Adopted Invert Levels for Underground Nodes in Appendix B**. The adopted number and size of inlet pits are listed in **Table B10- Adopted Inlet Pits in Appendix B**.*

*The calculated flood levels for the catchment are summarised in **Table C1- Surface Network- Peak Water Levels and Depths in Appendix C- Model Results**.*

Table C1 provides a summary of the peak water levels calculated within the study area. Flood levels presented in the table can be located via Figures B1 to B5 in **Appendix B- Model Data**.

Each table lists the relevant peak value obtained from the range of storm durations modelled and quotes the storm duration which produced the peak value.

In the case of the underground drainage network peak water level tables (Tables C3, C7, C11, C15 and C19), the surface level applicable to each node is specified together with whether the water level in the underground system is above (i.e. surcharging) or within 150 mm of the surface level.

In the case of the peak flow and velocity tables for the surface (i.e. road) network (Tables C4, C8, C12, C16, and C20), the water depth at the upstream and downstream ends of each link is listed together with the resultant value of velocity multiplied by depth.

It can be noted that for relief drainage works of the type considered in the investigation, the Queensland Urban Drainage Manual (Neville Jones & Associates et al 1992, p 11-3) notes that

“Whilst the criteria set down in this Manual should be adhered to if possible for relief drainage works, economic and physical limitations may require the adoption of less stringent criteria.”

Consequently, the judgement of whether proposed drainage works are acceptable is somewhat subjective in relief drainage investigations. The following design criteria were adopted for the investigation:

DRAINAGE DESIGN CRITERIA		
Minor Events (2 and 10 Year)		
<i>Pipe Drainage:</i>	In General:	Freeboard to surface invert >150 mm. No surcharge within existing drainage lines connected to new drainage lines.
	Worst Case:	Water level within 150 mm of surface invert. No surcharging allowed on new drainage lines.
<i>Surface Drainage:</i>	In General:	Water levels in road system and above gully pits to be not greater than the top of kerb level.
	Worst Case:	Maximum depth of water not to exceed 250 mm in areas serviced by new drainage lines.
Major Events (20, 50, and 100 Year):		
<i>Surface Drainage:</i>	In General:	Water depth not to exceed 450 mm. Velocity Depth product not to exceed 0.6 m ² /s.
	Worst Case:	Absolute maximum water depth (neglecting tailwater) 1,200 mm. Increase in water depth above tailwater level not to exceed 750 mm. Velocity Depth product not to exceed 0.9 m ² /s.

It can be noted that the adopted maximum depth criteria is consistent with that specified by Hervey Bay City Council (Hervey Bay City Council 1997, p 16-20) and QUDM (Neville Jones & Associates 1992, p 6-17).

The following sections detail the drainage solution adopted for each major subcatchment and provide comment with respect to the acceptability of the proposed solution.

4.2 Minimum Floor Level

The minimum habitable floor level to be adopted at any point in the study area is the maximum of the flood level due to local flooding and that due to ocean storm surge. At the downstream end of the catchment the minimum level is governed by the ocean storm surge level.

Hervey Bay City Council has indicated that the following minimum habitable floor levels are applicable for the Urangan catchment for the case of ocean storm surge (Hervey Bay Beach Report Table 7.1, Charts 7-10 and 7-11, p160, John Wilson & Partners Toogoom Area Drainage Study 1998, as quoted in Lavey, 2000):

- 20 Year Event RL 3.40 m AHD
- 50 Year Event RL 3.80 m AHD
- 100 Year Event RL 4.15 m AHD

The levels quoted above include the ocean level for each event plus the following components:

- Greenhouse allowance 0.3 m
- Wave Setup 0.2 m for 20 year event
 0.3 m for 50 year event
 0.38 m for 100 year event
- Wave runup 0.1 m
- Freeboard 0.3 m
- Habitable floor level allowance 0.15 m

It can be noted that the fill level required to achieve immunity to flooding is 150 mm less than the specified habitable floor level. The specified habitable floor level therefore has a freeboard of 450mm.

In upstream areas of the catchment, flood levels due to local catchment events govern minimum required floor levels. In such areas, the habitable floor level was defined as being 250 mm above the relevant flood level. In order to provide a transition between the 450 mm freeboard specified for the storm surge event and the 250 mm freeboard adopted for catchment flooding, a freeboard of 350 mm was applied to the flood level predicted for catchment flooding either side of Shell Street.

The required minimum habitable floor levels within the catchment are shown on the following figures:

- Figure 2- Pilot Street- 20 Year**
- Figure 3- Pilot Street- 50 Year**
- Figure 4- Pilot Street- 100 Year**

It can be noted that the criterion for minimum habitable floor level can differ depending on the land use (eg Commercial versus Residential) and type of development. The levels adopted for this report relate to residential land use. Council can therefore consider the relaxation of nominated habitable floor levels in certain circumstances.

4.3 Elizabeth Street Catchment (A)

Drainage design standard: Q10

The inclusion of the properties between Coral Street and Dayman Street in the total catchment area of the Urangan coastal strip necessitated the extension of the Elizabeth Street drainage system derived in the Concept Phase Investigation to the low point located on Elizabeth Street midway between Coral Street and Dayman Street.

The following drainage system was found to be necessary for the Elizabeth Street catchment:

- | | |
|---|-------------|
| ▪ Midway between Coral St and Dayman St to Coral St | 1/450 Dia |
| ▪ Coral St to Shell St | 1/600 Dia |
| ▪ Shell St to downstream of King St | 1/900 Dia |
| ▪ Downstream of King St to Hibiscus St | 2/900 Dia |
| ▪ Hibiscus St to low point upstream of Charlton Esplanade | 3/900 Dia |
| ▪ Low point to ocean | 3/1,200 Dia |

Adopted invert levels varied from RL 0.0 m AHD at the outlet of the system to RL 2.1 m AHD at the low point between Dayman Street and Coral Street.

Minor Storm Event Performance

Under minor event flow conditions, the pipe drainage system complies with the requirements of the *Queensland Urban Drainage Manual (QUDM)* (Neville Jones & Associates 1992, Section 5) and the Hervey Bay City Council *Development Manual* (Hervey Bay City Council 1997).

The existing drainage lines on King Street and Cyprus Street will ultimately be connected to the new Elizabeth Street pipe system. The analysis indicated that tailwater levels in the new drainage line were sufficiently low to prevent surcharging on existing drainage lines.

Flow conditions within the existing drainage line between Hibiscus Street and Charlton Esplanade (LAU07/03 and LBU06/01) were found to be acceptable due to the low inlet capacity of the system and the difficulty of providing relief drainage to this location.

Water depths in the road system were found to be 150 mm or less within new drainage areas and were therefore considered to be within acceptable limits.

Major Storm Event Performance

For major event flooding, the drainage system was found to surcharge (i.e. the water level in the underground system is greater than the adjacent ground level) over the majority of its length.

However, except at the low points located upstream of Charlton Esplanade (NAS01/11) and downstream of King Street (NAS01/09), water levels in the road network were found to be less than 450 mm. At the two low points, water depths up to 580 mm were calculated.

Due to the lack of an overland flow path to the ocean at the downstream end of Elizabeth Street, the low point upstream of Charlton Esplanade acts as a collection point for flow not collected in the upstream pipe drainage system. As a consequence, ponding of water to a reasonable depth is unavoidable. Although the calculated depth is considerable, enlargement of the downstream drainage system to further reduce the flood level at the low point is not considered to be warranted.

The low point downstream of King Street collects surface runoff from a considerable length of King Street. Only a relatively small portion of the runoff in King Street is collected by the existing drainage line on King Street, with the remainder of the runoff travelling to the low point on Elizabeth Street.

As the resultant depth of flooding is not considered to be excessive, it is preferable to accept the depth of flooding at the low point instead of upgrading the existing King Street drainage system which would be likely to just move the problem to a point further downstream.

The velocity depth product at all locations for all major events was found to be $0.5 \text{ m}^2/\text{s}$ or less (in general less than $0.3 \text{ m}^2/\text{s}$) and therefore considered to be acceptable.

Overall, it was concluded that the proposed drainage system performs acceptably for both minor and major events.

4.4 Pier Street Catchment (B)

Drainage design standard: Q10

As was the case for the Elizabeth Street catchment, the inclusion of the entire region between Coral Street and Dayman Street to the Urangan coastal strip catchment necessitated the extension of the Pier Street stormwater drainage system developed as part of the Concept Phase Investigation to the low point at the intersection of Pier Street and Dayman Street.

The proposed regrading of Pier Street south of King Street will result in the creation of sag conditions at the intersection of Pier Street with Coral Street and Pier Street with Shell Street. The drainage system therefore also needs to extend upstream of King Street in order to collect water from these sags.

The following drainage system was found to be necessary for the Pier Street catchment:

- | | |
|--|--|
| ▪ Coral St to Shell St | 1/600 Dia |
| ▪ Shell St to King St | 1/750 Dia |
| ▪ King St to downstream of between King St and Hibiscus St | 1/900 Dia |
| ▪ Between King St and Hibiscus St to Charlton Esplanade | 1/1,200 Dia |
| ▪ Charlton Esplanade | 2/900 Dia |
| ▪ Outlet to ocean | 2/2.1x0.6 RCBC and 1/750 Dia low flow pipe |

Adopted invert levels for the drainage line varied from RL 0.0 m AHD at the outlet to the ocean to RL 2.0 m AHD at Coral Street.

Minor Storm Event Performance

The proposed system was found to perform acceptably under minor flow conditions.

Flow conditions within the existing drainage line commencing at Johnson Street and running between Hibiscus Street and Charlton Esplanade (LBU05/01 and LBU04/02) were found to be acceptable due to the low inlet capacity of the system and the connection of the system to the trunk drainage line from Pier Street.

The maximum depth of water in the road system was found to be generally less than 150 mm and at worst 220 mm. The 220 mm depth was obtained on Charlton Esplanade and resulted in a flood level of RL 2.7 m (NBS06/02), which is well below the likely storm surge level. Consequently, these water depths were considered to be acceptable.

Major Storm Event Performance

For major event flooding, the drainage system was found to surcharge (i.e. the water level in the underground system is greater than the adjacent ground level) over the majority of its length.

However, except on Charlton Esplanade (NBS01/11 and NBS 01/12), water levels in the road network were found to be generally 300 mm or less and considered to be acceptable for major event conditions.

When considering conditions along Charlton Esplanade, the effect of tailwater levels on flooding needs to be recognised. With a tailwater level of RL 2.4 m AHD for the major event, Charlton Esplanade is already flooded to depths of between 40 mm (NBS01/11) and 360 mm (NBS01/12) at the start of a rainfall event. Ponding of water during the major event produces a relative increase in water depth of up to 480 mm at both locations. Therefore, the effect of catchment runoff on flood levels along Charlton Esplanade is not as great as an initial review of water depths would suggest. The enlargement of the pipes draining the Esplanade would only provide an incremental benefit as the largest pipe sizes would only produce flood depths approaching 360 mm. Therefore, an acceptable limit to ponding depth needs to be determined.

It is considered that the drainage proposed downstream of Charlton Esplanade provides a reasonable degree of flood control. If the drainage was increased in size to 2/ 1,800 mm diameter pipes, it is estimated that flood levels would be reduced by about 100 mm, providing an overall peak water depth of up to 720 mm. Given the minimal reduction in water level afforded by the enlargement and the likely additional cost associated with the use of larger pipes, it is considered that the use of the proposed drainage is acceptable.

The velocity depth product at all locations for the major events was found to be 0.7 m²/s or less (in general less than 0.3 m²/s) and therefore acceptable.

Overall, it was concluded that the proposed drainage system performs acceptably for both minor and major events.

4.5 Pilot Street Catchment (C)

Drainage design standard: Q2/ Q10 with Overland Flow Path Outlet at end of Pilot Street

Within this catchment, it was decided to retain the existing stormwater drainage system on Pilot Street upstream of King Street. Relief drainage works were therefore designed for the length of Pilot Street between King Street and Charlton Esplanade. For this catchment, it is proposed to construct an overland flow path at the end of Pilot Street to allow for the discharge of flow during large flood events to the ocean.

For the length of the new drainage system, a single 1,050 mm diameter pipe is proposed.

Adopted invert levels for the drainage line varied from RL 0.0 m AHD at the outlet of the system to RL 1.31 m AHD near King Street.

Minor Storm Event Performance

The adopted pipe size provides for the drainage of the 10 year event flow downstream of King Street without surcharging. The reduction in tailwater level afforded by the provision of relief drainage downstream of King Street was found to result in the existing drainage system upstream of King Street performing acceptably for the 2 year design event. The acceptable performance of the existing system in this region was attributed to the reduced design standard, the low inlet capacity of existing pits and the reduction of tailwater levels due to the introduction of larger diameter pipes downstream of King Street.

The existing drainage system on Charlton Esplanade (LCU04/02 and LCU1) was found to surcharge to a depth of about 200 mm for the 2 year event.

The depth of water in the road system upstream of King Street for the 2 year event was found to be 240 mm or less.

Downstream of King Street, the maximum water depth reached during the 10 year event was 180 mm or less, which was judged to be acceptable.

Major Storm Event Performance

For major event flooding, the proposed drainage system was found to surcharge. Water depths in the road system between King Street and Charlton Esplanade were found to be 580 mm (NCS01/08) or less. Although the maximum depth reached in this region was relatively high, it was considered to be acceptable for the following reasons:

- Minimum fill levels in adjacent lots which are considerably higher than the peak flood level (RL 3.71 m AHD) (refer Section 4.2),
- Inundation would be for a relatively short period, and
- There is a relatively large contributing catchment area to the sag.

The inlet pits provided at the point of maximum depth could be enlarged to reduce this flood level if required. However, this would result in an increase in flood levels upstream and thereby increase flood levels within the existing drainage network. Given the water levels calculated in the region upstream of King Street (i.e. NCS01/04 and NCS01/05), on balance it was considered preferable to accept the conditions downstream of King Street.

Upstream of King Street, the major storm event produces water depths of up to 450 mm. Given the nature of the drainage in the area, this water depth was considered to be acceptable.

Of particular importance is the result that water levels in Pilot Street do not increase sufficiently during the 100 year event to cause flow to enter the proposed overland flow path to the ocean. Due to road grading constraints, the level in the sag area downstream of King Street would have to increase by at least 100 mm in order for the overland flow path to come into use even for the 100 year event.

Given that the proposed drainage solution provides flood levels less than those which would occur if the overland flow path actually conveyed flow, it is considered that the drainage of the catchment is best served by a system which does not rely on the overland flow path. Although an overland flow path could be established to allow for discharge during extreme flood events, its value would need to be balanced against the increased potential for flooding during king tide/ storm surge events which would result from channel excavation.

Consideration could be given to reducing the pipe sizes adopted for the catchment and allowing more ponding to occur in the downstream part of the catchment. However, any reduction in pipe size would result in a worsening of flooding in the existing system upstream of King Street and a likely increase in water depths in the region between King Street and Charlton Esplanade. On balance, it was concluded that the proposed drainage works provided an acceptable compromise between pipe size and flood levels.

The velocity depth product at all locations for the major event was found to be 0.3 m²/s or less and therefore acceptable.

Overall, it was concluded that the proposed drainage system performs acceptably for both minor and major events.

4.6 Guard Street Catchment (D)

Drainage design standard: Q2, with Overland Flow Path Outlet at end of Pulgul Street

The following drainage system was found to be necessary to drain the Guard Street catchment:

- King St to Hibiscus St 1/450 Dia
- Hibiscus St to Larsen St 1/825 Dia
- Larsen St to Outlet 2/1,200 Dia

Adopted invert levels for the drainage line varied from RL 0.05 m AHD to RL 1.8 m AHD at King Street.

Minor Storm Event Performance

The proposed system was found to perform acceptably under minor flow conditions. Although the calculated water level in the underground system was within 150 mm of the surface at one point (NDS01/04A), it was considered that increasing the size of pipes downstream was not warranted.

Water depths in Guard Street for the minor event were found to be 155 mm or less and were therefore considered to be acceptable.

Despite the use of the 2 year design standard, the existing drainage line which runs from Shell Street beneath King Street, Hibiscus Street and Prince Street (LCU03/02, LDU02/03, LDU03/03, LDU07/01 and LDU08/01) was found to surcharge significantly under minor flow conditions. Water depths of up to 320 mm were obtained in the road network above the drainage line. Unfortunately, due to the location of the existing drainage line beneath developed properties and the lack of an existing overland flow path, it is unlikely that the construction of relief drainage works would be viable.

The existing drainage line in Pulgul Street performs acceptably under minor flow conditions except at the downstream end of the system at the intersection of Pulgul Street and Charlton Esplanade (NDS04/05) where surcharging occurs. This could be attributable in part to the limited information available with regard to the existing outlet in the area. A water depth of 520 mm was obtained for the minor event storm. As the area concerned was located some distance from the relief drainage works under investigation, the buildup of water at this point was not considered to be significant. Water depths in upstream areas of Pulgul Street were found to be acceptable.

At present, water surcharging from the system and overland flow ponds at the intersection before travelling along Charlton Esplanade and discharging via the Guard Street outlet. Enlargement of the outlet of the Pulgul Street system or the provision of an overland flow path to the ocean with a lower invert level than at present could minimise the extent of flooding which is predicted to occur.

Major Storm Event Performance

For major event flooding, the proposed Guard Street drainage system was found to surcharge over its entire length.

Maximum water depths of up to 1,070 mm (NDS01/05) were obtained in Guard Street for the major storm event. However, the effect of tailwater conditions needs to be borne in mind when considering this result. With a tailwater level of RL 2.4 m AHD for the major event, Guard Street is already flooded to depths of between 200 mm (NDS01/07) and 500 mm (NDS01/05). Ponding of water during the major event produces a relative increase in water depth of up to 570 mm. Therefore, it can be concluded that the effect of catchment runoff on flood levels along Guard Street is not as great as an initial review of water depths would suggest.

The enlargement of the proposed Guard Street drainage system would only provide an incremental benefit as the largest pipe sizes would still only provide flood depths approaching 500 mm. Further, increasing the pipe size would negate the benefit obtained in terms of reduced pipe size by adopting a 2 year rather than a 10 year design standard for the area.

Given the above, it is considered that the proposed drainage system provides an acceptable level of drainage for the major storm event and that the enlargement of the drainage system is not warranted.

Water depths of up to 620 mm were calculated for the existing drainage system between above Shell Street and Charlton Esplanade. As noted in the consideration of the performance of the system under the minor event, it is unlikely that relief drainage works in this area would be viable from a cost perspective.

If relief works were to be considered, it is likely that the Guard street system would need to be extended to collect water ponded in sags in King Street, Hibiscus Street, and Prince Street. In each case, a distance of around 200 metres would need to be piped together with the enlargement of the trunk drainage system on Guard Street.

Calculated water depths within Pulgul Street were found to be generally acceptable except at the intersection of Pulgul Street and Charlton Esplanade (NDS04/05). At this point, a water depth of 740 mm was obtained. As noted for the minor storm event performance, works to the existing overland flow path at this point could achieve a considerable reduction in water level.

The velocity depth product at all locations was found to be generally less than $0.6 \text{ m}^2/\text{s}$ and therefore acceptable. At one location (LDS01/02), a product of $1.5 \text{ m}^2/\text{s}$ was obtained. Due to the significant change in road elevation and short length of the channel concerned, additional pipe works would be unlikely to improve the situation at this point.

Overall, it was concluded that the proposed drainage system performs acceptably for both minor and major events.

4.7 Estimated Cost of Works

Based on the drainage system proposed for each catchment and its associated inlet works, an estimate of the cost of constructing the drainage system was made.

Construction costs were adopted based on the cost of similar works in the Hervey Bay area. The rates used in the costing are presented in **Table D1- Typical Construction Costs** in **Appendix D- Estimated Construction Costs**. The costs reflect typical construction costs for 2003.

The estimated cost of works is presented in **Table 4- Estimated Cost of Construction**.

It can be noted that the costs listed in Table 4 are estimates of the cost of construction only. The costing included an allowance of \$140,000 for the connection of the existing system to the new drainage system where necessary.

CDS Technologies was approached regarding the likely cost of gross pollutant traps. They provided the following preliminary estimates of cost:

▪ Elizabeth Street (A)	\$200,000
▪ Pier Street (B)	\$130,000
▪ Pilot Street (C)	\$110,000
▪ Guard Street (D)	\$130,000

The above figures are of a preliminary nature only and could only be confirmed following greater investigation. The costs are considered to be of the same order as other commercially available GPT units.

It can be noted that the costs presented in Table 4 do not include the cost of the road regrading works proposed as part of the drainage solution for the catchment.

TABLE 4
Estimated Cost of Construction

Catchment	Main Line	Inlet Pits + Minor Drainage Feeding to Main System	Gross Pollutant Traps	Total
Elizabeth Street (A)	\$570,000	\$210,000	\$200,000	\$980,000
Pier Street (B)	\$610,000	\$140,000	\$130,000	\$880,000
Pilot Street (C)	\$295,000	\$105,000	\$110,000	\$510,000
Guard Street (D)	\$385,000	\$130,000	\$130,000	\$645,000
<i>SUBTOTAL</i>	<i>\$1,860,000</i>	<i>\$585,000</i>	<i>\$570,000</i>	<i>\$3,015,000</i>
Contingencies (10%)	\$186,000	\$58,500	\$57,000	\$301,500
Services Contingency (10%)	\$186,000	\$58,500	\$57,000	\$301,500
TOTAL	\$2,232,000	\$702,000	\$684,000	\$3,618,000

5.0 CONCLUSION

A comprehensive XP-STORM hydraulic model of part of the Urangan Coastal strip was created to confirm the viability of relief drainage works proposed following a Concept Design Review of available drainage alternatives for the region.

The proposed drainage works consist of piped systems for Elizabeth Street, Pier Street, Pilot Street and Guard Street, together with some road regrading work to provide improved catchment definition. Proposed habitable floor levels are sufficiently high to allow for the storage of runoff during major storm events without causing the inundation of habitable areas.

The analysis involved the iterative derivation of pipe sizes suitable under specified Minor and Major event flooding conditions and the determination of fill levels for the catchment.

KEY RESULTS OF STUDY:

The recommended drainage works and floor levels for the catchment are presented in:

Figure 2- Pilot Street - 20 Year

Figure 3- Pilot Street - 50 Year

Figure 4- Pilot Street - 100 Year

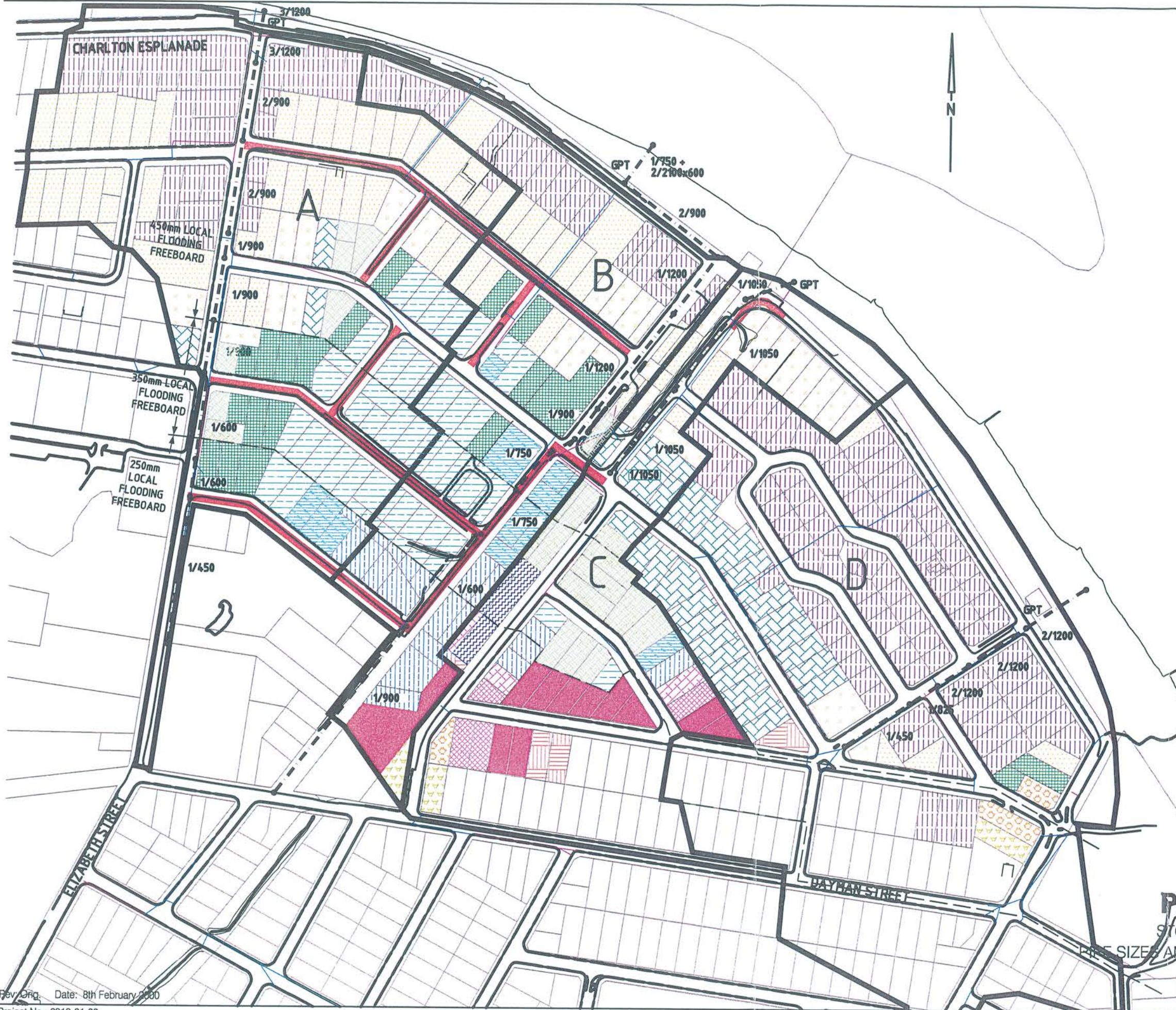
*The proposed relief drainage works are tabulated in **Table B5- Stormwater Drainage Details in Appendix B- Hydraulic Data**. This table also lists the pipes proposed between each gully pit and the main drainage lines.*

*The invert levels adopted for each drainage line are presented in **Table B6- Adopted Invert Levels for Underground Nodes in Appendix B**. The adopted number and size of inlet pits are listed in **Table B10- Adopted Inlet Pits in Appendix B**.*

*The calculated flood levels for the catchment are summarised in **Table C1- Surface Network- Peak Water Levels and Depths in Appendix C- Model Results**.*

6.0 REFERENCES

- Argue, J.R. 1986, *Storm Drainage Design in Small Urban Catchments: A Handbook for Australian Practice*. Australian Road Research Board. Special Report SR34.
- Brisbane City Council. 1994, *Brisbane City Council Supplement to the Queensland Urban Drainage Manual*, July.
- Cardno & Davies. 1999, *Minutes of Meeting with Hervey Bay City Council*, 11 March.
- Cardno & Davies. 1998a, *Western Creek, Milton, Relief Drainage Investigation, Volume 1- Report Text*, Report completed for Brisbane City Council, August.
- Cardno & Davies. 1998b, *Letter to Hervey Bay City Council*, 10 September.
- CSR-Humes. 1996, *Humeceptor Design Manual*.
- Gutteridge Haskins & Davey. 1996, *Drainage Strategies*, report prepared for Hervey Bay City Council.
- Hervey Bay City Council. 1998, *Statement of Proposals, Hervey Bay Planning Scheme Amendment*, August.
- Hervey Bay City Council. 1997, *Hervey Bay City Council Development Manual Volume 1*, July.
- Institution of Engineers Australia. 1987, *Australian Rainfall and Runoff, A Guide to Flood Estimation, Volumes 1 and 2*.
- John Wilson & Partners. 1998, *Toogoom Area Drainage Study*, Report for Hervey Bay City Council, November.
- Lavey, N. 2000, Personal communication to Cardno MBK, January.
- Neville Jones & Associates and Australian Water Engineering. 1992, *Queensland Urban Drainage Manual, Volume 1: Text*, Report prepared for Water Resources Commission, The Local Government Engineers' Association of Queensland, and Brisbane City Council, Edition 1, September.
- O'Loughlin, G. 1988, *ILSAX Users Manual*, University of Wollongong.
- Rocla. 1998, *Cleansall Manual*.
- XP Software. 1999, Personal Communication with Cardno & Davies, April.
- XP Software. 2002, *XP-SWMM User Manual*, (Supplied in Adobe Acrobat Reader format on Installation CD), October.



ALLOTMENT HABITABLE FLOOR LEVELS

LEVEL	COLOUR
R.L. 3.800	[Pattern]
R.L. 3.950	[Pattern]
R.L. 4.100	[Pattern]
R.L. 4.250	[Pattern]
R.L. 4.400	[Pattern]
R.L. 4.600	[Pattern]
R.L. 4.800	[Pattern]
R.L. 5.000	[Pattern]
R.L. 5.200	[Pattern]
R.L. 5.400	[Pattern]
R.L. 5.600	[Pattern]
R.L. 5.800	[Pattern]
R.L. 6.000	[Pattern]
R.L. 6.200	[Pattern]
R.L. 6.500	[Pattern]
R.L. 6.800	[Pattern]
Exist. levels satisfactory	[Pattern]

CATCHMENT TABLE

CATCHMENT	AREA
A	14.921 Ha.
B	14.713 Ha.
C	10.485 Ha.
D	22.957 Ha.
TOTAL	63.076 Ha.

LEGEND

[Symbol]	EXISTING STORMWATER DRAINAGE
[Symbol]	PROPOSED TRUNK DRAINAGE
[Symbol]	ROADS TO BE REGRADED TO DEFINE SUB-CATCHMENT BOUNDARIES

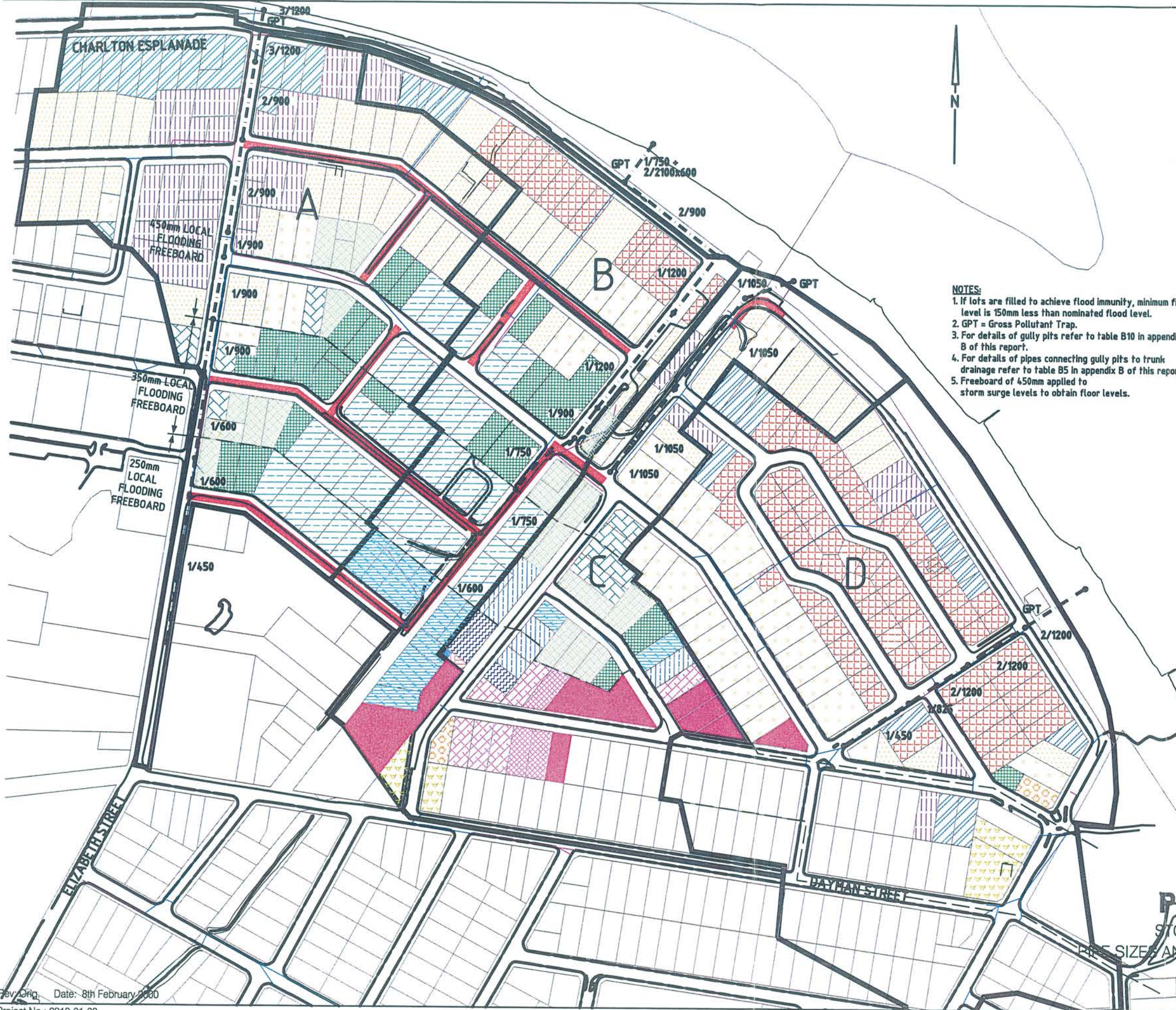
NOTES:

1. If lots are filled to achieve flood immunity, minimum fill level is 150mm less than nominated flood level.
2. GPT = Gross Pollutant Trap.
3. For details of gully pits refer to table B10 in appendix B of this report.
4. For details of pipes connecting gully pits to trunk drainage refer to table B5 in appendix B of this report.
5. Freeboard of 450mm applied to storm surge levels to obtain flood levels.

Scale 1:4000

PILOT STREET - 50 YEAR
STORMWATER TRUNK DRAINAGE STRATEGY
PIPE SIZES AND FILL LEVEL - ULTIMATE DEVELOPMENT

FIGURE 3



ALLOTMENT HABITABLE FLOOR LEVELS

LEVEL	COLOUR
R.L. 3.400	[Pattern]
R.L. 3.650	[Pattern]
R.L. 3.800	[Pattern]
R.L. 3.950	[Pattern]
R.L. 4.100	[Pattern]
R.L. 4.250	[Pattern]
R.L. 4.400	[Pattern]
R.L. 4.600	[Pattern]
R.L. 4.800	[Pattern]
R.L. 5.000	[Pattern]
R.L. 5.200	[Pattern]
R.L. 5.400	[Pattern]
R.L. 5.600	[Pattern]
R.L. 5.800	[Pattern]
R.L. 6.000	[Pattern]
R.L. 6.200	[Pattern]
R.L. 6.500	[Pattern]
R.L. 6.800	[Pattern]
Exist. levels satisfactory	[Pattern]

- NOTES:**
1. If lots are filled to achieve flood immunity, minimum fill level is 150mm less than nominated flood level.
 2. GPT = Gross Pollutant Trap.
 3. For details of gully pits refer to table B10 in appendix B of this report.
 4. For details of pipes connecting gully pits to trunk drainage refer to table B5 in appendix B of this report.
 5. Freeboard of 450mm applied to storm surge levels to obtain floor levels.

CATCHMENT TABLE

CATCHMENT	AREA
A	14.921 Ha.
B	14.713 Ha.
C	10.485 Ha.
D	22.957 Ha.
TOTAL	63.076 Ha.

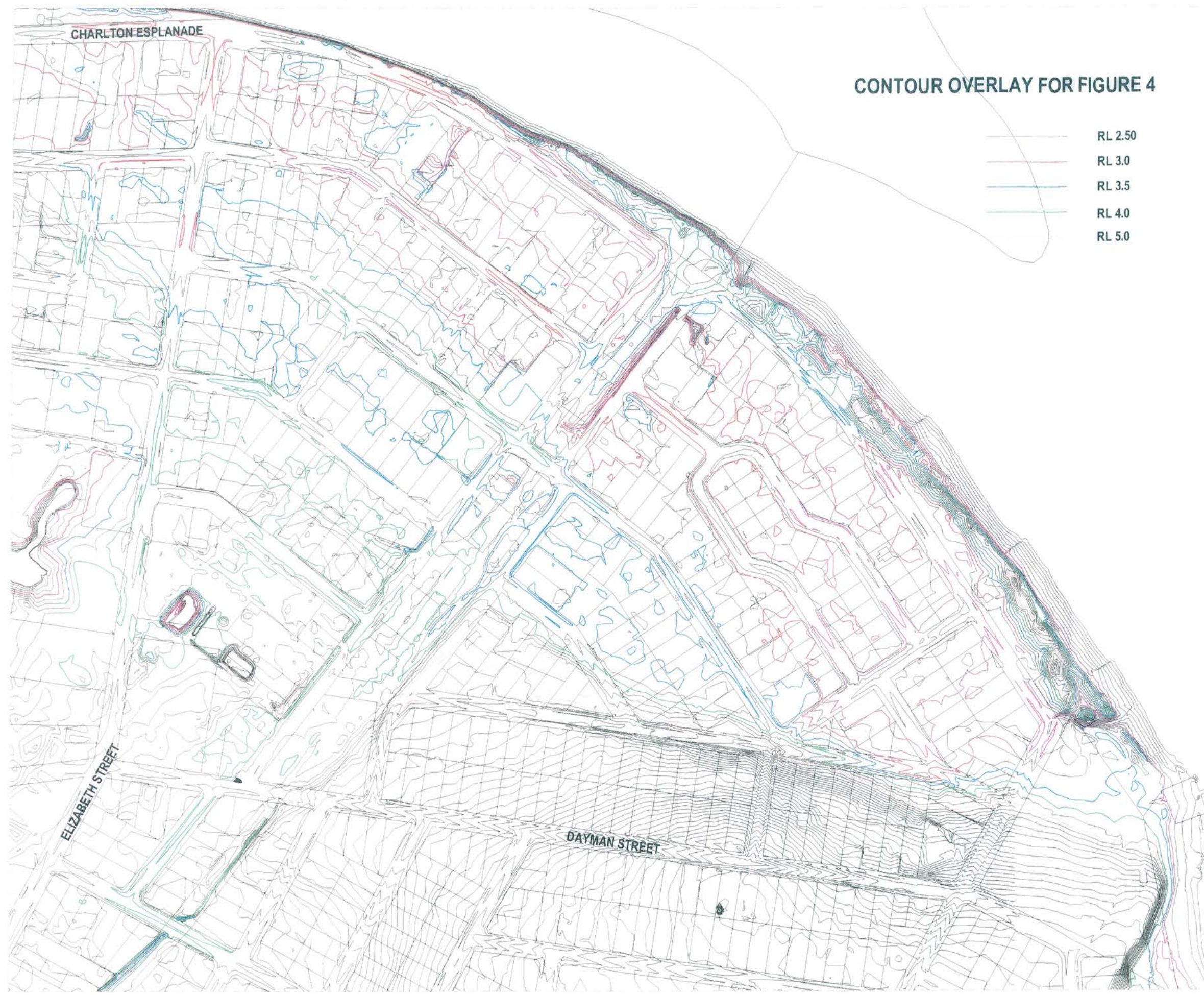
LEGEND

- EXISTING STORMWATER DRAINAGE
- PROPOSED TRUNK DRAINAGE
- ROADS TO BE REGRADED TO DEFINE SUB-CATCHMENT BOUNDARIES

Scale 1:4000

PILOT STREET - 20 YEAR
STORMWATER TRUNK DRAINAGE STRATEGY
PIPE SIZES AND FILL LEVEL - ULTIMATE DEVELOPMENT

FIGURE 2



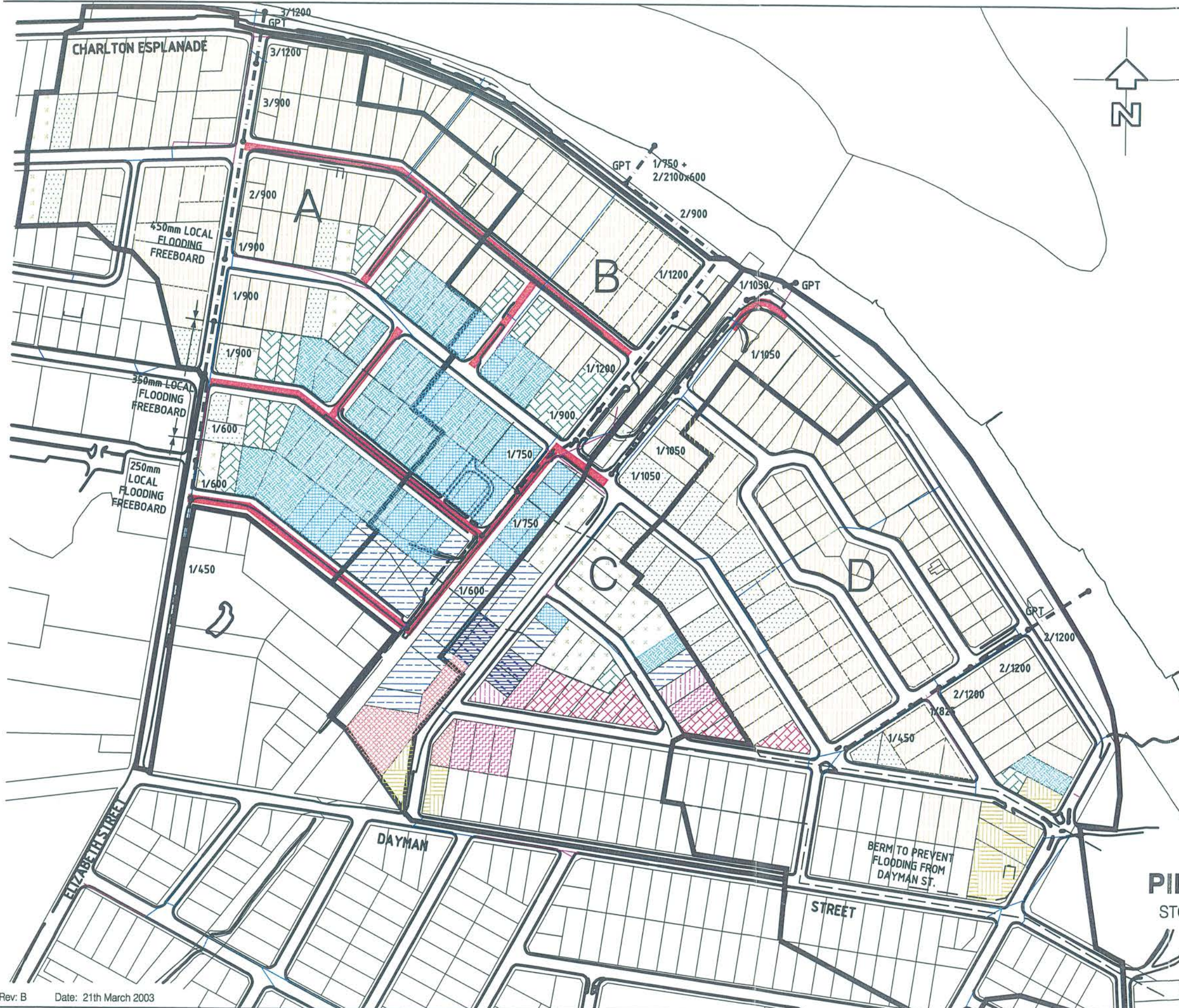
CONTOUR OVERLAY FOR FIGURE 4

- RL 2.50
- RL 3.0
- RL 3.5
- RL 4.0
- RL 5.0

CHARLTON ESPLANADE

ELIZABETH STREET

DAYMAN STREET



ALLOTMENT HABITABLE FLOOR LEVELS

LEVEL	COLOUR
R.L. 4.150	[Pattern]
R.L. 4.300	[Pattern]
R.L. 4.450	[Pattern]
R.L. 4.600	[Pattern]
R.L. 4.800	[Pattern]
R.L. 5.000	[Pattern]
R.L. 5.200	[Pattern]
R.L. 5.400	[Pattern]
R.L. 5.600	[Pattern]
R.L. 5.800	[Pattern]
R.L. 6.000	[Pattern]
R.L. 6.200	[Pattern]
R.L. 6.500	[Pattern]
R.L. 6.800	[Pattern]
Exist. levels satisfactory	[Pattern]

CATCHMENT TABLE

CATCHMENT	AREA
A	14.921 Ha.
B	14.713 Ha.
C	10.485 Ha.
D	22.957 Ha.
TOTAL	63.076 Ha.

LEGEND

- EXISTING STORMWATER DRAINAGE
- PROPOSED TRUNK DRAINAGE
- ROADS TO BE REGRADED TO DEFINE SUB-CATCHMENT BOUNDARIES

- NOTES:**
- If lots are filled to achieve flood immunity, minimum fill level is 150mm less than nominated flood level.
 - GPT = Gross Pollutant Trap.
 - For details of gully pits refer to table B10 in appendix B of this report.
 - For details of pipes connecting gully pits to trunk drainage refer to table B5 in appendix B of this report.
 - Freeboard of 450mm applied to storm surge levels to obtain floor levels.

Scale 1:4000

PILOT STREET - 100 YEAR
STORMWATER TRUNK DRAINAGE STRATEGY
PIPE SIZES AND FLOOR LEVEL
ULTIMATE DEVELOPMENT
FIGURE 4

Hydrologic Data**List of Tables**

Table A1	Catchment Areas, Weighted Runoff Coefficients and Times of Concentration	A1-A3
Table A2	Peak Flows- Minor Events	A4-A5
Table A3	Peak Flows- Major Events	A6-A7

List of Figures

Figure A1	Catchment Plan
Figure A2	Zoning Plan

TABLE A1
Catchment Areas, Weighted Runoff Coefficients and Times of Concentration

Adopted Basic Runoff Coefficients																
C_2	0.701	0.701	0.723	0.723	0.765	0.748	0.561									
C_{10}	0.85	0.825	0.85	0.85	0.9	0.88	0.66									
C_{20}	0.893	0.866	0.893	0.893	0.945	0.924	0.693									
C_{50}	0.978	0.949	0.978	0.978	1	1	0.759									
C_{100}	1	0.99	1	1	1	1	0.792									
Node	Inlet Time (Min)	Total Area (ha)	Area Components							Weighted C Values						
			Road (ha)	Res. Low Density (ha)	Res. Med. Density (ha)	Foreshore Med. Den. (ha)	Local Shopping (ha)	Village Centre (ha)	Beach (ha)	2 Year Event	10 Year Event	20 Year Event	50 Year Event	100 Year Event		
A- Elizabeth Street																
NAS01/02	5	0.308	0.308		0							0.7	0.85	0.89	0.98	1.00
NAS01/04	15	0.602	0.234			0.368						0.71	0.85	0.89	0.98	1.00
NAS01/05	15	0.267	0.084			0.083	0.1					0.73	0.87	0.91	0.99	1.00
NAS01/06	15	0.549	0.168			0.381						0.72	0.85	0.89	0.98	1.00
NAS01/07	15	0.338	0.102			0.236						0.72	0.85	0.89	0.98	1.00
NAS01/08	15	1.341	0.282		0.05	1.009						0.72	0.85	0.89	0.98	1.00
NAS01/09	15	0.8	0.22			0.58						0.72	0.85	0.89	0.98	1.00
NAS01/10	15	0.739	0.256			0.281			0.202			0.72	0.86	0.91	0.98	1.00
NAS01/11	15	0.74	0.216		0.328			0.094	0.102			0.73	0.86	0.9	0.98	1.00
NAS01/12	15	0.632	0.237						0.395			0.73	0.87	0.91	0.99	1.00
NAS02/02	12.3	0.716	0.21			0.506						0.72	0.85	0.89	0.98	1.00
NAS03/02	10.4	1.278	0.268			1.01						0.72	0.85	0.89	0.98	1.00
NAS03/03	15	0.748	0.174			0.574						0.72	0.85	0.89	0.98	1.00
NAS05/02	15	0.432	0.078			0.354						0.72	0.85	0.89	0.98	1.00
NAS05/03	15	0.307	0.105			0.202						0.72	0.85	0.89	0.98	1.00
NAS05/04	15	0.608	0.204			0.404						0.72	0.85	0.89	0.98	1.00
NAS05/05	15	0.438	0.14			0.298						0.72	0.85	0.89	0.98	1.00
NAS05/06	15	0.344	0.04		0.051	0.253						0.72	0.85	0.89	0.98	1.00
NAS07/02	15	0.763	0.232			0.216	0.315					0.73	0.87	0.91	0.99	1.00
NAS07/03	15	1.22	0.203			0.416	0.501	0.1				0.74	0.87	0.92	0.99	1.00
NAS07/01	14	0.431	0.077			0.199	0.155					0.73	0.87	0.91	0.99	1.00
NAS08/02	15	0.923	0.298			0.625						0.72	0.85	0.89	0.98	1.00
NAS08/03	15	0.397	0.172			0.225						0.71	0.85	0.89	0.98	1.00
NAS12/02	15	2.432	0.345		1.217	0.64				0.23		0.7	0.83	0.87	0.96	1.00

Node	Inlet Time (Min)	Total Area (ha)	Area Components							Weighted C Values					
			Road (ha)	Res. Low Density (ha)	Res. Med. Density (ha)	Foreshore Med. Den. (ha)	Local Shopping (ha)	Village Centre (ha)	Beach (ha)	2 Year Event	10 Year Event	20 Year Event	50 Year Event	100 Year Event	
B- Pier Street															
NBS01/04	15	2.01	0.527		1.079	0.404					0.72	0.85	0.89	0.98	1.00
NBS01/05	15	1.854	0.528		0.719	0.607					0.72	0.85	0.89	0.98	1.00
NBS01/07	15	1.155	0.356		0.293	0.304	0.202				0.72	0.86	0.9	0.98	1.00
NBS01/08	15	0.494	0.143		0.09		0.261				0.74	0.88	0.92	0.99	1.00
NBS01/09	13	0.676	0.126		0.107		0.443				0.75	0.88	0.93	0.99	1.00
NBS01/10	15	0.28	0.121		0.108		0.051				0.72	0.86	0.9	0.98	1.00
NBS01/11	15	0.451	0.182		0.103		0.166				0.73	0.87	0.91	0.99	1.00
NBS01/12	15	1.775	0.441				0.552	0.317	0.465		0.69	0.82	0.86	0.93	0.95
NBS02/01	11.2	0.197	0.045			0.152					0.72	0.85	0.89	0.98	1.00
NBS04/02	15	1.673	0.361			0.302	0.708	0.302			0.74	0.88	0.92	0.99	1.00
NBS05/01	15	0.335	0.134		0.101		0.1				0.73	0.86	0.91	0.98	1.00
NBS06/01	15	0.651	0.231						0.42		0.73	0.87	0.91	0.99	1.00
NBS06/02	12.4	0.73	0.225				0.167	0.338			0.74	0.88	0.92	0.99	1.00
C- Pilot Street															
NCS01/01	15	0.301	0.271		0.03						0.7	0.85	0.89	0.98	1.00
NCS01/02	9.8	2.26	0.548		1.427	0.285					0.72	0.85	0.89	0.98	1.00
NCS01/03	13.2	0.626	0.346		0.043	0.237					0.71	0.85	0.89	0.98	1.00
NCS01/04	15	1.173	0.454		0.028	0.691					0.71	0.85	0.89	0.98	1.00
NCS01/05	15	0.432	0.162		0.027	0.243					0.71	0.85	0.89	0.98	1.00
NCS01/06	15	0.603	0.296		0.039	0.268					0.71	0.85	0.89	0.98	1.00
NCS01/08	15	0.292	0.095		0.035	0.162					0.72	0.85	0.89	0.98	1.00
NCS01/09	15	0.59	0.192	0.281	0.117						0.71	0.84	0.88	0.96	1.00
NCS01/10	14	0.585	0.24	0.121	0.224						0.71	0.84	0.89	0.97	1.00
NCS03/01	13.4	1.385	0.218		1.095	0.072					0.72	0.85	0.89	0.98	1.00
NCS03/02	15	1.013	0.193			0.82					0.72	0.85	0.89	0.98	1.00
NCS04/02	15	1.225	0.355	0.776					0.094		0.69	0.82	0.86	0.94	0.98
D- Guard Street															
NDS01/02	10	1.759	0.528		1.231						0.72	0.85	0.89	0.98	1.00
NDS01/04	15	1.254	0.479	0.614		0.161					0.7	0.84	0.88	0.96	1.00
NDS01/05	15	1.058	0.328	0.054		0.676					0.72	0.85	0.89	0.98	1.00
NDS01/06	12.8	0.863	0.291	0.572							0.7	0.83	0.88	0.96	0.99
NDS01/07	15	1.522	0.371	0.406					0.745		0.63	0.75	0.79	0.86	0.90
NDS02/01	13	0.184	0.103			0.081					0.71	0.85	0.89	0.98	1.00
NDS02/02	15	1.28	0.249			1.031					0.72	0.85	0.89	0.98	1.00

Node	Inlet Time (Min)	Total Area (ha)	Area Components							Weighted C Values						
			Road (ha)	Res. Low Density (ha)	Res. Med. Density (ha)	Foreshore Med. Den. (ha)	Local Shopping (ha)	Village Centre (ha)	Beach (ha)	2 Year Event	10 Year Event	20 Year Event	50 Year Event	100 Year Event		
NDS02/04	15	1.835	0.374		0.202	1.259						0.72	0.85	0.89	0.98	1.00
NDS03/01	13	0.068	0.068									0.7	0.85	0.89	0.98	1.00
NDS03/02	11	0.822	0.14	0.285	0.397							0.71	0.84	0.88	0.97	1.00
NDS03/03	15	1.044	0.363	0.453		0.228						0.71	0.84	0.88	0.97	1.00
NDS04/01	13	0.994	0.27		0.724							0.72	0.85	0.89	0.98	1.00
NDS04/02	13	0.226	0.205		0.021							0.7	0.85	0.89	0.98	1.00
NDS04/03	13	0.179	0.179									0.7	0.85	0.89	0.98	1.00
NDS04/04	13	0.691	0.112							0.579		0.58	0.69	0.73	0.79	0.83
NDS04/05	15	1.33	0.285	0.298				0.207		0.54		0.65	0.78	0.81	0.89	0.91
NDS05/01	13	2.18	0.576	1.459				0.145				0.71	0.84	0.88	0.96	0.99
NDS06/02	14.1	1.32	0.186		1.003	0.131						0.72	0.85	0.89	0.98	1.00
NDS06/03	10	1.52	0.283		0.849			0.388				0.73	0.86	0.91	0.98	1.00
NDS07/01	15	1.598	0.455	0.712	0.431							0.71	0.84	0.88	0.97	1.00
NDS08/01	15	1.23	0.33	0.643						0.257		0.67	0.8	0.84	0.92	0.95
TOTAL		63.076	17.618	6.674	12.169	16.974	4.555	2.176	2.91							

TABLE A2
Peak Flows- Minor Events

Node Name	2 Year Event								10 Year Event							
	15 Min	20 Min	25 Min	30 Min	45 Min	1 Hour	90 Min	2 Hour	15 Min	20 Min	25 Min	30 Min	45 Min	1 Hour	90 Min	2 Hour
	Storm	Storm	Storm	Storm	Storm	Storm	Storm	Storm	Storm	Storm	Storm	Storm	Storm	Storm	Storm	Storm
	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)
A- Elizabeth Street																
NAS01/02	0.057	0.050	0.044	0.040	0.033	0.028	0.021	0.018	0.098	0.085	0.076	0.069	0.056	0.047	0.037	0.031
NAS01/04	0.113	0.098	0.088	0.080	0.064	0.055	0.042	0.035	0.191	0.167	0.149	0.135	0.109	0.092	0.072	0.060
NAS01/05	0.051	0.045	0.040	0.037	0.029	0.025	0.019	0.016	0.087	0.076	0.068	0.061	0.049	0.042	0.033	0.027
NAS01/06	0.104	0.091	0.081	0.074	0.060	0.051	0.039	0.033	0.174	0.152	0.136	0.124	0.099	0.084	0.066	0.055
NAS01/07	0.064	0.056	0.050	0.046	0.037	0.031	0.024	0.020	0.107	0.093	0.084	0.076	0.061	0.052	0.041	0.034
NAS01/08	0.254	0.222	0.199	0.181	0.146	0.124	0.096	0.079	0.425	0.371	0.332	0.302	0.242	0.206	0.161	0.134
NAS01/09	0.152	0.133	0.119	0.108	0.087	0.074	0.057	0.047	0.254	0.221	0.198	0.180	0.144	0.123	0.096	0.080
NAS01/10	0.140	0.122	0.110	0.100	0.080	0.068	0.053	0.044	0.237	0.207	0.185	0.168	0.135	0.115	0.090	0.075
NAS01/11	0.142	0.124	0.111	0.101	0.081	0.069	0.054	0.044	0.237	0.207	0.185	0.168	0.135	0.115	0.090	0.075
NAS01/12	0.122	0.106	0.095	0.087	0.070	0.059	0.046	0.038	0.205	0.179	0.160	0.146	0.117	0.099	0.078	0.065
NAS02/02	0.136	0.119	0.106	0.097	0.078	0.066	0.051	0.042	0.227	0.198	0.177	0.161	0.129	0.110	0.086	0.072
NAS03/02	0.242	0.212	0.189	0.173	0.139	0.118	0.091	0.076	0.405	0.353	0.316	0.288	0.231	0.196	0.153	0.128
NAS03/03	0.142	0.124	0.111	0.101	0.081	0.069	0.053	0.044	0.237	0.207	0.185	0.168	0.135	0.115	0.090	0.075
NAS05/02	0.082	0.072	0.064	0.058	0.047	0.040	0.031	0.026	0.137	0.119	0.107	0.097	0.078	0.066	0.052	0.043
NAS05/03	0.058	0.051	0.046	0.041	0.033	0.028	0.022	0.018	0.097	0.085	0.076	0.069	0.055	0.047	0.037	0.031
NAS05/04	0.115	0.101	0.090	0.082	0.066	0.056	0.043	0.036	0.193	0.168	0.150	0.137	0.110	0.093	0.073	0.061
NAS05/05	0.083	0.073	0.065	0.059	0.048	0.041	0.031	0.026	0.139	0.121	0.108	0.099	0.079	0.067	0.052	0.044
NAS05/06	0.065	0.057	0.051	0.046	0.037	0.032	0.025	0.020	0.109	0.095	0.085	0.077	0.062	0.053	0.041	0.034
NAS07/02	0.147	0.128	0.115	0.104	0.084	0.072	0.055	0.046	0.248	0.216	0.193	0.176	0.141	0.120	0.094	0.078
NAS07/03	0.238	0.208	0.186	0.169	0.136	0.116	0.090	0.074	0.396	0.345	0.309	0.281	0.226	0.192	0.150	0.125
NAS07/01	0.083	0.072	0.065	0.059	0.047	0.040	0.031	0.026	0.140	0.122	0.109	0.099	0.080	0.068	0.053	0.044
NAS08/02	0.175	0.153	0.137	0.125	0.100	0.085	0.066	0.055	0.293	0.255	0.228	0.208	0.167	0.142	0.111	0.092
NAS08/03	0.074	0.065	0.058	0.053	0.043	0.036	0.028	0.023	0.126	0.110	0.098	0.089	0.072	0.061	0.048	0.040
NAS12/02	0.449	0.392	0.351	0.319	0.257	0.219	0.169	0.140	0.753	0.657	0.587	0.534	0.429	0.365	0.285	0.238
B- Pier Street																
NBS01/04	0.381	0.333	0.298	0.271	0.218	0.186	0.144	0.119	0.638	0.556	0.497	0.452	0.363	0.309	0.241	0.201
NBS01/05	0.352	0.307	0.275	0.250	0.201	0.171	0.132	0.110	0.588	0.513	0.459	0.417	0.335	0.285	0.222	0.186
NBS01/07	0.219	0.191	0.171	0.156	0.125	0.107	0.082	0.068	0.371	0.323	0.289	0.263	0.211	0.179	0.140	0.117
NBS01/08	0.096	0.084	0.075	0.069	0.055	0.047	0.036	0.030	0.162	0.141	0.126	0.115	0.092	0.079	0.061	0.051
NBS01/09	0.134	0.117	0.104	0.095	0.076	0.065	0.050	0.042	0.222	0.194	0.173	0.157	0.126	0.107	0.084	0.070
NBS01/10	0.053	0.046	0.042	0.038	0.030	0.026	0.020	0.017	0.090	0.078	0.070	0.064	0.051	0.043	0.034	0.028
NBS01/11	0.087	0.076	0.068	0.062	0.050	0.042	0.033	0.027	0.146	0.128	0.114	0.104	0.083	0.071	0.055	0.046
NBS01/12	0.323	0.282	0.252	0.230	0.185	0.157	0.121	0.101	0.543	0.474	0.424	0.385	0.309	0.263	0.205	0.171
NBS02/01	0.037	0.033	0.029	0.027	0.021	0.018	0.014	0.012	0.062	0.054	0.049	0.044	0.036	0.030	0.024	0.020
NBS04/02	0.326	0.285	0.255	0.232	0.187	0.159	0.123	0.102	0.549	0.479	0.428	0.390	0.313	0.266	0.208	0.173
NBS05/01	0.064	0.056	0.050	0.046	0.037	0.031	0.024	0.020	0.108	0.094	0.084	0.076	0.061	0.052	0.041	0.034

Node Name	2 Year Event								10 Year Event							
	15 Min Storm (m³/s)	20 Min Storm (m³/s)	25 Min Storm (m³/s)	30 Min Storm (m³/s)	45 Min Storm (m³/s)	1 Hour Storm (m³/s)	90 Min Storm (m³/s)	2 Hour Storm (m³/s)	15 Min Storm (m³/s)	20 Min Storm (m³/s)	25 Min Storm (m³/s)	30 Min Storm (m³/s)	45 Min Storm (m³/s)	1 Hour Storm (m³/s)	90 Min Storm (m³/s)	2 Hour Storm (m³/s)
	NBS06/01	0.125	0.109	0.098	0.089	0.072	0.061	0.047	0.039	0.211	0.184	0.165	0.150	0.120	0.102	0.080
NBS06/02	0.142	0.124	0.111	0.101	0.081	0.069	0.054	0.044	0.240	0.209	0.187	0.170	0.136	0.116	0.091	0.076
C- Pilot Street																
NCS01/01	0.056	0.048	0.043	0.040	0.032	0.027	0.021	0.017	0.095	0.083	0.074	0.068	0.054	0.046	0.036	0.030
NCS01/02	0.429	0.374	0.335	0.305	0.245	0.209	0.161	0.134	0.717	0.625	0.559	0.509	0.408	0.347	0.271	0.226
NCS01/03	0.117	0.102	0.092	0.083	0.067	0.057	0.044	0.037	0.199	0.173	0.155	0.141	0.113	0.096	0.075	0.063
NCS01/04	0.219	0.192	0.172	0.156	0.126	0.107	0.083	0.068	0.372	0.324	0.290	0.264	0.212	0.180	0.141	0.117
NCS01/05	0.081	0.071	0.063	0.058	0.046	0.039	0.030	0.025	0.137	0.119	0.107	0.097	0.078	0.066	0.052	0.043
NCS01/06	0.113	0.098	0.088	0.080	0.065	0.055	0.042	0.035	0.191	0.167	0.149	0.136	0.109	0.093	0.072	0.060
NCS01/08	0.055	0.048	0.043	0.039	0.032	0.027	0.021	0.017	0.093	0.081	0.072	0.066	0.053	0.045	0.035	0.029
NCS01/09	0.110	0.096	0.086	0.079	0.063	0.054	0.042	0.034	0.185	0.161	0.144	0.131	0.105	0.090	0.070	0.058
NCS01/10	0.109	0.096	0.086	0.078	0.063	0.053	0.041	0.034	0.183	0.160	0.143	0.130	0.104	0.089	0.069	0.058
NCS03/01	0.263	0.229	0.205	0.187	0.150	0.128	0.099	0.082	0.439	0.383	0.343	0.312	0.250	0.213	0.166	0.139
NCS03/02	0.192	0.168	0.150	0.137	0.110	0.094	0.072	0.060	0.321	0.280	0.251	0.228	0.183	0.156	0.121	0.101
NCS04/02	0.223	0.194	0.174	0.159	0.127	0.109	0.084	0.070	0.375	0.327	0.292	0.266	0.213	0.181	0.142	0.118
D- Guard Street																
NDS01/02	0.334	0.291	0.261	0.238	0.191	0.163	0.126	0.104	0.558	0.487	0.435	0.396	0.318	0.270	0.211	0.176
NDS01/04	0.231	0.202	0.181	0.165	0.132	0.113	0.087	0.072	0.393	0.343	0.307	0.279	0.224	0.190	0.148	0.124
NDS01/05	0.201	0.175	0.157	0.143	0.115	0.098	0.076	0.063	0.336	0.293	0.262	0.238	0.191	0.162	0.127	0.106
NDS01/06	0.159	0.139	0.124	0.113	0.091	0.078	0.060	0.050	0.267	0.233	0.208	0.190	0.152	0.129	0.101	0.084
NDS01/07	0.253	0.221	0.197	0.180	0.145	0.123	0.095	0.079	0.426	0.371	0.332	0.302	0.243	0.206	0.161	0.134
NDS02/01	0.034	0.030	0.027	0.025	0.020	0.017	0.013	0.011	0.058	0.051	0.046	0.041	0.033	0.028	0.022	0.018
NDS02/02	0.243	0.212	0.190	0.173	0.139	0.118	0.091	0.076	0.406	0.354	0.317	0.288	0.231	0.197	0.153	0.128
NDS02/04	0.348	0.304	0.272	0.248	0.199	0.170	0.131	0.109	0.582	0.508	0.454	0.413	0.331	0.282	0.220	0.184
NDS03/01	0.013	0.011	0.010	0.009	0.007	0.006	0.005	0.004	0.022	0.019	0.017	0.015	0.012	0.010	0.008	0.007
NDS03/02	0.154	0.134	0.120	0.109	0.088	0.075	0.058	0.048	0.258	0.225	0.201	0.183	0.147	0.125	0.097	0.081
NDS03/03	0.195	0.171	0.153	0.139	0.112	0.095	0.074	0.061	0.327	0.285	0.255	0.232	0.186	0.158	0.124	0.103
NDS04/01	0.189	0.165	0.147	0.134	0.108	0.092	0.071	0.059	0.315	0.275	0.246	0.224	0.180	0.153	0.119	0.100
NDS04/02	0.042	0.036	0.033	0.030	0.024	0.020	0.016	0.013	0.072	0.063	0.056	0.051	0.041	0.035	0.027	0.023
NDS04/03	0.033	0.029	0.026	0.023	0.019	0.016	0.012	0.010	0.057	0.050	0.044	0.040	0.032	0.027	0.021	0.018
NDS04/04	0.106	0.092	0.083	0.075	0.060	0.051	0.040	0.033	0.178	0.155	0.139	0.126	0.101	0.086	0.067	0.056
NDS04/05	0.228	0.199	0.178	0.162	0.130	0.111	0.086	0.071	0.387	0.338	0.302	0.275	0.220	0.187	0.146	0.122
NDS05/01	0.408	0.356	0.319	0.290	0.233	0.199	0.154	0.127	0.683	0.596	0.533	0.485	0.389	0.331	0.258	0.216
NDS06/02	0.250	0.219	0.196	0.178	0.143	0.122	0.094	0.078	0.419	0.365	0.326	0.297	0.238	0.203	0.158	0.132
NDS06/03	0.292	0.255	0.229	0.208	0.167	0.143	0.110	0.091	0.488	0.425	0.380	0.346	0.278	0.236	0.184	0.154
NDS07/01	0.299	0.261	0.234	0.213	0.171	0.146	0.113	0.093	0.501	0.437	0.391	0.355	0.285	0.242	0.189	0.158
NDS08/01	0.217	0.190	0.170	0.155	0.124	0.106	0.082	0.068	0.367	0.320	0.286	0.260	0.209	0.178	0.139	0.116

TABLE A3
Peak Flows- Major Events

Node Name	20 Year Event								50 Year Event								100 Year Event							
	15 Min Storm (m³/s)	20 Min Storm (m³/s)	25 Min Storm (m³/s)	30 Min Storm (m³/s)	45 Min Storm (m³/s)	1 Hour Storm (m³/s)	90 Min Storm (m³/s)	2 Hour Storm (m³/s)	15 Min Storm (m³/s)	20 Min Storm (m³/s)	25 Min Storm (m³/s)	30 Min Storm (m³/s)	45 Min Storm (m³/s)	1 Hour Storm (m³/s)	90 Min Storm (m³/s)	2 Hour Storm (m³/s)	15 Min Storm (m³/s)	20 Min Storm (m³/s)	25 Min Storm (m³/s)	30 Min Storm (m³/s)	45 Min Storm (m³/s)	1 Hour Storm (m³/s)	90 Min Storm (m³/s)	2 Hour Storm (m³/s)
A- Elizabeth Street																								
NAS01/02	0.117	0.102	0.091	0.083	0.067	0.057	0.044	0.037	0.151	0.132	0.118	0.107	0.086	0.073	0.057	0.048	0.171	0.149	0.133	0.121	0.097	0.082	0.065	0.055
NAS01/04	0.229	0.200	0.179	0.163	0.130	0.111	0.087	0.073	0.295	0.257	0.230	0.209	0.168	0.142	0.112	0.094	0.335	0.292	0.261	0.237	0.190	0.161	0.127	0.107
NAS01/05	0.104	0.091	0.081	0.074	0.059	0.050	0.039	0.033	0.132	0.115	0.103	0.094	0.075	0.064	0.050	0.042	0.149	0.129	0.116	0.105	0.084	0.071	0.056	0.047
NAS01/06	0.209	0.182	0.163	0.148	0.119	0.101	0.079	0.066	0.269	0.235	0.210	0.191	0.153	0.130	0.102	0.086	0.306	0.266	0.238	0.216	0.173	0.147	0.116	0.097
NAS01/07	0.129	0.112	0.100	0.091	0.073	0.062	0.049	0.041	0.166	0.145	0.129	0.117	0.094	0.080	0.063	0.053	0.188	0.164	0.146	0.133	0.107	0.090	0.071	0.060
NAS01/08	0.511	0.445	0.398	0.362	0.290	0.247	0.193	0.162	0.658	0.573	0.512	0.466	0.373	0.317	0.249	0.209	0.747	0.650	0.581	0.528	0.423	0.359	0.283	0.238
NAS01/09	0.305	0.266	0.238	0.216	0.173	0.147	0.115	0.097	0.393	0.342	0.306	0.278	0.223	0.189	0.149	0.125	0.445	0.388	0.347	0.315	0.252	0.214	0.169	0.142
NAS01/10	0.288	0.251	0.224	0.204	0.164	0.139	0.109	0.091	0.363	0.316	0.282	0.257	0.206	0.175	0.137	0.115	0.411	0.358	0.320	0.291	0.233	0.198	0.156	0.131
NAS01/11	0.285	0.249	0.222	0.202	0.162	0.138	0.108	0.090	0.363	0.316	0.283	0.257	0.206	0.175	0.137	0.115	0.412	0.359	0.321	0.291	0.233	0.198	0.156	0.131
NAS01/12	0.246	0.215	0.192	0.174	0.140	0.119	0.093	0.078	0.313	0.273	0.244	0.222	0.178	0.151	0.119	0.100	0.352	0.306	0.274	0.249	0.199	0.169	0.133	0.112
NAS02/02	0.273	0.238	0.213	0.193	0.155	0.132	0.103	0.086	0.351	0.306	0.274	0.249	0.199	0.169	0.133	0.112	0.399	0.347	0.310	0.282	0.226	0.192	0.151	0.127
NAS03/02	0.487	0.424	0.379	0.345	0.277	0.235	0.184	0.154	0.627	0.546	0.488	0.444	0.356	0.302	0.237	0.199	0.711	0.620	0.554	0.503	0.403	0.342	0.270	0.227
NAS03/03	0.285	0.248	0.222	0.202	0.162	0.138	0.108	0.090	0.367	0.320	0.286	0.260	0.208	0.177	0.139	0.117	0.416	0.363	0.324	0.295	0.236	0.200	0.158	0.133
NAS05/02	0.165	0.143	0.128	0.117	0.094	0.080	0.062	0.052	0.212	0.185	0.165	0.150	0.120	0.102	0.080	0.067	0.240	0.209	0.187	0.170	0.136	0.116	0.091	0.077
NAS05/03	0.117	0.102	0.091	0.083	0.067	0.056	0.044	0.037	0.151	0.131	0.117	0.107	0.085	0.073	0.057	0.048	0.171	0.149	0.133	0.121	0.097	0.082	0.065	0.054
NAS05/04	0.232	0.202	0.181	0.164	0.132	0.112	0.088	0.073	0.298	0.260	0.232	0.211	0.169	0.144	0.113	0.095	0.338	0.295	0.263	0.239	0.192	0.163	0.128	0.108
NAS05/05	0.167	0.145	0.130	0.118	0.095	0.081	0.063	0.053	0.215	0.187	0.167	0.152	0.122	0.104	0.081	0.068	0.244	0.212	0.190	0.172	0.138	0.117	0.092	0.078
NAS05/06	0.131	0.114	0.102	0.093	0.075	0.063	0.050	0.042	0.169	0.147	0.131	0.120	0.096	0.081	0.064	0.054	0.192	0.167	0.149	0.135	0.109	0.092	0.073	0.061
NAS07/02	0.297	0.259	0.232	0.211	0.169	0.144	0.112	0.094	0.378	0.330	0.295	0.268	0.215	0.182	0.143	0.120	0.425	0.370	0.330	0.300	0.241	0.204	0.161	0.135
NAS07/03	0.481	0.419	0.374	0.341	0.273	0.232	0.182	0.152	0.605	0.527	0.471	0.428	0.343	0.291	0.229	0.192	0.679	0.592	0.528	0.480	0.385	0.327	0.257	0.216
NAS07/01	0.168	0.146	0.131	0.119	0.095	0.081	0.064	0.053	0.214	0.186	0.166	0.151	0.121	0.103	0.081	0.068	0.240	0.209	0.187	0.170	0.136	0.115	0.091	0.076
NAS08/02	0.352	0.307	0.274	0.249	0.200	0.170	0.133	0.111	0.453	0.395	0.353	0.321	0.257	0.218	0.171	0.144	0.514	0.448	0.400	0.363	0.291	0.247	0.195	0.164
NAS08/03	0.151	0.132	0.118	0.107	0.086	0.073	0.057	0.048	0.195	0.170	0.152	0.138	0.111	0.094	0.074	0.062	0.221	0.192	0.172	0.156	0.125	0.106	0.084	0.070
NAS12/02	0.906	0.790	0.706	0.642	0.515	0.438	0.343	0.287	1.169	1.019	0.910	0.828	0.663	0.563	0.443	0.372	1.354	1.179	1.053	0.958	0.767	0.651	0.513	0.432
B- Pier Street																								
NBS01/04	0.766	0.668	0.597	0.543	0.435	0.370	0.290	0.243	0.987	0.859	0.768	0.698	0.560	0.475	0.373	0.314	1.119	0.975	0.871	0.791	0.634	0.538	0.424	0.357
NBS01/05	0.707	0.616	0.551	0.501	0.402	0.341	0.267	0.224	0.910	0.793	0.708	0.644	0.516	0.438	0.344	0.289	1.032	0.899	0.803	0.730	0.585	0.496	0.391	0.329
NBS01/07	0.445	0.388	0.347	0.315	0.253	0.215	0.168	0.141	0.567	0.494	0.441	0.401	0.322	0.273	0.215	0.180	0.643	0.560	0.500	0.455	0.364	0.309	0.244	0.205
NBS01/08	0.195	0.170	0.152	0.138	0.111	0.094	0.074	0.062	0.245	0.213	0.191	0.173	0.139	0.118	0.093	0.078	0.275	0.240	0.214	0.195	0.156	0.132	0.104	0.088
NBS01/09	0.269	0.235	0.210	0.191	0.153	0.130	0.102	0.085	0.335	0.292	0.261	0.237	0.190	0.161	0.127	0.107	0.376	0.328	0.293	0.266	0.213	0.181	0.143	0.120
NBS01/10	0.108	0.094	0.084	0.076	0.061	0.052	0.041	0.034	0.137	0.120	0.107	0.097	0.078	0.066	0.052	0.044	0.156	0.136	0.121	0.110	0.088	0.075	0.059	0.050
NBS01/11	0.176	0.153	0.137	0.125	0.100	0.085	0.066	0.056	0.224	0.195	0.174	0.158	0.127	0.108	0.085	0.071	0.251	0.219	0.195	0.178	0.142	0.121	0.095	0.080
NBS01/12	0.654	0.570	0.509	0.463	0.372	0.316	0.247	0.207	0.827	0.720	0.644	0.585	0.469	0.398	0.313	0.263	0.939	0.818	0.730	0.664	0.532	0.451	0.356	0.299
NBS02/01	0.075	0.065	0.058	0.053	0.043	0.036	0.028	0.024	0.097	0.084	0.075	0.068	0.055	0.047	0.037	0.031	0.110	0.096	0.085	0.078	0.062	0.053	0.042	0.035
NBS04/02	0.659	0.574	0.514	0.467	0.375	0.318	0.249	0.209	0.830	0.723	0.646	0.587	0.471	0.400	0.314	0.264	0.931	0.811	0.725	0.659	0.528	0.448	0.353	0.297
NBS05/01	0.131	0.114	0.102	0.092	0.074	0.063	0.049	0.041	0.164	0.143	0.128	0.116	0.093	0.079	0.062	0.052	0.186	0.162	0.145	0.132	0.106	0.090	0.071	0.059

Node Name	20 Year Event								50 Year Event								100 Year Event								
	15 Min Storm	20 Min Storm	25 Min Storm	30 Min Storm	45 Min Storm	1 Hour Storm	90 Min Storm	2 Hour Storm	15 Min Storm	20 Min Storm	25 Min Storm	30 Min Storm	45 Min Storm	1 Hour Storm	90 Min Storm	2 Hour Storm	15 Min Storm	20 Min Storm	25 Min Storm	30 Min Storm	45 Min Storm	1 Hour Storm	90 Min Storm	2 Hour Storm	
	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	
NBS06/01	0.254	0.221	0.198	0.180	0.144	0.122	0.096	0.080	0.323	0.281	0.251	0.228	0.183	0.156	0.122	0.103	0.362	0.316	0.282	0.256	0.205	0.174	0.137	0.116	
NBS06/02	0.288	0.251	0.224	0.204	0.163	0.139	0.109	0.091	0.362	0.315	0.282	0.256	0.205	0.174	0.137	0.115	0.406	0.354	0.316	0.287	0.230	0.195	0.154	0.130	
C- Pilot Street																									
NCS01/01	0.115	0.100	0.089	0.081	0.065	0.055	0.043	0.036	0.148	0.129	0.115	0.105	0.084	0.071	0.056	0.047	0.168	0.146	0.130	0.119	0.095	0.081	0.063	0.053	
NCS01/02	0.861	0.751	0.671	0.610	0.490	0.416	0.326	0.273	1.109	0.966	0.864	0.785	0.629	0.534	0.420	0.353	1.258	1.096	0.979	0.890	0.713	0.605	0.477	0.401	
NCS01/03	0.239	0.208	0.186	0.169	0.136	0.115	0.090	0.076	0.307	0.268	0.239	0.217	0.174	0.148	0.116	0.098	0.348	0.304	0.271	0.246	0.197	0.168	0.132	0.111	
NCS01/04	0.447	0.390	0.348	0.317	0.254	0.216	0.169	0.142	0.576	0.502	0.448	0.407	0.327	0.277	0.218	0.183	0.653	0.569	0.508	0.462	0.370	0.314	0.247	0.208	
NCS01/05	0.165	0.143	0.128	0.117	0.094	0.080	0.062	0.052	0.212	0.185	0.165	0.150	0.120	0.102	0.080	0.067	0.240	0.209	0.187	0.170	0.136	0.116	0.091	0.077	
NCS01/06	0.230	0.200	0.179	0.163	0.131	0.111	0.087	0.073	0.296	0.258	0.230	0.209	0.168	0.143	0.112	0.094	0.336	0.292	0.261	0.237	0.190	0.161	0.127	0.107	
NCS01/08	0.111	0.097	0.087	0.079	0.063	0.054	0.042	0.035	0.143	0.125	0.112	0.101	0.081	0.069	0.054	0.046	0.163	0.142	0.126	0.115	0.092	0.078	0.062	0.052	
NCS01/09	0.222	0.194	0.173	0.158	0.126	0.107	0.084	0.070	0.284	0.247	0.221	0.201	0.161	0.137	0.107	0.090	0.328	0.286	0.256	0.232	0.186	0.158	0.124	0.105	
NCS01/10	0.223	0.194	0.174	0.158	0.127	0.108	0.084	0.071	0.284	0.248	0.221	0.201	0.161	0.137	0.108	0.090	0.326	0.284	0.253	0.230	0.185	0.157	0.123	0.104	
NCS03/01	0.528	0.460	0.411	0.374	0.300	0.255	0.200	0.167	0.680	0.592	0.529	0.481	0.386	0.327	0.257	0.216	0.771	0.672	0.600	0.545	0.437	0.371	0.292	0.246	
NCS03/02	0.386	0.336	0.301	0.274	0.219	0.186	0.146	0.122	0.497	0.433	0.387	0.352	0.282	0.240	0.188	0.158	0.564	0.491	0.439	0.399	0.320	0.271	0.214	0.180	
NCS04/02	0.451	0.393	0.351	0.320	0.256	0.218	0.171	0.143	0.577	0.502	0.449	0.408	0.327	0.278	0.218	0.183	0.668	0.582	0.520	0.473	0.379	0.321	0.253	0.213	
D- Guard Street																									
NDS01/02	0.670	0.584	0.522	0.475	0.381	0.324	0.253	0.212	0.863	0.752	0.672	0.611	0.490	0.416	0.327	0.274	0.979	0.853	0.762	0.693	0.555	0.471	0.371	0.312	
NDS01/04	0.473	0.412	0.368	0.335	0.269	0.228	0.179	0.150	0.603	0.525	0.469	0.427	0.342	0.290	0.228	0.192	0.698	0.608	0.543	0.494	0.396	0.336	0.264	0.223	
NDS01/05	0.403	0.351	0.314	0.286	0.229	0.195	0.152	0.128	0.519	0.452	0.404	0.368	0.295	0.250	0.197	0.165	0.589	0.513	0.458	0.417	0.334	0.283	0.223	0.188	
NDS01/06	0.325	0.283	0.253	0.230	0.185	0.157	0.123	0.103	0.415	0.361	0.323	0.294	0.235	0.200	0.157	0.132	0.476	0.414	0.370	0.336	0.270	0.229	0.180	0.152	
NDS01/07	0.515	0.449	0.401	0.365	0.293	0.249	0.195	0.163	0.656	0.571	0.510	0.464	0.372	0.316	0.248	0.208	0.763	0.664	0.593	0.539	0.432	0.367	0.289	0.243	
NDS02/01	0.070	0.061	0.055	0.050	0.040	0.034	0.027	0.022	0.090	0.079	0.070	0.064	0.051	0.044	0.034	0.029	0.102	0.089	0.080	0.072	0.058	0.049	0.039	0.033	
NDS02/02	0.488	0.425	0.380	0.346	0.277	0.236	0.184	0.155	0.628	0.547	0.489	0.445	0.356	0.303	0.238	0.200	0.713	0.621	0.554	0.504	0.404	0.343	0.270	0.227	
NDS02/04	0.699	0.609	0.545	0.496	0.397	0.338	0.264	0.222	0.901	0.785	0.701	0.637	0.511	0.434	0.341	0.286	1.022	0.890	0.795	0.722	0.579	0.491	0.387	0.326	
NDS03/01	0.026	0.023	0.020	0.018	0.015	0.013	0.010	0.008	0.033	0.029	0.026	0.024	0.019	0.016	0.013	0.011	0.038	0.033	0.029	0.027	0.021	0.018	0.014	0.012	
NDS03/02	0.310	0.270	0.241	0.219	0.176	0.150	0.117	0.098	0.399	0.348	0.311	0.283	0.227	0.192	0.151	0.127	0.458	0.399	0.356	0.324	0.259	0.220	0.173	0.146	
NDS03/03	0.393	0.343	0.307	0.279	0.224	0.190	0.149	0.125	0.507	0.442	0.395	0.359	0.288	0.244	0.192	0.161	0.581	0.506	0.452	0.411	0.329	0.280	0.220	0.185	
NDS04/01	0.379	0.330	0.295	0.268	0.215	0.183	0.143	0.120	0.488	0.425	0.380	0.345	0.277	0.235	0.185	0.155	0.553	0.482	0.431	0.391	0.314	0.266	0.210	0.176	
NDS04/02	0.086	0.075	0.067	0.061	0.049	0.042	0.033	0.027	0.111	0.097	0.086	0.079	0.063	0.053	0.042	0.035	0.126	0.110	0.098	0.089	0.071	0.061	0.048	0.040	
NDS04/03	0.068	0.059	0.053	0.048	0.039	0.033	0.026	0.022	0.088	0.077	0.068	0.062	0.050	0.042	0.033	0.028	0.100	0.087	0.078	0.070	0.056	0.048	0.038	0.032	
NDS04/04	0.216	0.188	0.168	0.153	0.123	0.104	0.082	0.068	0.273	0.238	0.213	0.194	0.155	0.132	0.103	0.087	0.319	0.278	0.248	0.226	0.181	0.154	0.121	0.102	
NDS04/05	0.461	0.402	0.359	0.327	0.262	0.223	0.174	0.146	0.593	0.516	0.462	0.420	0.336	0.286	0.224	0.188	0.674	0.587	0.524	0.477	0.382	0.324	0.255	0.215	
NDS05/01	0.821	0.716	0.640	0.582	0.467	0.397	0.311	0.260	1.048	0.913	0.816	0.742	0.595	0.505	0.397	0.333	1.201	1.046	0.935	0.850	0.681	0.578	0.455	0.383	
NDS06/02	0.503	0.438	0.392	0.356	0.286	0.243	0.190	0.159	0.648	0.564	0.504	0.459	0.368	0.312	0.245	0.206	0.735	0.640	0.572	0.520	0.416	0.353	0.278	0.234	
NDS06/03	0.592	0.516	0.461	0.420	0.337	0.286	0.224	0.188	0.746	0.650	0.581	0.528	0.423	0.359	0.282	0.237	0.846	0.737	0.658	0.598	0.479	0.407	0.321	0.270	
NDS07/01	0.602	0.525	0.469	0.427	0.342	0.291	0.228	0.191	0.776	0.676	0.604	0.549	0.440	0.374	0.294	0.247	0.890	0.775	0.692	0.629	0.504	0.428	0.337	0.284	
NDS08/01	0.442	0.386	0.345	0.313	0.251	0.214	0.167	0.140	0.567	0.494	0.441	0.401	0.322	0.273	0.215	0.180	0.650	0.567	0.506	0.460	0.369	0.313	0.246	0.207	



Scale 1:5000

URANGAN CATCHMENT
RELIEF DRAINAGE INVESTIGATION
CATCHMENT PLAN

FIGURE A1

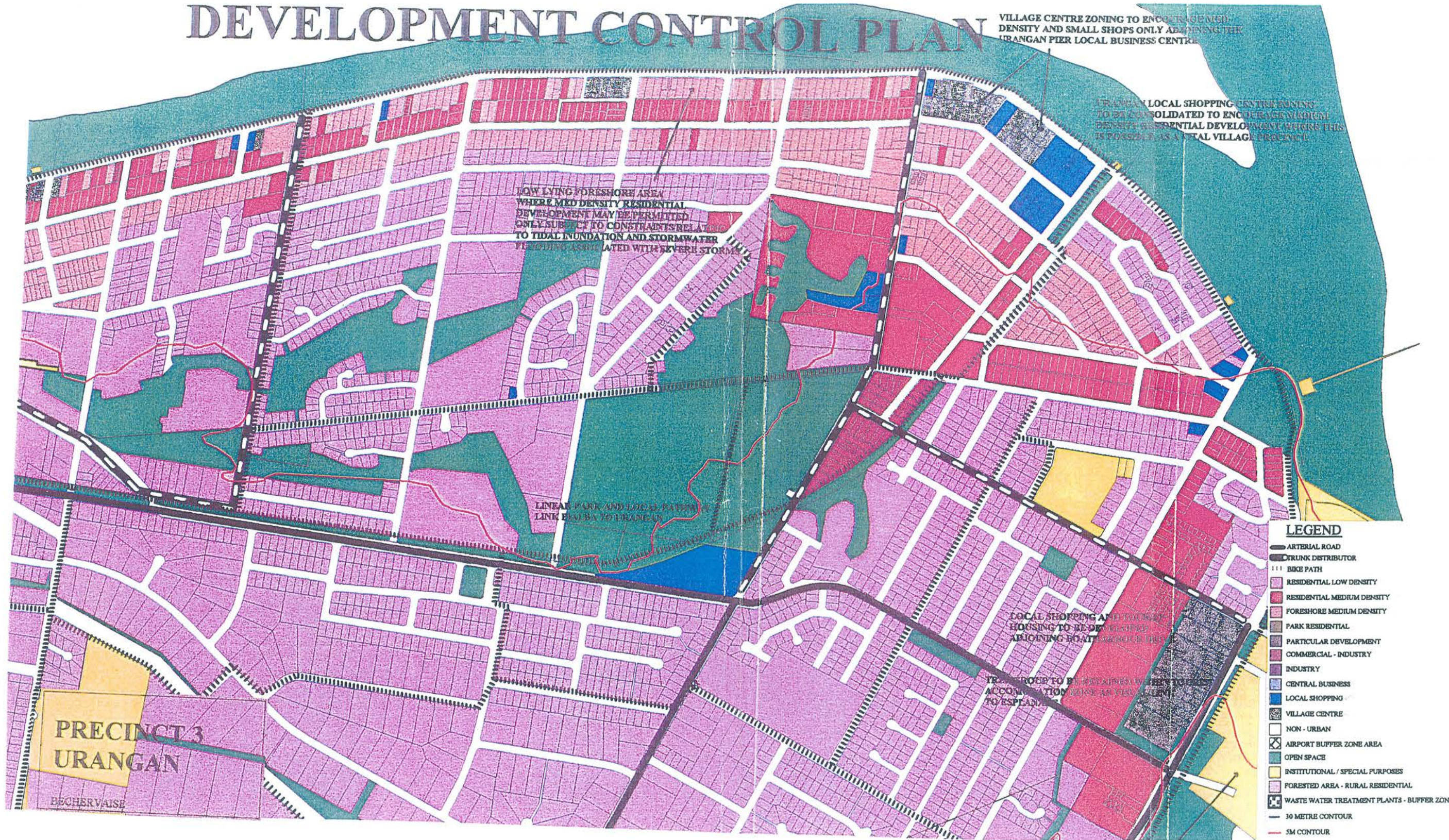
Rev: A Date: 21th March 2003

Project No.: 2919/31/02

Autocad: I:\2919-31\Acad 2003\FIGURE-A1.DWG

X-Ref:

DEVELOPMENT CONTROL PLAN



From Statement of Proposals, Hervey Bay Planning Scheme Amendment, Hervey Bay City Council, August 1998

Zoning Plan
FIGURE A2

APPENDIX B

Hydraulic Data

List of Tables

Table B1	Surface Links- General Data	B1-B2
Table B2	Surface Links- Cross Sections	B3-B7
Table B3	Adopted Invert Levels for Surface Nodes	B8-B9
Table B4	Surface Weirs	B10-B11
Table B5	Stormwater Drainage Details	B12-B13
Table B6	Adopted Invert Levels for Underground Nodes	B14-B15
Table B7	Adopted Manhole Loss Coefficients	B16
Table B8	Inlet Capacity of Side Entry Pits and Grates	B17
Table B9	Equivalent Weir Dimensions for Inlet Pits	B18
Table B10	Adopted Inlet Pits	B19-B20

List of Figures

Figure B1	XP-STORM Model Layout
Figure B2	XP-STORM Model Layout- Detail Sheet 1 of 4
Figure B3	XP-STORM Model Layout- Detail Sheet 2 of 4
Figure B4	XP-STORM Model Layout- Detail Sheet 3 of 4
Figure B5	XP-STORM Model Layout- Detail Sheet 4 of 4

TABLE B1
Surface Links- General Data

Surface Link Name	Upstream Node	Downstream Node	Channel Length (m)	Left Bank Chainage (m)	Right Bank Chainage (m)	Left Bank n (-)	Channel n (-)	Right Bank n (-)	Section Min Elev (m AHD)
A- Elizabeth Street									
LAS01/01	NAS01/01	NAS01/02	140	0	4.01	0.04	0.018	0.04	4.463
LAS01/03	NAS01/03	NAS01/04	55	0	4.01	0.04	0.018	0.04	4.117
LAS01/03A1	NAS01/03	NAS01/02	100	6	10	0.04	0.018	0.04	4.096
LAS01/04	NAS01/04	NAS01/05	35	0	6.01	0.04	0.018	0.04	3.75
LAS01/05	NAS01/05	NAS01/06	85	0	6.01	0.04	0.018	0.04	4.005
LAS01/06	NAS01/06	NAS01/07	57	0	6.01	0.04	0.018	0.04	3.6
LAS01/07	NAS01/07	NAS01/08	63	4	16.01	0.04	0.018	0.04	3.213
LAS01/08	NAS01/08	NAS01/09	22	4	17.01	0.04	0.018	0.04	3
LAS01/10	NAS01/10	NAS01/11	85	3	17.01	0.04	0.018	0.04	2.95
LAS01/10A1	NAS01/10	NAS01/09	98	3	16.01	0.04	0.018	0.04	3.078
LAS01/10A8	NAS01/10	NAS08/03	76	7	19.01	0.04	0.018	0.04	2.929
LAS01/12A1	NAS01/12	NAS01/11	30	3	17.01	0.04	0.018	0.04	2.75
LAS02/01	NAS02/01	NAS02/02	99	6.975	13.025	0.04	0.014	0.04	4.561
LAS02/02	NAS02/02	NAS01/04	84	6.975	13.025	0.04	0.014	0.04	4.221
LAS03/01	NAS03/01	NAS03/02	103	6.975	13.025	0.04	0.014	0.04	4.239
LAS03/02	NAS03/02	NAS03/03	67	6.975	13.025	0.04	0.014	0.04	4.049
LAS03/03	NAS03/03	NAS01/06	71	6.975	13.025	0.04	0.014	0.04	3.854
LAS04/01A3	NAS04/01	NAS03/02	58	6.975	13.025	0.04	0.014	0.04	4.242
LAS04/01A5	NAS04/01	NAS05/03	63	6.975	13.025	0.04	0.014	0.04	4.294
LAS05/01	NAS05/01	NAS05/02	62	7	19.01	0.04	0.018	0.04	4.182
LAS05/02	NAS05/02	NAS05/03	22	7	19.01	0.04	0.018	0.04	4.119
LAS05/03	NAS05/03	NAS05/04	58	7	19.01	0.04	0.018	0.04	3.927
LAS05/04	NAS05/04	NAS05/05	44	7	19.01	0.04	0.018	0.04	3.795
LAS05/05	NAS05/05	NAS05/06	20	7	19.01	0.04	0.018	0.04	3.5
LAS05/06	NAS05/06	NAS01/08	78	7	18.01	0.04	0.018	0.04	3.25
LAS06/01A5	NAS06/01	NAS05/04	57.5	6.975	13.025	0.04	0.014	0.04	3.986
LAS06/01A7	NAS06/01	NAS07/02	64	6.975	13.025	0.04	0.014	0.04	3.457
LAS07/01	NAS07/01	NAS07/02	32	6.975	13.025	0.04	0.014	0.04	3.059
LAS07/03A2	NAS07/03	NAS07/02	30	6.975	13.025	0.04	0.014	0.04	2.909
LAS07/04	NAS07/04	NAS01/10	70	6.975	13.025	0.04	0.014	0.04	3.297
LAS07/04A7	NAS07/04	NAS07/03	103.5	6.975	13.025	0.04	0.014	0.04	3.199
LAS08/01	NAS08/01	NAS08/02	107	1	13.01	0.04	0.018	0.04	3.342
LAS08/02	NAS08/02	NAS08/03	35	1	13.01	0.04	0.018	0.04	2.931
LAS09/01	NAS09/01	NAS08/02	95	6.5	13.01	0.04	0.018	0.04	3.442
LAS10/01	NAS10/01	NAS01/12	110	2	7	0.04	0.018	0.04	3.076
LAS12/01	NAS12/01	NAS12/02	100	6	18	0.04	0.018	0.04	2.828
LAS12/03	NAS12/03	NAS12/02	74	6	18	0.04	0.018	0.04	2.783
LAS12/03A1	NAS12/03	NAS01/12	21	6	19	0.04	0.018	0.04	2.89
B- Pier Street									
LAS02/01B1	NAS02/01	NBS01/04	92	6.975	13.025	0.04	0.014	0.04	4.63
LAS03/01B1	NAS03/01	NBS01/05	98	6.975	13.025	0.04	0.014	0.04	4.292
LAS05/01B2	NAS05/01	NBS02/01	20	1	13.01	0.04	0.018	0.04	4.305
LAS07/03B6	NAS11/01	NAS07/03	54	0	3.5	0.04	0.02	0.04	3.239
LAS10/01B6	NAS10/01	NBS06/01	118	0	6.01	0.04	0.018	0.04	2.977
LAS11/01B6	NAS11/01	NBS06/01	54	0	3.5	0.04	0.02	0.04	3.168
LBS01/03	NBS01/03	NBS01/04	119	6.975	13.025	0.04	0.014	0.04	4.715
LBS01/04	NBS01/04A	NBS01/05	63	6.975	13.025	0.04	0.014	0.04	4.465
LBS01/04B1	NBS01/04A	NBS01/04	59	6.975	13.025	0.04	0.014	0.04	4.615
LBS01/06	NBS01/06	NBS01/07	44	6.975	13.025	0.04	0.014	0.04	4.165
LBS01/06B1	NBS01/06	NBS01/05	79	6.975	13.025	0.04	0.014	0.04	4.315
LBS01/07	NBS01/07	NBS01/08	66	4	16.01	0.04	0.018	0.04	3.465
LBS01/09	NBS01/09	NBS01/10	60	3.5	16.01	0.04	0.018	0.04	3.029
LBS01/09B1	NBS01/09	NBS01/08	54	4.5	16.51	0.04	0.018	0.04	3.087
LBS01/10	NBS01/10	NBS01/11	66	3.5	16.01	0.04	0.018	0.04	2.667
LBS01/11	NBS01/11	NBS01/12	124	2.51	8.5	0.04	0.018	0.04	2.203
LBS02/01	NBS02/01	NBS01/07	125	1	13.01	0.04	0.018	0.04	3.863
LBS02/01B5	NBS02/01	NBS05/01	73	6.975	13.025	0.04	0.014	0.04	4.227
LBS03/01	NBS03/01	NBS01/07	35	5.975	14.025	0.04	0.014	0.04	3.99
LBS04/01	NBS04/01	NBS04/02	71	6.975	13.025	0.04	0.014	0.04	3.397

Surface Link Name	Upstream Node	Downstream Node	Channel Length (m)	Left Bank Chainage (m)	Right Bank Chainage (m)	Left Bank n (-)	Channel n (-)	Right Bank n (-)	Section Min Elev (m AHD)
LBS04/01A7	NBS04/01	NAS07/01	38	6.975	13.025	0.04	0.014	0.04	3.445
LBS04/02B1	NBS04/02	NBS01/12	122	0	2.5	0.04	0.02	0.04	2.641
LBS04/03	NBS04/03	NBS01/09	61.5	6.975	13.025	0.04	0.014	0.04	3.454
LBS04/03B4	NBS04/03	NBS04/02	70	6.975	13.025	0.04	0.014	0.04	3.338
LBS05/01	NBS05/01	NBS04/02	48	6.975	13.025	0.04	0.014	0.04	3.607
LBS06/01	NBS06/01	NBS06/02	100	0	5.01	0.04	0.018	0.04	2.674
LBS06/02	NBS06/02	NBS01/12	78	0	5.01	0.04	0.018	0.04	2.162
C- Pilot Street									
LBS03/01C1	NBS03/01	NCS01/07	27	6.975	15.025	0.04	0.014	0.04	3.94
LCS01/01	NCS01/01	NCS01/02	124	9	17.01	0.04	0.018	0.04	5.801
LCS01/02	NCS01/02	NCS01/03	102	8.5	16.51	0.04	0.018	0.04	4.893
LCS01/03	NCS01/03	NCS01/04	52	8	16.01	0.04	0.018	0.04	4.1
LCS01/04	NCS01/04	NCS01/05	44	8	16.01	0.04	0.018	0.04	3.502
LCS01/05	NCS01/05	NCS01/06	64	7.5	16.01	0.04	0.018	0.04	3.5
LCS01/07	NCS01/07	NCS01/08	28	7	12.51	0.04	0.018	0.04	3.349
LCS01/08	NCS01/08	NCS01/09	82	7.5	12.51	0.04	0.018	0.04	3.113
LCS01/09D3	NCS01/09	NDS03/01	42	6.51	19.01	0.04	0.018	0.04	2.951
LCS01/10C1	NCS01/10	NCS01/09	122	7.5	12.51	0.04	0.018	0.04	3.053
LCS01/11	NCS01/11	NCS04/02	66	17.975	24.025	0.04	0.014	0.04	3.39
LCS01/11C1	NCS01/11	NCS01/10	30	6.975	13.025	0.04	0.014	0.04	3.415
LCS02/01	NCS02/01	NCS01/02	128	6.5	16.51	0.04	0.018	0.04	5.231
LCS02/01C3	NCS02/01	NCS03/01	108	3.5	14.01	0.04	0.018	0.04	5.589
LCS03/01	NCS03/01	NCS03/02	98	8.5	19.01	0.04	0.018	0.04	4.292
LCS03/02	NCS03/02	NCS01/04	110	8	19.01	0.04	0.018	0.04	3.358
LCS04/01	NCS04/01	NCS04/02	102	3	9.01	0.04	0.018	0.04	3.25
D- Guard Street									
LCS03/01D1	NCS03/01	NDS01/01	5	5	29.01	0.04	0.018	0.04	5.51
LCS04/01D8	NCS04/01	NDS08/01	104	20	28.01	0.04	0.018	0.04	3.172
LDS01/01	NDS01/01	NDS01/02	146	5	14.01	0.04	0.018	0.04	5.227
LDS01/02	NDS01/02	NDS01/03	24	8	24.01	0.04	0.018	0.04	5
LDS01/03	NDS01/03	NDS01/04	78	3.5	16.51	0.04	0.018	0.04	2.465
LDS01/03D2	NDS01/03	NDS02/04	130	6	18.01	0.04	0.018	0.04	3.292
LDS01/04	NDS01/04	NDS01/05	24	3	16.01	0.04	0.018	0.04	1.902
LDS01/06	NDS01/06	NDS01/05	52	4	17.01	0.04	0.018	0.04	2.061
LDS01/07	NDS01/07	NDS01/06	64	4	16.51	0.04	0.018	0.04	2.144
LDS02/01	NDS02/01	NDS02/02	58	1	14.01	0.04	0.018	0.04	3.172
LDS02/03	NDS02/03	NDS02/02	64	6	18.01	0.04	0.018	0.04	3.216
LDS02/03D2	NDS02/03	NDS02/04	50	2	14.51	0.04	0.018	0.04	3.19
LDS03/01	NDS03/01	NDS03/02	70	1	14.01	0.04	0.018	0.04	2.7
LDS03/02	NDS03/02	NDS03/03	120	1	14.51	0.04	0.018	0.04	2.638
LDS03/02D7	NDS03/02	NDS07/01	130	1	13.01	0.04	0.018	0.04	2.656
LDS03/04	NDS03/04	NDS03/03	62	5	17.51	0.04	0.018	0.04	2.567
LDS03/04D1	NDS03/04	NDS01/04	136	1.5	15.01	0.04	0.018	0.04	2.408
LDS04/01	NDS04/01	NDS04/02	24	0	14.01	0.04	0.018	0.04	10.598
LDS04/02	NDS04/02	NDS04/03	72	4.5	17.51	0.04	0.018	0.04	7.775
LDS04/03	NDS04/03	NDS04/04	48	4.5	21.51	0.04	0.018	0.04	4.577
LDS04/03D6	NDS04/03	NDS06/03	88	5	18.51	0.04	0.018	0.04	3.65
LDS04/04	NDS04/04	NDS04/05	54	4	16.51	0.04	0.018	0.04	2.386
LDS05/01	NDS05/01	NDS04/02	28	5	0	0.04	0.018	0.04	10.094
LDS06/01	NDS06/01	NDS06/02	78	2.5	14	0.04	0.018	0.04	3.671
LDS06/02	NDS06/02	NDS06/03	66	2	14	0.04	0.018	0.04	2.867
LDS06/03	NDS06/03	NDS01/05	118	6	18.01	0.04	0.018	0.04	2.031
LDS07/02	NDS07/02	NDS07/01	96	5	18.51	0.04	0.018	0.04	2.464
LDS07/02D1	NDS07/02	NDS01/06	86	2	14.51	0.04	0.018	0.04	2.401
LDS08/02	NDS08/02	NDS08/01	58	4.5	13.01	0.04	0.018	0.04	2.465
LDS08/02D1	NDS08/02	NDS01/07	114	45	53.51	0.04	0.018	0.04	2.37
LDS09/01	NDS09/01	NDS01/07	35	5	12.51	0.04	0.018	0.04	2.384
LDS09/01D4	NDS09/01	NDS04/05	74	33.5	40.51	0.04	0.018	0.04	2.457

Note: Shaded links represent those roads which are to be regraded

TABLE B2
Surface Links- Cross Sections

Surface Link Name	No. of Points	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
Points in order of increasing chainage, Chainage in first row, elevation in second																					
A- Elizabeth Street																					
LAS01/01	Ch	6	0.000	0.010	4.000	4.010	9.000	10.000													
	Elev	6	11.000	4.585	4.463	4.463	4.500	4.667													
LAS01/03A1	Ch	6	0.000	0.010	1.000	6.000	6.010	10.000													
	Elev	6	11.000	4.397	4.230	4.096	4.096	4.250													
LAS01/03	Ch	6	0.000	0.010	4.000	4.010	9.000	10.000													
	Elev	6	11.000	4.223	4.117	4.117	4.250	4.416													
LAS01/04	Ch	6	0.000	0.010	6.000	6.010	9.000	10.000													
	Elev	6	11.000	4.052	3.750	3.900	4.000	4.167													
LAS01/05	Ch	6	0.000	0.010	6.000	6.010	9.000	10.000													
	Elev	6	11.000	4.073	4.005	4.155	4.111	4.278													
LAS01/06	Ch	6	0.000	0.010	6.000	6.010	9.000	10.000													
	Elev	6	11.000	3.788	3.600	3.750	3.902	4.069													
LAS01/07	Ch	10	0.000	0.010	1.000	4.000	4.010	10.000	16.000	16.010	19.000	20.000									
	Elev	10	11.000	3.657	3.490	3.400	3.250	3.363	3.213	3.363	3.453	3.620									
LAS01/08	Ch	10	0.000	0.010	1.000	4.000	4.010	10.500	17.000	17.010	19.000	20.000									
	Elev	10	11.000	3.407	3.240	3.150	3.000	3.129	3.000	3.150	3.210	3.377									
LAS01/10A1	Ch	10	0.000	0.010	1.000	3.000	3.010	9.500	16.000	16.010	19.000	20.000									
	Elev	10	11.000	3.819	3.652	3.228	3.078	3.240	3.100	3.250	3.340	3.507									
LAS01/10	Ch	10	0.000	0.010	1.000	3.000	3.010	10.000	17.000	17.010	19.000	20.000									
	Elev	10	11.000	3.327	3.160	3.100	2.950	3.183	2.950	3.100	3.160	3.327									
LAS01/12A1	Ch	10	0.000	0.010	1.000	3.000	3.010	10.000	17.000	17.010	19.000	20.000									
	Elev	10	11.000	3.227	3.060	3.000	2.850	3.107	2.750	2.900	2.960	3.127									
LAS02/01	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000							
	Elev	12	11.000	5.045	4.880	4.701	4.561	4.584	4.646	4.584	4.561	4.701	4.880	5.047							
LAS02/02	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000							
	Elev	12	11.000	4.705	4.540	4.361	4.221	4.244	4.306	4.244	4.221	4.361	4.540	4.707							
LAS03/01	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000							
	Elev	12	11.000	4.723	4.558	4.379	4.239	4.262	4.324	4.262	4.239	4.379	4.558	4.725							
LAS03/02	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000							
	Elev	12	11.000	4.510	4.345	4.166	4.026	4.049	4.111	4.049	4.026	4.166	4.345	4.512							
LAS03/03	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000							
	Elev	12	11.000	4.338	4.173	3.994	3.854	3.877	3.939	3.877	3.854	3.994	4.173	4.340							
LAS04/01A3	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000							
	Elev	12	11.000	4.726	4.561	4.382	4.242	4.265	4.327	4.265	4.242	4.382	4.561	4.728							
LAS04/01A5	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000							
	Elev	12	11.000	4.778	4.613	4.434	4.294	4.317	4.379	4.317	4.294	4.434	4.613	4.780							
LAS05/01	Ch	9	0.000	0.010	1.000	7.000	7.010	13.000	19.000	19.010	20.000										
	Elev	9	11.000	4.678	4.512	4.332	4.182	4.515	4.182	4.332	4.499										
LAS05/02	Ch	9	0.000	0.010	1.000	7.000	7.010	13.000	19.000	19.010	20.000										
	Elev	9	11.000	4.449	4.269	4.269	4.119	4.319	4.227	4.377	4.543										
LAS05/03	Ch	9	0.000	0.010	1.000	7.000	7.010	13.000	19.000	19.010	20.000										
	Elev	9	11.000	4.424	4.257	4.077	3.927	4.052	3.985	4.102	4.269										
LAS05/04	Ch	9	0.000	0.010	1.000	7.000	7.010	13.000	19.000	19.010	20.000										
	Elev	9	11.000	4.292	4.125	3.945	3.795	3.945	3.825	3.975	4.142										

Surface Link Name		No. of Points	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	
			Points in order of increasing chainage, Chainage in first row, elevation in second																				
LAS05/05	Ch	9	0.000	0.010	1.000	7.000	7.010	13.000	19.000	19.010	20.000												
	Elev	9	11.000	4.032	3.866	3.686	3.536	3.750	3.500	3.650	3.817												
LAS05/06	Ch	10	0.000	0.010	1.000	7.000	7.010	12.500	18.000	18.000	19.000	20.000											
	Elev	10	11.000	3.747	3.580	3.400	3.250	3.407	3.250	3.400	3.430	3.597											
LAS06/01A5	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000									
	Elev	12	11.000	4.470	4.305	4.126	3.986	4.009	4.071	4.009	3.986	4.126	4.305	4.472									
LAS06/01A7	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000									
	Elev	12	11.000	3.941	3.776	3.597	3.457	3.480	3.542	3.480	3.457	3.597	3.776	3.943									
LAS07/01	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000									
	Elev	12	11.000	3.543	3.378	3.199	3.059	3.082	3.144	3.082	3.059	3.199	3.378	3.545									
LAS07/03A2	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000									
	Elev	12	11.000	3.393	3.228	3.049	2.909	2.932	2.994	2.932	2.909	3.049	3.228	3.395									
LAS07/04A7	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000									
	Elev	12	11.000	3.683	3.518	3.339	3.199	3.222	3.284	3.222	3.199	3.339	3.518	3.685									
LAS07/04	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000									
	Elev	12	11.000	3.781	3.616	3.437	3.297	3.320	3.382	3.320	3.297	3.437	3.616	3.783									
LAS08/01	Ch	9	0.000	0.010	1.000	1.010	7.000	13.000	13.010	19.000	20.000												
	Elev	9	11.000	3.421	3.421	3.342	3.592	3.342	3.492	3.672	3.839												
LAS08/02	Ch	9	0.000	0.010	1.000	1.010	7.000	13.000	13.010	19.000	20.000												
	Elev	9	11.000	3.161	3.131	3.056	3.306	2.931	3.081	3.261	3.428												
LAS01/10A8	Ch	9	0.000	0.010	1.000	7.000	7.010	13.000	19.000	19.010	20.000												
	Elev	9	11.000	3.453	3.286	3.106	2.929	3.350	3.110	3.185	3.215												
LAS09/01	Ch	10	0.000	0.010	1.000	6.500	6.510	9.750	13.000	13.010	19.000	20.000											
	Elev	10	11.000	3.879	3.712	3.547	3.523	3.604	3.442	3.517	3.697	3.864											
LAS10/01	Ch	5	0.000	1.000	2.000	2.010	7.000																
	Elev	5	11.000	3.423	3.256	3.076	3.326																
LAS12/03A1	Ch	11	0.000	0.010	1.500	6.000	6.010	6.310	12.000	18.690	18.990	19.000	25.000										
	Elev	11	11.000	3.390	3.140	3.036	2.890	2.915	3.101	3.064	3.039	3.138	3.200										
LAS12/03	Ch	11	0.000	0.010	1.500	6.000	6.010	6.310	12.000	17.690	17.990	18.000	25.000										
	Elev	11	11.000	3.250	3.000	2.933	2.783	2.808	3.084	3.074	3.049	3.224	3.300										
LAS12/01	Ch	11	0.000	0.010	1.500	6.000	6.010	6.310	12.000	17.690	17.990	18.000	40.000										
	Elev	11	11.000	3.250	3.000	2.978	2.828	2.853	3.182	2.919	2.894	3.044	3.500										
B- Pier Street																							
LBS01/03	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000									
	Elev	12	11.000	5.199	5.034	4.855	4.715	4.738	4.800	4.738	4.715	4.855	5.034	5.201									
LBS01/04	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000									
	Elev	12	11.000	4.949	4.784	4.605	4.465	4.488	4.550	4.488	4.465	4.605	4.784	4.951									
LBS01/04B1	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000									
	Elev	12	11.000	5.099	4.934	4.755	4.615	4.638	4.700	4.638	4.615	4.755	4.934	5.101									
LBS01/06B1	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000									
	Elev	12	11.000	4.799	4.634	4.455	4.315	4.338	4.400	4.338	4.315	4.455	4.634	4.801									
LBS01/06	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000									
	Elev	12	11.000	4.649	4.484	4.305	4.165	4.188	4.250	4.188	4.165	4.305	4.484	4.651									
LBS01/07	Ch	10	0.000	0.010	1.000	4.000	4.010	10.000	16.000	16.010	19.500	20.500											
	Elev	10	11.000	3.887	3.720	3.615	3.465	3.676	3.576	3.726	3.831	3.998											
LBS01/09B1	Ch	10	0.000	0.010	1.000	4.500	4.510	10.500	16.500	16.510	19.000	20.000											
	Elev	10	11.000	3.806	3.640	3.535	3.385	3.326	3.087	3.237	3.640	3.494											
LBS01/09	Ch	10	0.000	0.010	1.000	3.500	3.510	9.750	16.000	16.010	19.500	20.500											

Surface Link Name	No. of Points	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	
		Points in order of increasing chainage, Chainage in first row, elevation in second																				
LBS01/10	Elev	10	11.000	3.421	3.254	3.179	3.029	3.442	3.353	3.503	3.608	3.775										
	Ch	10	0.000	0.010	1.000	3.500	3.510	9.750	16.000	16.010	19.500	20.500										
LAS02/01B1	Elev	10	11.000	3.058	2.892	2.817	2.667	2.927	3.017	3.167	3.272	3.439										
	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000								
LAS03/01B1	Elev	12	11.000	5.114	4.949	4.770	4.630	4.653	4.715	4.653	4.630	4.770	4.949	5.116								
	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000								
LAS05/01B2	Elev	12	11.000	4.776	4.611	4.432	4.292	4.315	4.377	4.315	4.292	4.432	4.611	4.778								
	Ch	9	0.000	0.010	1.000	1.010	7.000	13.000	13.010	19.000	20.000											
LBS02/01	Elev	9	11.000	4.672	4.505	4.355	4.605	4.305	4.455	4.635	4.802											
	Ch	9	0.000	0.010	1.000	1.010	7.000	13.000	13.010	19.000	20.000											
LBS03/01	Elev	9	11.000	4.341	4.174	4.024	4.238	3.863	4.013	4.193	4.360											
	Ch	12	0.000	0.010	1.000	5.975	6.250	6.525	10.000	13.475	13.750	14.025	19.000	20.000								
LBS04/01A7	Elev	12	11.000	4.444	4.279	4.130	3.990	4.013	4.100	4.013	3.990	4.130	4.279	4.446								
	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000								
LBS04/01	Elev	12	11.000	3.929	3.764	3.585	3.445	3.468	3.530	3.468	3.445	3.585	3.764	3.931								
	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000								
LBS04/03B4	Elev	12	11.000	3.881	3.716	3.537	3.397	3.420	3.482	3.420	3.397	3.537	3.716	3.883								
	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000								
LBS04/03	Elev	12	11.000	3.822	3.657	3.478	3.338	3.361	3.423	3.361	3.338	3.478	3.657	3.824								
	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000								
LBS02/01B5	Elev	12	11.000	3.938	3.773	3.594	3.454	3.477	3.539	3.477	3.454	3.594	3.773	3.940								
	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000								
LBS05/01	Elev	12	11.000	4.711	4.546	4.367	4.227	4.250	4.312	4.250	4.227	4.367	4.546	4.713								
	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000								
LBS01/11	Elev	12	11.000	4.091	3.926	3.747	3.607	3.630	3.692	3.630	3.607	3.747	3.926	4.093								
	Ch	6	0.000	0.010	1.000	2.500	2.510	8.500														
LAS10/01B6	Elev	6	11.000	2.565	2.398	2.353	2.203	2.390														
	Ch	6	0.000	0.010	6.000	6.010	11.000	12.000														
LBS06/01	Elev	6	11.000	3.227	2.977	3.127	3.277	3.444														
	Ch	6	0.000	0.010	5.000	5.010	13.500	14.500														
LBS06/02	Elev	6	11.000	2.924	2.674	2.824	3.079	3.246														
	Ch	6	0.000	0.010	6.000	6.010	10.500	11.500														
LAS11/01	Elev	6	11.000	2.591	2.162	2.312	2.432	2.599														
	Ch	3	0.000	0.010	3.500																	
LAS11/01B6	Elev	3	11.000	3.239	3.294																	
	Ch	3	0.000	0.010	3.500																	
LBS04/02B1	Elev	3	11.000	3.168	3.223																	
	Ch	3	0.000	0.010	2.500																	
C- Pilot Street																						
LCS01/01	Elev	9	11.000	6.752	6.604	6.362	6.212	6.082	5.801	5.591	6.044											
	Ch	9	0.000	0.010	1.000	9.000	9.010	12.500	17.000	17.010	20.000											
LCS01/02	Elev	10	11.000	5.435	5.268	5.043	4.893	5.003	4.972	5.122	5.227	5.394										
	Ch	10	0.000	0.010	1.000	8.000	8.010	12.000	16.000	16.010	20.000	21.000										
LCS01/03	Elev	10	11.000	4.626	4.460	4.250	4.100	4.189	4.202	4.352	4.472	4.639										
	Ch	10	0.000	0.010	1.000	8.000	8.010	12.000	16.000	16.010	19.000	20.000										
LCS01/04	Elev	10	11.000	4.057	3.891	3.681	3.531	3.656	3.502	3.652	3.742	3.909										
	Ch	10	0.000	0.010	1.000	8.000	8.010	12.000	16.000	16.010	19.000	20.000										

Surface Link Name		No. of Points	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	
			Points in order of increasing chainage, Chainage in first row, elevation in second																				
LCS01/05	Ch	10	0.000	0.010	1.000	7.500	7.510	11.750	16.000	16.010	19.000	20.000											
	Elev	10	11.000	4.042	3.875	3.680	3.530	3.693	3.500	3.650	3.740	3.907											
LCS01/07	Ch	10	0.000	0.010	1.000	7.000	7.010	9.750	12.500	12.510	19.000	20.000											
	Elev	10	11.000	3.890	3.723	3.543	3.393	3.442	3.349	3.499	3.694	3.861											
LCS01/08	Ch	10	0.000	0.010	1.000	7.500	7.510	10.000	12.500	12.510	19.000	20.000											
	Elev	10	11.000	3.604	3.437	3.242	3.092	3.206	3.113	2.263	3.458	3.625											
LCS01/10C1	Ch	10	0.000	0.010	1.000	7.500	7.510	10.000	12.500	12.510	19.000	20.000											
	Elev	10	11.000	3.650	3.483	3.288	3.138	3.086	3.053	3.203	3.398	3.565											
LCS01/11C1	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	10.000	12.475	12.750	13.025	19.000	20.000									
	Elev	12	11.000	3.899	3.734	3.555	3.415	3.438	3.500	3.438	3.415	3.555	3.734	3.901									
LCS02/01	Ch	9	0.000	0.010	6.500	6.510	11.500	16.500	16.510	19.000	20.000												
	Elev	9	11.000	5.946	5.381	5.231	5.841	5.563	5.713	5.788	5.955												
LCS02/01C3	Ch	9	0.000	0.010	1.000	3.500	3.510	8.750	14.000	14.010	20.000												
	Elev	9	11.000	6.142	5.975	5.900	5.750	5.881	5.589	5.739	6.866												
LCS03/01	Ch	9	0.000	0.010	1.000	8.500	8.510	14.000	19.000	19.010	20.000												
	Elev	9	11.000	4.834	4.667	4.442	4.292	4.295	4.371	4.521	4.688												
LCS03/02	Ch	9	0.000	0.010	1.000	8.000	8.010	13.500	19.000	19.010	20.000												
	Elev	9	11.000	4.232	4.065	3.855	3.705	3.664	3.358	3.508	3.675												
LCS01/09D3	Ch	9	0.000	0.010	1.000	6.500	6.510	12.750	19.000	19.010	20.000												
	Elev	9	11.000	3.446	3.279	3.114	2.964	3.033	2.951	3.101	3.268												
LCS04/01	Ch	11	0.000	0.010	1.000	3.000	3.010	6.000	9.000	9.010	12.500	30.000	34.000										
	Elev	11	11.000	3.627	3.460	3.400	3.250	3.404	3.358	3.508	3.500	3.500	3.750										
LCS01/11	Ch	13	0.000	0.010	11.000	12.000	17.975	18.250	18.525	21.000	23.475	23.750	24.025	30.000	31.000								
	Elev	13	11.000	4.000	3.876	3.709	3.530	3.390	3.413	3.475	3.413	3.390	3.530	3.709	3.876								
LBS03/01C1	Ch	12	0.000	0.010	1.000	6.975	7.250	7.525	11.000	14.475	14.750	15.025	21.000	22.000									
	Elev	12	11.000	4.424	4.259	4.080	3.940	3.963	4.050	3.963	3.940	4.080	4.259	4.426									
LCS03/01D1	Ch	8	0.000	0.010	5.000	5.010	15.500	29.000	29.010	30.000													
	Elev	8	11.000	6.925	6.150	6.000	5.985	5.510	5.670	5.700													
D- Guard Street																							
LDS01/01	Ch	9	0.000	0.010	1.000	5.000	5.010	9.500	14.000	14.010	20.000												
	Elev	9	11.000	5.664	5.497	5.377	5.227	5.477	5.468	5.618	6.400												
LDS01/02	Ch	10	0.000	0.010	1.000	8.000	8.010	11.000	19.000	24.000	24.010	29.000											
	Elev	10	11.000	5.467	5.300	5.150	5.000	5.250	5.500	3.350	5.500	5.750											
LDS01/03	Ch	10	0.000	0.010	1.000	3.500	3.510	10.000	16.500	16.510	19.000	20.000											
	Elev	10	11.000	2.857	2.690	2.615	2.465	2.692	2.490	2.640	2.715	2.882											
LDS01/04	Ch	10	0.000	0.010	1.000	3.000	3.010	9.500	16.000	16.010	19.000	20.000											
	Elev	10	11.000	2.278	2.112	2.052	1.902	2.308	1.982	2.133	2.223	2.390											
LDS01/06	Ch	10	0.000	0.010	1.000	4.000	4.010	10.500	17.000	17.010	19.000	20.000											
	Elev	10	11.000	2.468	2.301	2.211	2.061	2.332	2.117	2.320	2.380	2.547											
LDS01/07	Ch	10	0.000	0.010	1.000	4.000	4.010	10.250	16.500	16.510	19.000	20.000											
	Elev	10	11.000	2.550	2.384	2.294	2.144	2.300	2.144	2.294	2.369	2.536											
LDS02/01	Ch	9	0.000	0.010	1.000	1.010	7.500	14.000	14.010	19.000	20.000												
	Elev	9	11.000	3.489	3.332	3.172	3.432	3.172	3.332	3.482	3.649												
LDS02/03	Ch	10	0.000	0.010	1.000	2.000	2.010	8.250	14.500	14.510	19.000	20.000											
	Elev	10	11.000	3.537	3.370	3.340	3.190	3.711	3.345	3.493	3.628	3.795											
LDS02/03D2	Ch	10	0.000	0.010	1.000	2.000	2.010	8.250	14.500	14.510	19.000	20.000											
	Elev	10	11.000	3.537	3.370	3.340	3.190	3.711	3.343	3.493	3.628	3.795											
LDS01/03D2	Ch	10	0.000	0.010	1.000	6.000	6.010	12.000	18.000	18.010	19.000	20.000											

Surface Link Name	No. of Points	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	
	Elev	10	11.000	3.959	3.792	3.642	3.492	3.792	3.292	3.442	3.472	3.639										
LDS03/01	Ch	9	0.000	0.010	1.000	1.010	7.000	14.000	14.010	19.000	20.000											
	Elev	9	11.000	3.017	2.850	2.700	3.033	2.715	2.865	3.015	3.182											
LDS03/02	Ch	9	0.000	0.010	1.000	1.010	7.000	14.500	14.510	19.000	20.000											
	Elev	9	11.000	2.954	2.788	2.638	2.860	2.652	2.802	2.937	3.104											
LDS03/04	Ch	10	0.000	0.010	1.000	5.000	5.010	12.000	17.500	17.510	19.000	20.000										
	Elev	10	11.000	3.119	2.952	2.832	2.682	2.817	2.567	2.717	2.762	2.929										
LDS03/04D1	Ch	10	0.000	0.010	1.000	1.500	1.510	8.000	15.000	15.010	19.000	20.000										
	Elev	10	11.000	2.740	2.573	2.558	2.408	2.605	2.456	2.606	2.726	2.893										
LDS04/01	Ch	5	0.000	0.010	14.000	14.010	22.000															
	Elev	5	16.000	10.598	10.643	10.781	10.469															
LDS04/02	Ch	8	0.000	0.010	4.500	4.510	10.500	17.500	17.510	25.000												
	Elev	8	16.000	7.916	7.916	7.775	7.969	7.917	8.056	8.125												
LDS04/03	Ch	9	0.000	0.010	1.000	4.500	4.510	9.500	21.500	21.510	25.000											
	Elev	9	11.000	4.672	4.505	4.400	4.250	4.462	4.577	4.654	4.792											
LDS04/04	Ch	9	0.000	0.010	1.000	4.000	4.010	10.000	16.500	16.510	24.500											
	Elev	9	11.000	2.792	2.625	2.536	2.386	2.850	2.386	2.536	2.849											
LDS05/01	Ch	5	0.000	0.010	5.000	5.010	11.000															
	Elev	5	16.000	10.094	10.423	10.314	10.598															
LDS06/01	Ch	9	0.000	0.010	1.000	2.500	2.510	8.500	14.000	17.500	20.000											
	Elev	9	11.000	4.026	3.859	3.814	3.671	4.179	3.774	4.500	4.900											
LDS06/02	Ch	8	0.000	0.010	1.000	2.000	2.010	8.000	14.000	20.000												
	Elev	8	11.000	3.241	3.074	3.044	2.901	4.059	2.867	3.750												
LDS04/03D6	Ch	9	0.000	0.010	5.000	5.010	12.000	18.500	18.510	19.000	20.000											
	Elev	9	11.000	4.625	4.106	3.963	4.036	3.650	3.778	3.793	3.960											
LDS06/03	Ch	10	0.000	0.010	1.000	6.000	6.010	12.000	18.000	18.010	19.000	20.000										
	Elev	10	11.000	2.707	2.539	2.389	2.239	2.364	2.031	2.181	2.211	2.377										
LDS03/02D7	Ch	9	0.000	0.010	1.000	1.010	6.000	13.000	13.010	19.000	20.000											
	Elev	9	11.000	2.973	2.806	2.656	2.748	2.656	2.806	2.986	3.153											
LDS07/02	Ch	10	0.000	0.010	1.000	5.000	5.010	12.500	18.500	18.510	19.000	20.000										
	Elev	10	11.000	2.929	2.762	2.642	2.492	2.695	2.464	2.614	2.629	2.769										
LDS07/02D1	Ch	10	0.000	0.010	1.000	2.000	2.010	7.500	14.500	14.510	19.000	20.000										
	Elev	10	11.000	3.133	2.967	2.937	2.787	2.681	2.401	2.551	2.686	2.853										
LCS04/01D8	Ch	12	0.000	0.010	4.000	8.500	13.000	20.000	20.010	24.500	28.000	28.010	32.000	33.000								
	Elev	12	11.000	4.985	4.750	4.500	4.250	3.322	3.172	3.396	3.212	3.362	3.482	3.648								
LDS08/02	Ch	13	0.000	0.010	1.000	4.500	4.510	9.000	13.000	13.010	20.000	28.000	32.000	34.000	43.500							
	Elev	13	11.000	2.887	2.720	2.615	2.465	2.702	2.491	2.641	2.662	2.750	3.000	3.250	4.885							
LDS08/02D1	Ch	14	0.000	0.010	7.500	19.000	22.000	32.500	38.000	45.000	45.010	49.500	53.500	53.510	57.000	58.000						
	Elev	14	11.000	4.594	4.000	2.750	2.500	2.500	2.611	2.595	2.370	2.566	2.392	2.542	2.647	2.814						
LDS09/01	Ch	13	0.000	0.010	1.000	5.000	5.010	8.000	12.500	12.510	20.000	31.000	34.000	56.000	59.000							
	Elev	13	11.000	2.833	2.667	2.547	2.397	2.730	2.384	2.534	2.550	2.750	3.000	5.000	5.250							
LDS09/01D4	Ch	14	0.000	0.010	2.500	9.000	13.000	21.000	25.000	33.500	33.510	37.000	40.500	40.510	44.000	45.000						
	Elev	14	11.000	4.800	4.750	4.250	3.750	3.250	3.000	2.682	2.532	2.632	2.457	2.607	2.697	2.864						

Note: Shaded links represent those roads which are to be regraded

TABLE B3
Adopted Invert Levels for Surface Nodes

NODE Name	Node IL (m AHD)
A- Elizabeth Street	
NAS01/01	4.655
NAS01/02	4.000
NAS01/03	4.194
NAS01/04	3.800
NAS01/05	3.651
NAS01/06	3.610
NAS01/07	3.323
NAS01/08	3.003
NAS01/09	2.799
NAS01/10	3.167
NAS01/11	2.537
NAS01/12	2.836
NAS02/01	4.745
NAS02/02	4.377
NAS03/01	4.368
NAS03/02	4.110
NAS03/03	3.943
NAS04/01	4.373
NAS05/01	4.310
NAS05/02	4.180
NAS05/03	4.100
NAS05/04	3.800
NAS05/05	3.602
NAS05/06	3.450
NAS06/01	4.066
NAS07/01	3.270
NAS07/02	2.847
NAS07/03	2.971
NAS07/04	3.426
NAS08/01	3.833
NAS08/02	3.202
NAS08/03	2.900
NAS09/01	3.917
NAS10/01	3.288
NAS11/01	3.506
NAS12/01	3.250
NAS12/02	2.722
NAS12/03	2.935
B- Pier Street	
NBS01/03	4.915
NBS01/04	4.515
NBS01/04A	4.715
NBS01/05	4.215
NBS01/06	4.415
NBS01/07	3.755
NBS01/08	3.080
NBS01/09	3.406
NBS01/10	2.827
NBS01/11	2.357
NBS01/12	2.036
NBS02/01	4.300
NBS03/01	4.090
NBS04/01	3.620
NBS04/02	3.173
NBS04/03	3.502

NODE Name	Node IL (m AHD)
NBS05/01	4.040
NBS06/01	2.829
NBS06/02	2.477
C- Pilot Street	
NCS01/01	6.275
NCS01/02	5.088
NCS01/03	4.607
NCS01/04	3.589
NCS01/05	3.500
NCS01/06	3.452
NCS01/07	3.403
NCS01/08	3.134
NCS01/09	3.050
NCS01/10	3.366
NCS01/11	3.665
NCS02/01	5.750
NCS03/01	5.520
NCS03/02	3.600
NCS04/01	3.447
NCS04/02	3.175
D- Guard Street	
NDS01/01	5.500
NDS01/02	5.181
NDS01/03	3.339
NDS01/04	2.159
NDS01/05	1.900
NDS01/06	2.100
NDS01/07	2.200
NDS02/01	3.273
NDS02/02	3.061
NDS02/03	3.294
NDS02/04	3.151
NDS03/01	2.950
NDS03/02	2.690
NDS03/03	2.412
NDS03/04	2.582
NDS04/01	10.873
NDS04/02	9.750
NDS04/03	6.041
NDS04/04	3.481
NDS04/05	2.226
NDS05/01	10.341
NDS06/01	4.000
NDS06/02	3.114
NDS06/03	2.653
NDS07/01	2.392
NDS07/02	2.554
NDS08/01	2.351
NDS08/02	2.500
NDS09/01	2.588
NCS01/12	0.544
NDS04/06	0.545

Note: Shaded entries represent those roads which are to be regraded

**TABLE B4
Surface Weirs**

\$TABLE

/* Specdiv turns weir on, specname gives name of weir */

	SPECDIV	SPECNAME
"LCS01/OUT"	1 1	4 "LCS01/OUT"
"LDS04/OUT"	1 1	4 "LDS04/OUT"
"LCS01/11B1"	1 1	4 "LCS01/11B1"
"LDS01/02D6"	1 1	4 "LDS01/02D6"
"LCS01/06"	1 1	4 "LCS01/06"
"LCS01/07D2"	1 1	4 "LCS01/07D2"
"LCS01/06D2"	1 1	4 "LCS01/06D2"

\$TABLE_END

\$TABLE

/* SPEC1 turns on special pipe*/

	SPEC1
"LCS01/OUT"	0 1 1
"LDS04/OUT"	0 1 1
"LCS01/11B1"	0 1 1
"LDS01/02D6"	0 1 1
"LCS01/06"	0 1 1
"LCS01/07D2"	0 1 1
"LCS01/06D2"	0 1 1

\$TABLE_END

/* SCV1= Depth, SCV2= Length, SCV3= Exponent, SCV4= Coefficient */

DATA SCV1 "LCS01/OUT"	1 11	0	0.062	0.063	0.075	0.085	0.335	0.585	0.835	1.085	1.335	6.335				
DATA SCV1 "LDS04/OUT"	1 14	0	0.474	0.475	0.524	0.774	1.024	1.106	1.274	1.524	1.774	2.024	2.274	2.356	7.356	
DATA SCV1 "LCS01/11B1"	1 8	0	0.513	0.514	0.563	0.634	0.789	0.835	5.563							
DATA SCV1 "LDS01/02D6"	1 8	0	0.119	0.12	0.17	0.369	0.619	0.719	5.719							
DATA SCV1 "LCS01/06"	1 7	0	0.298	0.299	0.305	0.459	0.488	5.488								
DATA SCV1 "LCS01/07D2"	1 7	0	0.001	0.05	0.097	0.347	0.507	5.507								
DATA SCV1 "LCS01/06D2"	1 7	0	0.001	0.05	0.118	0.258	0.298	5.298								
DATA SCV2 "LCS01/OUT"	1 11	0	0.001	0.361	0.361	1.408	20.598	36.771	51.66	57.678	60.525	64.048				
DATA SCV2 "LDS04/OUT"	1 14	0	0.001	0.324	0.324	3.499	6.692	23.39	27.12	31.74	35.44	40.21	45.07	47.83	59.58	
DATA SCV2 "LCS01/11B1"	1 8	0	0.001	0.109	0.109	0.834	4.138	6.705	26.274							
DATA SCV2 "LDS01/02D6"	1 8	0	0.001	0.0957	0.0957	1.673	3.63	6.861	8.721							
DATA SCV2 "LCS01/06"	1 7	0	0.001	0.0491	0.0491	3.532	5.102	19.313								
DATA SCV2 "LCS01/07D2"	1 7	0	3.09	3.09	6	10	13.5	13.5								

DATA SCV2 "LCS01/06D2"	1	7	0	1.69	1.69	3.97	16.66	18	18							
DATA SCV3 "LCS01/OUT"	1	11	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5
DATA SCV3 "LDS04/OUT"	1	14	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5
DATA SCV3 "LCS01/11B1"	1	8	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5						
DATA SCV3 "LDS01/02D6"	1	8	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5						
DATA SCV3 "LCS01/06"	1	7	1.5	1.5	1.5	1.5	1.5	1.5	1.5							
DATA SCV3 "LCS01/07D2"	1	7	1.5	1.5	1.5	1.5	1.5	1.5	1.5							
DATA SCV3 "LCS01/06D2"	1	7	1.5	1.5	1.5	1.5	1.5	1.5	1.5							
DATA SCV4 "LCS01/OUT"	1	11	0.1	0.1	1	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7			
DATA SCV4 "LDS04/OUT"	1	14	0.1	0.1	1	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
DATA SCV4 "LCS01/11B1"	1	28	0.1	0.1	1	1.7	1.7	1.7	1.7	1.7						
DATA SCV4 "LDS01/02D6"	1	8	0.1	0.1	1	1.7	1.7	1.7	1.7	1.7						
DATA SCV4 "LCS01/06"	1	7	0.1	0.1	1	1.7	1.7	1.7	1.7							
DATA SCV4 "LCS01/07D2"	1	7	0.1	1	1.7	1.7	1.7	1.7	1.7							
DATA SCV4 "LCS01/06D2"	1	7	0.1	1	1.7	1.7	1.7	1.7	1.7							

TABLE B5
Stormwater Drainage Details

Link Name	Upstream Node	Downstream Node	Pipe Length (m)	Pipe Diameter (mm)	No. of Barrels	Pipe n	Entry Loss
A- Elizabeth Street							
LAU01/02	NAU01/02	NAU01/04	158	450	1	0.013	4.5
LAU01/04	NAU01/04	NAU01/05	34	600	1	0.013	1
LAU01/04A	NAU01/04A	NAU01/04	7	450	2	0.013	4
LAU01/05	NAU01/05	NAU01/06	83	600	1	0.013	1
LAU01/05A	NAU01/05A	NAU01/05	5	600	1	0.013	4
LAU01/06	NAU01/06	NAU01/07	53	900	1	0.013	1.2
LAU01/06A	NAU01/06A	NAU01/06	5	600	2	0.013	4
LAU01/07	NAU01/07	NAU01/08	65	900	1	0.013	0.5
LAU01/07A	NAU01/07A	NAU01/07	6	375	1	0.013	4
LAU01/08	NAU01/08	NAU01/09	20	900	1	0.013	1.5
LAU01/08A	NAU01/08A	NAU01/08	10	375	2	0.013	4
LAU01/09	NAU01/09	NAU01/10	102	900	2	0.013	1
LAU01/09A	NAU01/09A	NAU01/09	6	900	2	0.013	4
LAU01/10	NAU01/10	NAU01/11	85	900	3	0.013	1
LAU01/10A	NAU01/10A	NAU01/10	12	450	2	0.013	4
LAU01/11	NAU01/11	NAU01/12	22	1200	3	0.013	0.5
LAU01/11A	NAU01/11A	NAU01/11	8	600	3	0.013	4
LAU01/12	NAU01/12	NAUOUT1	25	1200	3	0.013	2
LAU01/12A	NAU01/12A	NAU01/12	10	375	2	0.013	4
LAU03/03	NAU03/03	NAU01/06	69	375	1	0.013	5.25
LAU05/02	NAU05/02	NAU05/04	68	375	1	0.013	4
LAU05/04	NAU05/04	NAU05/06	75	375	1	0.013	3.5
LAU05/05	NAU05/05	NAU01/08	104	375	1	0.013	6.5
LAU05/06	NAU05/06	NAU01/08	80	375	1	0.013	1
LAU08/02	NAU08/02	NAU08/03	50	300	1	0.013	4
LAU08/03	NAU08/03	NAU01/10	82	600	1	0.013	6
LAU07/02	NAU07/02	NAU07/03	48	450	1	0.013	9
LAU07/03	NAU07/03	NAU06/01	121	450	1	0.013	2.5
LAU12/02	NAU12/02	NAUOUT2	38	300	1	0.013	4
B- Pier Street							
LBU06/01	NAU06/01	NBU1	14	450	1	0.013	1
LBU1	NBU1	NBUOUT1	30	300	1	0.013	0.2
LBU05/01	NBU05/01	NBU04/02	57	375	1	0.013	6.5
LBU04/02	NBU04/02	NBU01/12	130	375	1	0.013	0.5
LBU01/12	NBU01/12	NBUOUT2	33.5	2.1x0.6	2	0.013	2.5
LBU01/12A	NBU01/12A	NBU01/12	11.1	825	3	0.013	4
LBU01/04	NBU01/04	NBU01/05	127	600	1	0.013	1.5
LBU01/04A	NBU01/04A	NBU01/04	8	600	2	0.013	5.25
LBU01/05	NBU01/05	NBU01/06	77	750	1	0.013	1
LBU01/05A	NBU01/05A	NBU01/05	5	600	3	0.013	4
LBU01/06	NBU01/06	NBU01/07	46	750	1	0.013	0.2
LBU01/07	NBU01/07	NBU01/08	54	900	1	0.013	2.5
LBU01/07A	NBU01/07A	NBU01/07	15	375	3	0.013	4
LBU01/08	NBU01/08	NBU01/10	125	1200	1	0.013	1
LBU01/08A	NBU01/08A	NBU01/08	6	750	2	0.013	4
LBU01/10	NBU01/10	NBU01/11	73	1200	1	0.013	0.5
LBU01/10A	NBU01/10A	NBU01/10	6	450	1	0.013	4
LBU01/11	NBU01/11	NBU01/12	123	900	2	0.013	3.5
LBU01/11A	NBU01/11A	NBU01/11	8	600	2	0.013	4
LBU01/12LF	NBU01/12	NBUOUT2A	45.2	750	1	0.013	4
C- Pilot Street							
LCU01/04	NCU01/04	NCU01/05	57	600	1	0.013	6.5
LCU01/05	NCU01/05	NCU01/06	62	600	1	0.013	1.5
LCU01/05A	NCU01/05A	NCU01/05	9	450	2	0.013	4
LCU01/06	NCU01/06	NCU01/07	34	1050	1	0.013	2.5
LCU01/06A	NCU01/06A	NCU01/06	10	450	2	0.013	4

Link Name	Upstream Node	Downstream Node	Pipe Length (m)	Pipe Diameter (mm)	No. of Barrels	Pipe n	Entry Loss
LCU01/07	NCU01/07	NCU01/08	27	1050	1	0.013	2.5
LCU01/08	NCU01/08	NCU01/09	83	1050	1	0.013	1
LCU01/09	NCU01/09	NCU01/11	114	1050	1	0.013	1
LCU01/09A	NCU01/09A	NCU01/09	15	375	3	0.013	6
LCU01/11	NCU01/11	NCUOUT	54	1050	1	0.013	4
LDU03/01	NDU03/01	NCU01/09	40	375	1	0.013	4
LCU04/02	NCU04/02	NCU1	49	300	1	0.013	6.5
LCU1	NCU1	NCUOUT2	30	600	1	0.013	2.5
LCU03/02	NCU03/02	NDU02/03	130	300	1	0.013	4.5
D- Guard Street							
LDU02/03	NDU02/03	NDU03/03	70	600	1	0.013	2.5
LDU03/03	NDU03/03	NDU07/01	79	600	1	0.013	3
LDU07/01	NDU07/01	NDU08/01	86	600	1	0.013	2.5
LDU08/01	NDU08/01	NDUOUT1	82	600	1	0.013	0.5
LDU02/02	NDU02/02	NDU02/03	67	300	1	0.013	6.5
LDU02/04	NDU02/04	NDU02/03	73	300	1	0.013	6.5
LDU01/02	NDU01/02	NDU01/03	56	375	2	0.013	10.5
LDU01/03	NDU01/03	NDU01/04	73	450	1	0.013	1
LDU01/04	NDU01/04	NDU01/05	36	825	1	0.013	0.5
LDU01/04A	NDU01/04A	NDU01/04	17	375	2	0.013	4
LDU01/05	NDU01/05	NDU01/06	49	1200	2	0.013	1
LDU01/05A	NDU01/05A	NDU01/05	20	1200	2	0.013	4
LDU01/06	NDU01/06	NDU01/07	59	1200	2	0.013	1
LDU01/06A	NDU01/06A	NDU01/06	20	450	2	0.013	4
LDU01/07	NDU01/07	NDUOUT2	90	1200	2	0.013	2
LDU01/07A	NDU01/07A	NDU01/07	13	450	2	0.013	4
LDU06/03	NDU06/03	NDU1	110	375	1	0.013	6
LDU1	NDU1	NDU01/05	18	375	1	0.013	2.5
LDU05/01	NDU05/01	NDU04/02	40	450	1	0.013	6.5
LDU04/02	NDU04/02	NDU04/03	80	375	1	0.013	2.5
LDU04/03	NDU04/03	NDU04/04	44	375	1	0.013	1
LDU04/03A	NDU04/03A	NDU04/03	25	375	1	0.013	4
LDU04/04	NDU04/04	NDU04/05	67	375	1	0.013	2.5
LDU04/04A	NDU04/04A	NDU04/04	7	375	1	0.013	4
LDU04/05	NDU04/05	NDUOUT3	80	375	1	0.013	2.5
LDU04/05A	NDU04/05A	NDU04/05	18	200	1	0.013	6.5
LDU04/01	NDU04/01	NDU04/02	25	375	1	0.013	6.5

Note: Shaded Links represent new drainage lines

TABLE B6
Adopted Invert Levels for Underground Nodes

Node Name	Invert Level m AHD
A-Elizabeth Street	
NAU01/02	2.100
NAU01/04	1.800
NAU01/04A	2.000
NAU01/05	1.600
NAU01/05A	1.800
NAU01/06	1.500
NAU01/06A	1.700
NAU01/07	1.290
NAU01/07A	1.500
NAU01/08	1.020
NAU01/08A	1.200
NAU01/09	0.940
NAU01/09A	1.200
NAU01/10	0.530
NAU01/10A	0.700
NAU01/11	0.190
NAU01/11A	0.400
NAU01/12	0.100
NAU01/12A	0.300
NAUOUT1	0.000
NAU03/03	2.500
NAU05/02	3.032
NAU05/04	2.690
NAU05/05	2.672
NAU05/06	2.680
NAU08/02	2.562
NAU08/03	2.291
NAU07/02	2.165
NAU07/03	2.021
NAU06/01	1.529
NAU12/02	2.113
NAUOUT2	0.744
B- Pier Street	
NBU1	0.378
NBUOUT1	0.350
NBU05/01	2.300
NBU04/02	1.853
NBU01/12	0.190
NBU01/12A	0.250
NBUOUT2	1.100
NBUOUT2A	0.100
NBU01/04	2.000
NBU01/04A	2.200
NBU01/05	1.800
NBU01/05A	2.200
NBU01/06	1.600
NBU01/07	1.500
NBU01/07A	2.200
NBU01/08	1.000
NBU01/08A	1.300
NBU01/10	0.700
NBU01/10A	1.000
NBU01/11	0.500
NBU01/11A	0.800
C- Pilot Street	
NCU01/04	2.979
NCU01/05	2.527
NCU01/05A	2.554
NCU01/06	1.583
NCU01/06A	1.613
NCU01/07	1.310
NCU01/08	1.270
NCU01/09	0.850

Node Name	Invert Level m AHD
NCU01/09A	2.230
NCU01/11	0.270
NCUOUT	0.000
NCUOUT2	0.000
NDU03/01	2.241
NCU04/02	2.370
NCU1	1.771
NCU03/02	2.970
D- Guard Street	
NDU02/03	1.954
NDU03/03	1.602
NDU07/01	1.222
NDU08/01	1.160
NDUOUT1	0.900
NDU02/02	2.387
NDU02/04	2.511
NDU01/02	4.081
NDU01/03	1.800
NDU01/04	0.500
NDU01/04A	0.551
NDU01/05	0.260
NDU01/05A	0.300
NDU01/06	0.211
NDU01/06A	0.291
NDU01/07	0.152
NDU01/07A	0.178
NDUOUT2	0.050
NDU06/03	2.054
NDU1	1.270
NDU05/01	9.571
NDU04/02	8.950
NDU04/03	4.533
NDU04/03A	4.733
NDU04/04	2.821
NDU04/04A	2.921
NDU04/05	1.845
NDU04/05A	1.890
NDUOUT3	0.900
NDU04/01	10.173

Note: Shaded entries represent new drainage lines

TABLE B7
Adopted Manhole Loss Coefficients



STRUCTURE SCHEMATIC DIAGRAM	APPLICABLE CHARTS		STRUCTURE SCHEMATIC DIAGRAM	APPLICABLE CHARTS		STRUCTURE SCHEMATIC DIAGRAM	APPLICABLE CHARTS		STRUCTURE SCHEMATIC DIAGRAM	APPLICABLE CHARTS	
	Ku	Kw		Ku	Kw		Ku	Kw		Ku	Kw
Kg=Kw	32	32	Ku≠Kw 2.5	38	39	Ku=Kw 2.5	49	49	58 N/A		
Kg=Kw	32	32	Ku≠Kw 2.5	40	41	Ku=Kw 1.5, Ku≠Kw 2.5	50	50	0.47, 0.25, 0.34, 0.12		
Ku=Kw	33	33	Ku=Kw 2.5	42	43	Ku=Kw 2.5	51	51	0.15		
Ku=Kw	34	34	Ku=Kw 2.5	44	45	Ku=Kw 0.5, 1.5, 2.0	52	52	0.20		
Ku=Kw	35	36	Ku=Kw 2.4, 46, 47	46	47	Ku=Kw 53, 53	53	53	0.58		
Ku=Kw	37	37	Ku=Kw 2.0, 2.5	48	48	Ku=Kw 54, N/A	54	N/A	0.58		
Ku=Kw	37	37	Ku=Kw 3.5	49	49	Ku=Kw 55, N/A	55	N/A	0.59		

□ DENOTES STRUCTURE WITH GRATE (GUTTER) INFLOW

○ DENOTES STRUCTURE WITH NO GRATE (GUTTER) INFLOW

Index to Pressure Change Coefficient Charts

CHART NO. 31

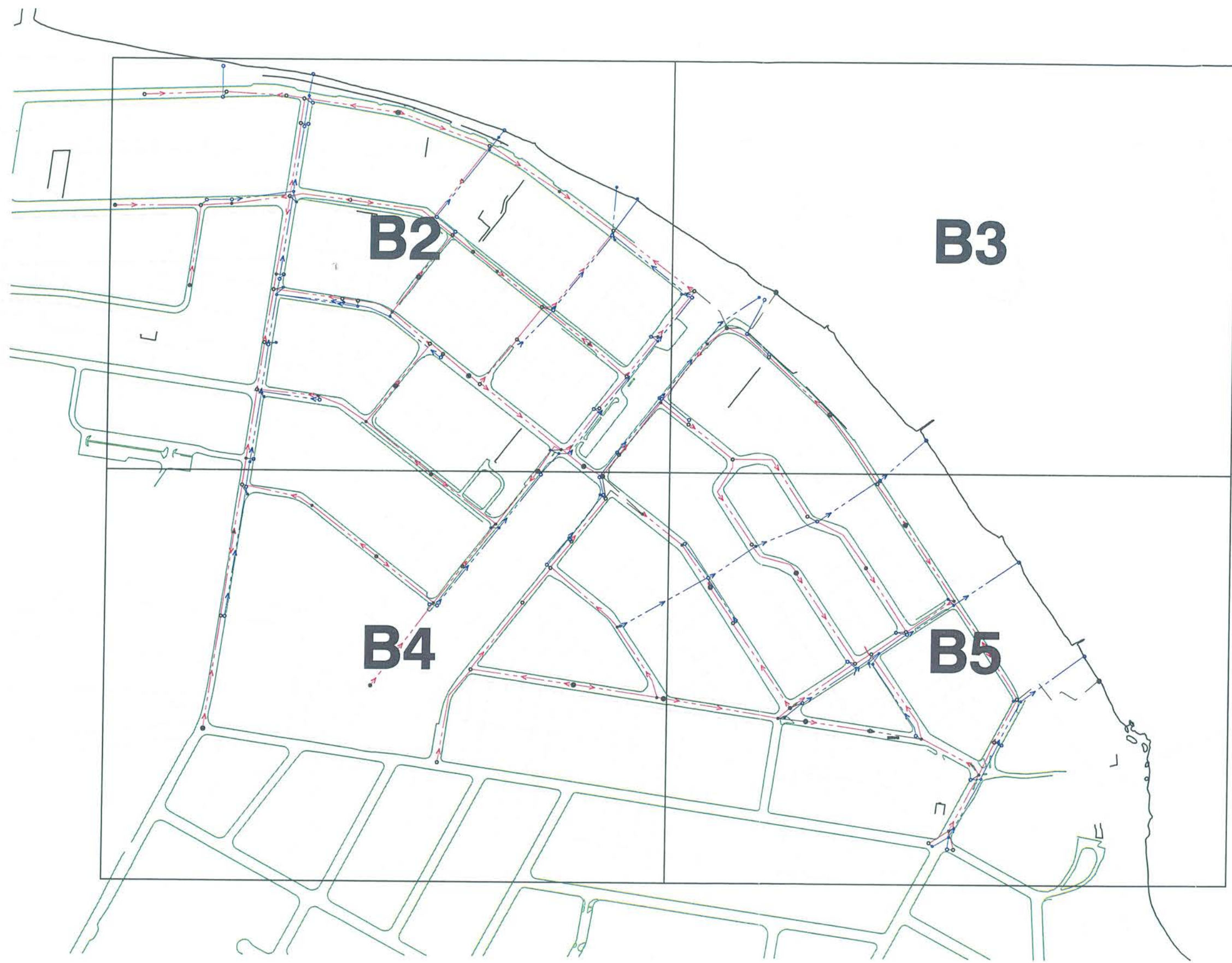
TABLE B8
Inlet Capacity of Side Entry Pits and Grates

Depth	1 Bay Side Entry Pit		2 Bay Side Entry Pit		3 Bay Side Entry Pit		4 Bay Side Entry Pit		Grated	
	On grade (m ³ /s)	Sag (m ³ /s)	On grade (m ³ /s)	Sag (m ³ /s)	On grade (m ³ /s)	Sag (m ³ /s)	On grade (m ³ /s)	Sag (m ³ /s)	On grade (m ³ /s)	Sag (m ³ /s)
0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
0.080	0.007	0.027	0.020	0.054	0.020	0.081	0.020	0.108	0.013	0.020
0.099	0.010	0.037	0.031	0.074	0.038	0.112	0.040	0.149	0.020	0.027
0.113	0.014	0.045	0.038	0.091	0.053	0.136	0.060	0.182	0.027	0.033
0.125	0.016	0.053	0.044	0.106	0.062	0.158	0.074	0.211	0.032	0.038
0.144	0.021	0.065	0.050	0.131	0.077	0.196	0.101	0.261	0.039	0.047
0.158	0.022	0.075	0.053	0.150	0.086	0.225	0.122	0.300	0.043	0.054
0.170	0.023	0.084	0.055	0.168	0.093	0.251	0.142	0.335	0.045	0.061
0.179	0.023	0.090	0.056	0.180	0.098	0.269	0.160	0.359	0.045	0.065
0.189	0.023	0.092	0.056	0.184	0.101	0.276	0.176	0.368	0.045	0.071
0.198	0.023	0.094	0.056	0.188	0.102	0.282	0.191	0.376	0.045	0.076
0.205	0.023	0.096	0.056	0.191	0.104	0.286	0.202	0.380	0.045	0.080
0.213	0.023	0.097	0.056	0.195	0.104	0.291	0.210	0.380	0.045	0.085
0.219	0.023	0.099	0.056	0.197	0.104	0.295	0.216	0.380	0.045	0.088
0.250	0.023	0.105	0.056	0.210	0.104	0.300	0.216	0.380	0.045	0.108
0.300	0.023	0.115	0.056	0.229	0.104	0.300	0.216	0.380	0.045	0.142
0.350	0.023	0.123	0.056	0.247	0.104	0.300	0.216	0.380	0.045	0.157
0.400	0.023	0.131	0.056	0.248	0.104	0.300	0.216	0.380	0.045	0.168
0.500	0.023	0.146	0.056	0.248	0.104	0.300	0.216	0.380	0.045	0.188
0.750	0.023	0.179	0.056	0.248	0.104	0.300	0.216	0.380	0.045	0.230
1.000	0.023	0.206	0.056	0.248	0.104	0.300	0.216	0.380	0.045	0.250
1.500	0.023	0.216	0.056	0.248	0.104	0.300	0.216	0.380	0.045	0.250
2.000	0.023	0.216	0.056	0.248	0.104	0.300	0.216	0.380	0.045	0.250
3.000	0.023	0.216	0.056	0.248	0.104	0.300	0.216	0.380	0.045	0.250
3.500	0.023	0.216	0.056	0.248	0.104	0.300	0.216	0.380	0.045	0.250
4.000	0.023	0.216	0.056	0.248	0.104	0.300	0.216	0.380	0.045	0.250
5.000	0.023	0.216	0.056	0.248	0.104	0.300	0.216	0.380	0.045	0.250

Note Allowable values quoted

TABLE B9
Equivalent Weir Dimensions for Inlet Pits

Depth	1 Bay Side Entry Pit		2 Bay Side Entry Pit		3 Bay Side Entry Pit		4 Bay Side Entry Pit		Grated	
	On grade (m)	Sag (m)	On grade (m)	Sag (m)	On grade (m)	Sag (m)	On grade (m)	Sag (m)	On grade (m)	Sag (m)
0.000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
0.080	0.0070	0.0216	0.0253	0.0431	0.0221	0.0630	0.0205	0.0808	0.0122	0.0278
0.099	0.0097	0.0298	0.0371	0.0594	0.0397	0.0869	0.0384	0.1115	0.0182	0.0352
0.113	0.0124	0.0364	0.0438	0.0725	0.0525	0.1060	0.0553	0.1361	0.0232	0.0409
0.125	0.0143	0.0423	0.0487	0.0844	0.0602	0.1235	0.0665	0.1585	0.0272	0.0461
0.144	0.0180	0.0524	0.0526	0.1044	0.0710	0.1528	0.0863	0.1962	0.0318	0.0553
0.158	0.0191	0.0603	0.0545	0.1201	0.0777	0.1758	0.1019	0.2257	0.0337	0.0628
0.170	0.0195	0.0673	0.0557	0.1341	0.0816	0.1963	0.1154	0.2521	0.0345	0.0698
0.179	0.0193	0.0723	0.0557	0.1440	0.0846	0.2100	0.1286	0.2700	0.0344	0.0754
0.189	0.0190	0.0741	0.0554	0.1477	0.0869	0.2154	0.1407	0.2770	0.0342	0.0819
0.198	0.0189	0.0758	0.0552	0.1510	0.0879	0.2202	0.1522	0.2831	0.0341	0.0879
0.205	0.0187	0.0770	0.0550	0.1535	0.0890	0.2238	0.1606	0.2864	0.0340	0.0927
0.213	0.0186	0.0784	0.0548	0.1563	0.0887	0.2279	0.1657	0.2865	0.0340	0.0982
0.219	0.0185	0.0795	0.0547	0.1583	0.0884	0.2310	0.1703	0.2865	0.0340	0.1023
0.250	0.0180	0.0846	0.0540	0.1686	0.0873	0.2349	0.1698	0.2868	0.0340	0.1248
0.300	0.0174	0.0924	0.0530	0.1841	0.0858	0.2352	0.1697	0.2871	0.0340	0.1641
0.350	0.0168	0.0995	0.0523	0.1984	0.0845	0.2355	0.1697	0.2875	0.0340	0.1819
0.400	0.0164	0.1062	0.0516	0.1997	0.0844	0.2357	0.1696	0.2877	0.0340	0.1945
0.500	0.0157	0.1185	0.0505	0.2000	0.0844	0.2361	0.1696	0.2882	0.0339	0.2175
0.750	0.0145	0.1449	0.0485	0.2006	0.0844	0.2368	0.1695	0.2891	0.0339	0.2664
1.000	0.0136	0.1673	0.0471	0.2010	0.0843	0.2373	0.1694	0.2897	0.0339	0.2893
1.500	0.0126	0.1762	0.0466	0.2017	0.0843	0.2381	0.1693	0.2906	0.0339	0.2894
2.000	0.0119	0.1766	0.0466	0.2021	0.0842	0.2386	0.1692	0.2913	0.0339	0.2894
3.000	0.0110	0.1771	0.0466	0.2028	0.0842	0.2394	0.1691	0.2922	0.0339	0.2894
3.500	0.0106	0.1774	0.0466	0.2030	0.0842	0.2397	0.1691	0.2926	0.0338	0.2895
4.000	0.0103	0.1775	0.0465	0.2032	0.0841	0.2399	0.1690	0.2929	0.0338	0.2895
5.000	0.0099	0.1778	0.0465	0.2036	0.0841	0.2403	0.1690	0.2934	0.0338	0.2895

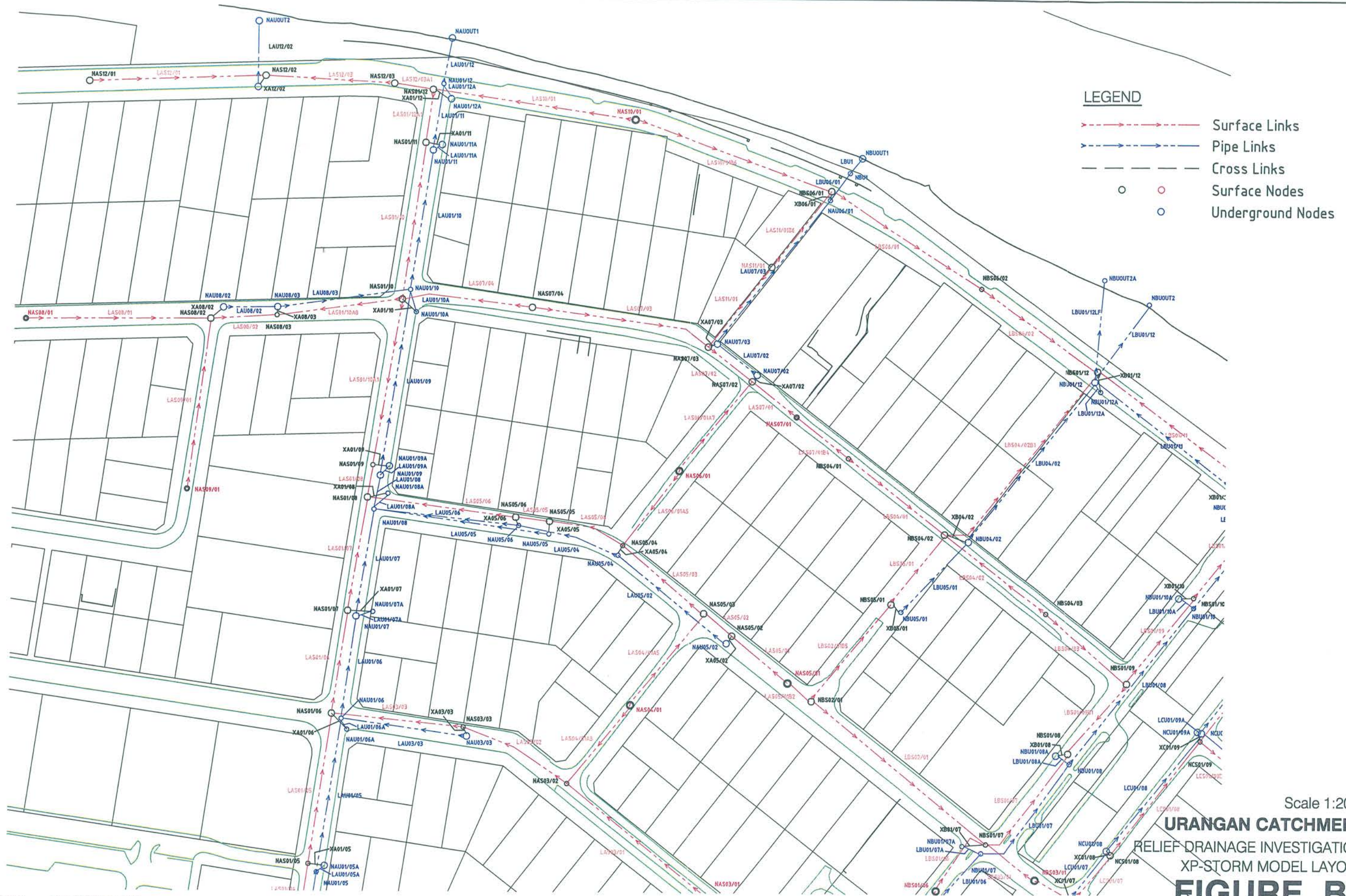


- LEGEND**
- - - - - Surface Links
 - - - - - Pipe Links
 - - - - - Cross Links
 - Surface Nodes
 - Underground Nodes

Scale 1:5000

URANGAN CATCHMENT
RELIEF DRAINAGE INVESTIGATION
XP-STORM MODEL LAYOUT

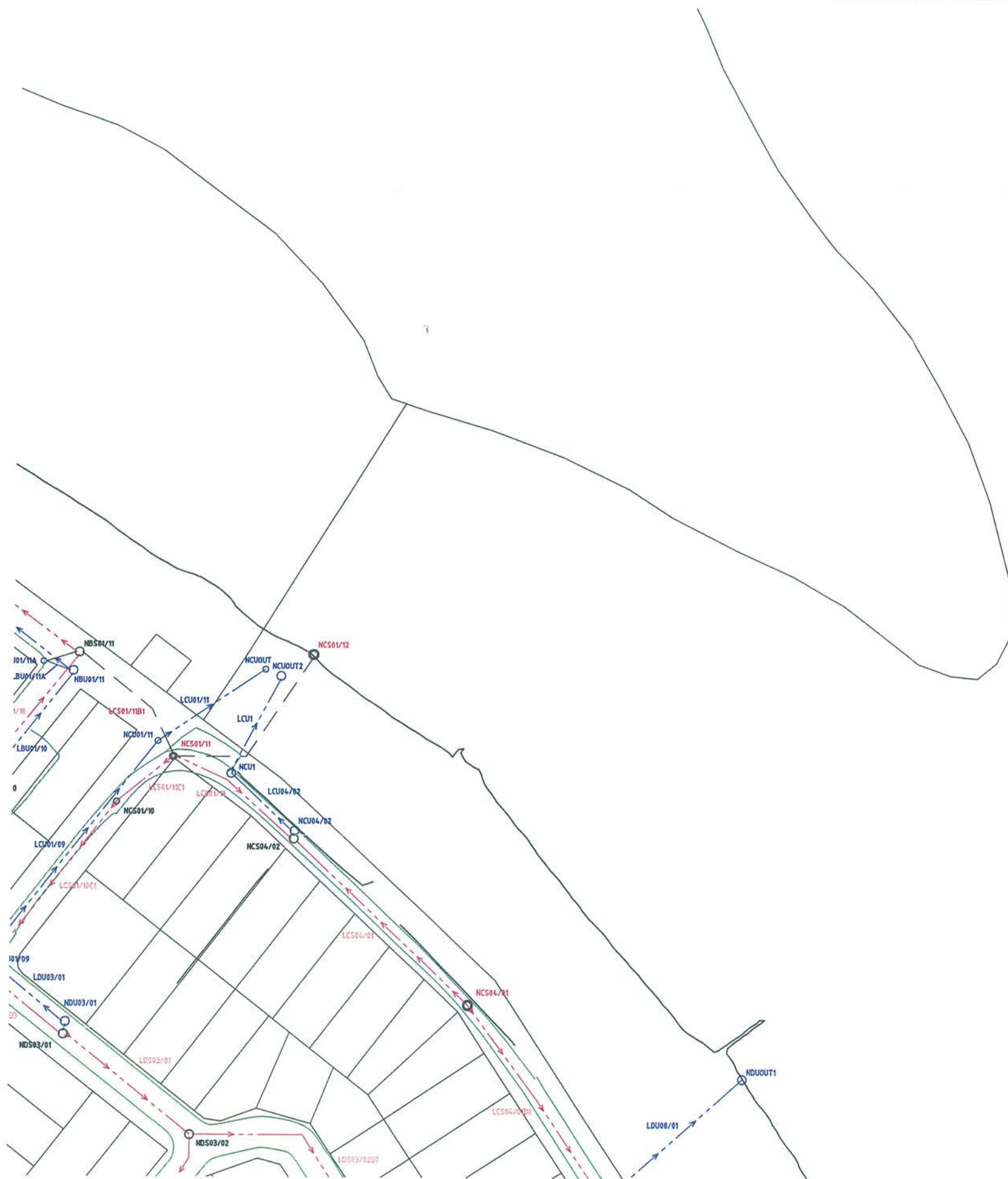
FIGURE B1



LEGEND

- - - - - Surface Links
- - - - - Pipe Links
- - - - - Cross Links
- Surface Nodes
- Underground Nodes

Scale 1:2000
URANGAN CATCHMENT
RELIEF DRAINAGE INVESTIGATION
XP-STORM MODEL LAYOUT
FIGURE B2



LEGEND






- - - - - Surface Links
- - - - - Pipe Links
- - - - - Cross Links
- Surface Nodes
- Underground Nodes

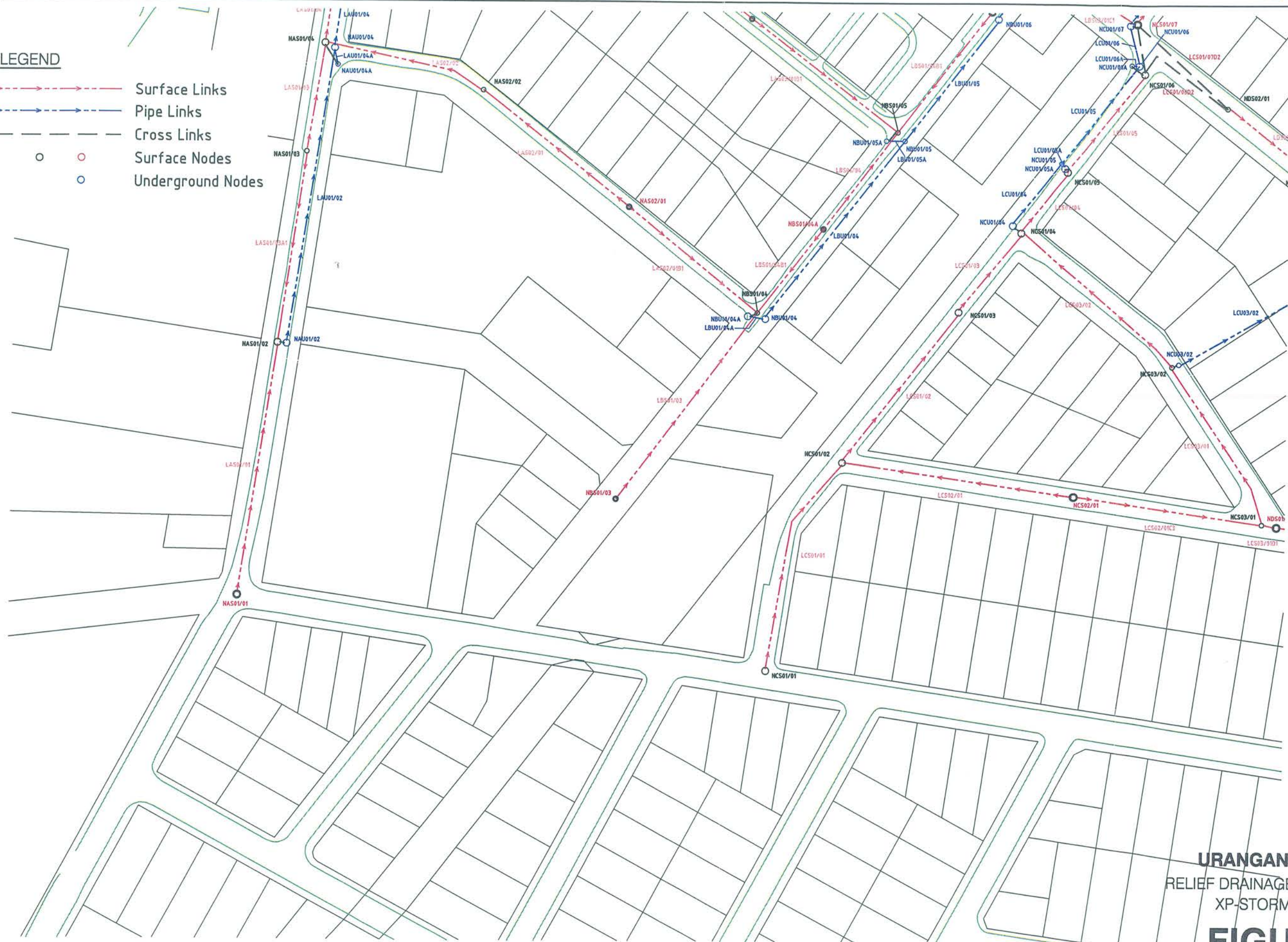
Scale 1:2000

URANGAN CATCHMENT
RELIEF DRAINAGE INVESTIGATION
XP-STORM MODEL LAYOUT

FIGURE B3

LEGEND

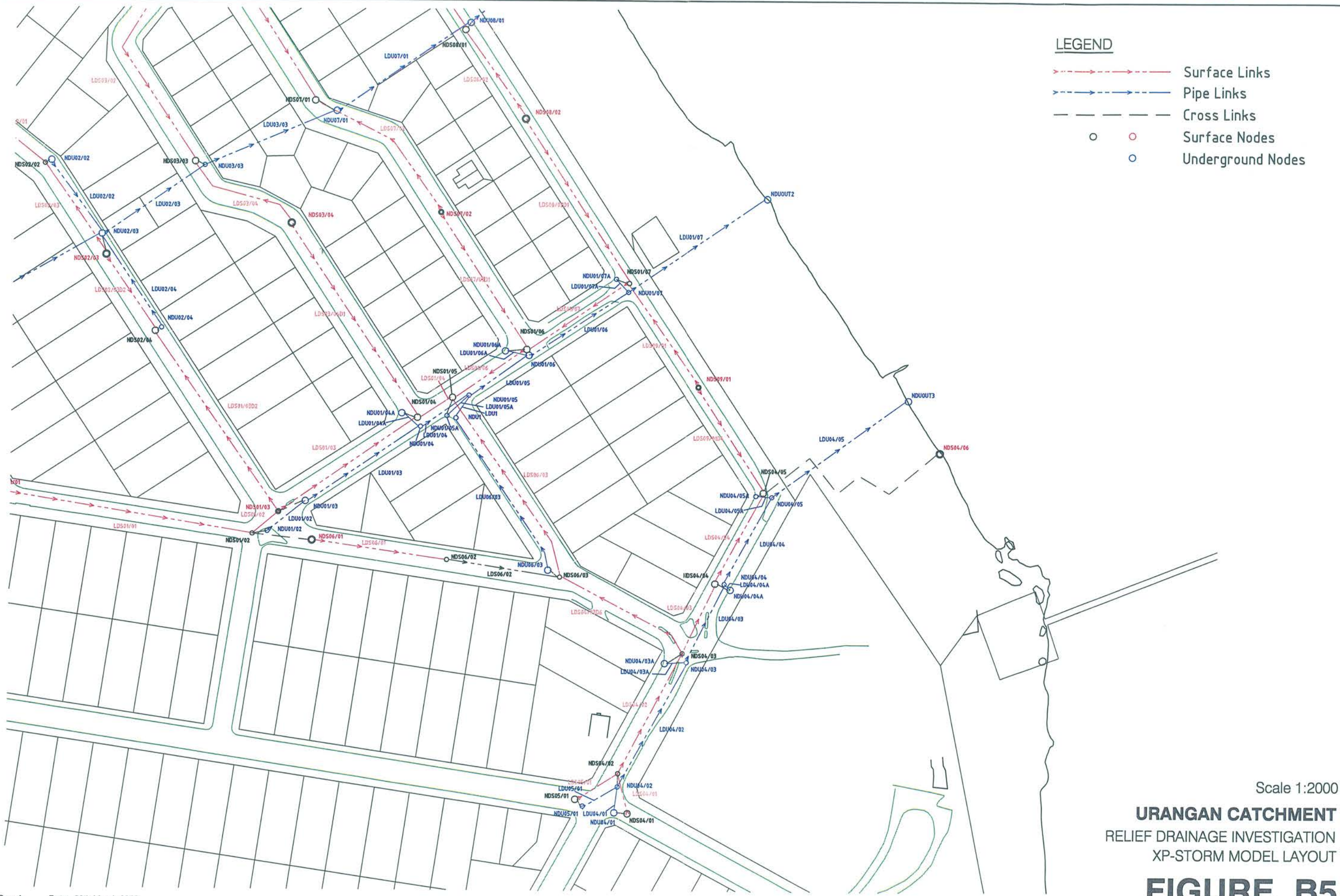
-  Surface Links
-  Pipe Links
-  Cross Links
-  Surface Nodes
-  Underground Nodes



Scale 1:2000

URANGAN CATCHMENT
RELIEF DRAINAGE INVESTIGATION
XP-STORM MODEL LAYOUT

FIGURE B4



LEGEND

- - - - - Surface Links
- - - - - Pipe Links
- - - - - Cross Links
- Surface Nodes
- Underground Nodes

Scale 1:2000

URANGAN CATCHMENT
RELIEF DRAINAGE INVESTIGATION
XP-STORM MODEL LAYOUT

FIGURE B5

APPENDIX C

Model Results

List of Tables

Table C1	Surface Network- Peak Water Levels and Depths	C1-C2
Table C2	Peak Water Levels- Surface Network- 2 Year Event	C3-C4
Table C3	Peak Water Levels- Underground Network- 2 Year Event	C5-C6
Table C4	Peak Flows and Velocities- Surface Network- 2 Year Event	C7-C9
Table C5	Peak Flows and Velocities- Underground Network- 2 Year Event	C10-C11
Table C6	Peak Water Levels- Surface Network- 10 Year Event	C12-C13
Table C7	Peak Water Levels- Underground Network- 10 Year Event	C14-C15
Table C8	Peak Flows and Velocities- Surface Network- 10 Year Event	C16-C18
Table C9	Peak Flows and Velocities- Underground Network- 10 Year Event	C19-C20
Table C10	Peak Water Levels- Surface Network- 20 Year Event	C21-C22
Table C11	Peak Water Levels- Underground Network- 20 Year Event	C23-C24
Table C12	Peak Flows and Velocities- Surface Network- 20 Year Event	C25-C27
Table C13	Peak Flows and Velocities- Underground Network- 20 Year Event	C28-C29
Table C14	Peak Water Levels- Surface Network- 50 Year Event	C30-C31
Table C15	Peak Water Levels- Underground Network- 50 Year Event	C32-C33
Table C16	Peak Flows and Velocities- Surface Network- 50 Year Event	C34-C36
Table C17	Peak Flows and Velocities- Underground Network- 50 Year Event	C37-C38
Table C18	Peak Water Levels- Surface Network- 100 Year Event	C39-C40
Table C19	Peak Water Levels- Underground Network- 100 Year Event	C41-C42
Table C20	Peak Flows and Velocities- Surface Network- 100 Year Event	C43-C45
Table C21	Peak Flows and Velocities- Underground Network- 100 Year Event	C46-C47

TABLE C1
Surface Network- Peak Water Levels and Depths

Node Name	Invert Level (m AHD)	Maximum Water Level (m AHD)					Maximum Depth (m)				
		2 Year Event	10 Year Event	20 Year Event	50 Year Event	100 Year Event	2 Year Event	10 Year Event	20 Year Event	50 Year Event	100 Year Event
A- Elizabeth Street											
NAS01/01	4.655	4.655	4.655	4.655	4.655	4.655	0.000	0.000	0.000	0.000	0.000
NAS01/02	4.000	4.037	4.048	4.052	4.072	4.086	0.037	0.048	0.052	0.072	0.086
NAS01/03	4.194	4.194	4.194	4.194	4.194	4.194	0.000	0.000	0.000	0.000	0.000
NAS01/04	3.800	3.909	3.937	3.966	4.002	4.021	0.109	0.137	0.166	0.202	0.221
NAS01/05	3.651	3.718	3.741	3.832	3.888	3.910	0.067	0.090	0.181	0.237	0.259
NAS01/06	3.610	3.719	3.746	3.813	3.870	3.892	0.109	0.136	0.203	0.260	0.282
NAS01/07	3.323	3.390	3.414	3.456	3.495	3.517	0.067	0.091	0.133	0.172	0.194
NAS01/08	3.003	3.102	3.133	3.228	3.332	3.377	0.099	0.130	0.225	0.329	0.374
NAS01/09	2.799	2.886	2.917	3.230	3.333	3.378	0.087	0.118	0.431	0.534	0.579
NAS01/10	3.167	3.212	3.227	3.234	3.315	3.353	0.045	0.060	0.067	0.148	0.186
NAS01/11	2.537	2.617	2.644	2.673	2.921	3.116	0.080	0.107	0.136	0.384	0.579
NAS01/12	2.836	2.928	2.986	3.018	3.027	3.114	0.092	0.150	0.182	0.191	0.278
NAS02/01	4.745	4.745	4.745	4.745	4.773	4.795	0.000	0.000	0.000	0.028	0.050
NAS02/02	4.377	4.437	4.458	4.467	4.483	4.492	0.060	0.081	0.090	0.106	0.115
NAS03/01	4.368	4.368	4.368	4.368	4.424	4.454	0.000	0.000	0.000	0.056	0.086
NAS03/02	4.110	4.219	4.252	4.265	4.283	4.295	0.109	0.142	0.155	0.173	0.185
NAS03/03	3.943	4.042	4.072	4.089	4.116	4.131	0.099	0.129	0.146	0.173	0.188
NAS04/01	4.373	4.373	4.373	4.373	4.373	4.373	0.000	0.000	0.000	0.000	0.000
NAS05/01	4.310	4.318	4.325	4.327	4.331	4.334	0.008	0.015	0.017	0.021	0.024
NAS05/02	4.180	4.265	4.293	4.304	4.314	4.320	0.084	0.113	0.124	0.134	0.140
NAS05/03	4.100	4.165	4.187	4.198	4.217	4.225	0.065	0.087	0.098	0.117	0.125
NAS05/04	3.800	3.888	3.916	3.927	3.940	3.949	0.088	0.116	0.127	0.140	0.149
NAS05/05	3.602	3.725	3.748	3.758	3.778	3.790	0.123	0.146	0.156	0.176	0.188
NAS05/06	3.450	3.536	3.570	3.584	3.604	3.615	0.086	0.120	0.134	0.154	0.165
NAS06/01	4.066	4.066	4.066	4.066	4.066	4.066	0.000	0.000	0.000	0.000	0.000
NAS07/01	3.270	3.308	3.323	3.364	3.458	3.498	0.037	0.052	0.094	0.188	0.228
NAS07/02	2.847	3.119	3.260	3.364	3.456	3.497	0.272	0.413	0.517	0.609	0.650
NAS07/03	2.971	3.119	3.260	3.364	3.457	3.497	0.148	0.289	0.393	0.486	0.526
NAS07/04	3.426	3.426	3.426	3.426	3.457	3.496	0.000	0.000	0.000	0.031	0.070
NAS08/01	3.833	3.833	3.833	3.833	3.833	3.833	0.000	0.000	0.000	0.000	0.000
NAS08/02	3.202	3.323	3.350	3.354	3.369	3.379	0.121	0.148	0.152	0.167	0.177
NAS08/03	2.900	2.985	3.020	3.171	3.323	3.357	0.085	0.120	0.271	0.423	0.457
NAS09/01	3.917	3.917	3.917	3.917	3.917	3.917	0.000	0.000	0.000	0.000	0.000
NAS10/01	3.288	3.288	3.288	3.288	3.288	3.288	0.000	0.000	0.000	0.000	0.000
NAS11/01	3.506	3.506	3.506	3.506	3.506	3.506	0.000	0.000	0.000	0.000	0.000
NAS12/01	3.250	3.250	3.250	3.250	3.250	3.250	0.000	0.000	0.000	0.000	0.000
NAS12/02	2.722	3.004	3.093	3.131	3.166	3.189	0.282	0.371	0.409	0.444	0.467
NAS12/03	2.935	3.000	3.073	3.100	3.126	3.144	0.065	0.138	0.165	0.191	0.209
B- Pier Street											
NBS01/03	4.915	4.915	4.915	4.915	4.915	4.915	0.000	0.000	0.000	0.000	0.000
NBS01/04	4.515	4.607	4.638	4.704	4.773	4.798	0.092	0.123	0.189	0.258	0.283
NBS01/04A	4.715	4.715	4.715	4.715	4.765	4.786	0.000	0.000	0.000	0.050	0.071
NBS01/05	4.215	4.315	4.348	4.365	4.434	4.472	0.100	0.133	0.150	0.219	0.257
NBS01/06	4.415	4.415	4.415	4.415	4.433	4.458	0.000	0.000	0.000	0.018	0.043
NBS01/07	3.755	3.838	3.864	3.875	3.886	3.892	0.083	0.109	0.120	0.131	0.137
NBS01/08	3.080	3.162	3.190	3.202	3.289	3.358	0.082	0.110	0.122	0.209	0.278
NBS01/09	3.406	3.496	3.528	3.536	3.543	3.546	0.090	0.122	0.130	0.137	0.140
NBS01/10	2.827	2.911	2.935	2.948	2.966	2.980	0.084	0.108	0.121	0.139	0.153
NBS01/11	2.357	2.454	2.477	2.722	2.818	2.877	0.097	0.120	0.365	0.461	0.520
NBS01/12	2.036	2.154	2.203	2.720	2.816	2.874	0.118	0.167	0.684	0.780	0.838
NBS02/01	4.300	4.329	4.341	4.345	4.352	4.356	0.029	0.041	0.045	0.052	0.056
NBS03/01	4.090	4.090	4.090	4.090	4.090	4.090	0.000	0.000	0.000	0.000	0.000
NBS04/01	3.620	3.620	3.620	3.620	3.620	3.620	0.000	0.000	0.000	0.000	0.000
NBS04/02	3.173	3.362	3.433	3.431	3.462	3.479	0.189	0.260	0.258	0.289	0.306
NBS04/03	3.502	3.502	3.521	3.526	3.531	3.534	0.000	0.018	0.024	0.029	0.032
NBS05/01	4.040	4.076	4.089	4.095	4.104	4.109	0.036	0.049	0.055	0.064	0.069
NBS06/01	2.829	2.934	2.963	2.979	2.996	3.006	0.105	0.134	0.150	0.167	0.177
NBS06/02	2.477	2.665	2.697	2.720	2.816	2.874	0.188	0.220	0.243	0.339	0.397
C- Pilot Street											
NCS01/01	6.275	6.450	6.497	6.517	6.533	6.539	0.175	0.222	0.242	0.258	0.264

Node Name	Invert Level (m AHD)	Maximum Water Level (m AHD)					Maximum Depth (m)				
		2 Year Event	10 Year Event	20 Year Event	50 Year Event	100 Year Event	2 Year Event	10 Year Event	20 Year Event	50 Year Event	100 Year Event
NCS01/02	5.088	5.269	5.306	5.322	5.345	5.358	0.181	0.218	0.234	0.257	0.269
NCS01/03	4.607	4.740	4.764	4.775	4.795	4.806	0.133	0.157	0.168	0.188	0.199
NCS01/04	3.589	3.829	3.889	3.928	3.976	4.004	0.240	0.300	0.339	0.387	0.415
NCS01/05	3.500	3.704	3.822	3.871	3.921	3.950	0.204	0.322	0.371	0.421	0.450
NCS01/06	3.452	3.539	3.603	3.644	3.698	3.733	0.087	0.151	0.192	0.246	0.281
NCS01/07	3.403	3.441	3.534	3.599	3.673	3.711	0.038	0.131	0.196	0.270	0.308
NCS01/08	3.134	3.220	3.332	3.597	3.673	3.711	0.086	0.198	0.463	0.539	0.577
NCS01/09	3.050	3.112	3.131	3.150	3.185	3.200	0.062	0.081	0.100	0.135	0.150
NCS01/10	3.366	3.448	3.478	3.492	3.508	3.518	0.082	0.112	0.126	0.142	0.152
NCS01/11	3.665	3.665	3.665	3.665	3.665	3.665	0.000	0.000	0.000	0.000	0.000
NCS01/12	0.544	1.500	1.500	2.400	2.400	2.400	0.956	0.956	1.856	1.856	1.856
NCS02/01	5.750	5.750	5.750	5.750	5.750	5.750	0.000	0.000	0.000	0.000	0.000
NCS03/01	5.520	5.558	5.572	5.578	5.588	5.595	0.038	0.052	0.058	0.068	0.075
NCS03/02	3.600	3.891	3.972	4.002	4.042	4.066	0.291	0.372	0.402	0.442	0.466
NCS04/01	3.447	3.447	3.482	3.498	3.539	3.563	0.000	0.035	0.051	0.092	0.116
NCS04/02	3.175	3.370	3.482	3.499	3.541	3.566	0.195	0.307	0.324	0.366	0.391
D- Guard Street											
NDS01/01	5.500	5.541	5.554	5.559	5.581	5.601	0.041	0.054	0.059	0.081	0.101
NDS01/02	5.181	5.336	5.456	5.506	5.571	5.598	0.155	0.275	0.325	0.390	0.417
NDS01/03	3.339	3.396	3.441	3.475	3.502	3.520	0.057	0.102	0.136	0.163	0.181
NDS01/04	2.159	2.265	2.361	2.798	2.906	2.970	0.106	0.202	0.639	0.747	0.811
NDS01/05	1.900	2.040	2.259	2.798	2.906	2.970	0.140	0.359	0.898	1.006	1.070
NDS01/06	2.100	2.198	2.278	2.798	2.907	2.971	0.098	0.178	0.698	0.807	0.871
NDS01/07	2.200	2.300	2.357	2.799	2.908	2.971	0.100	0.157	0.599	0.708	0.771
NDS02/01	3.273	3.455	3.550	3.615	3.684	3.721	0.182	0.277	0.342	0.411	0.448
NDS02/02	3.061	3.454	3.549	3.615	3.683	3.720	0.393	0.488	0.554	0.622	0.659
NDS02/03	3.294	3.452	3.543	3.604	3.671	3.708	0.158	0.249	0.310	0.377	0.414
NDS02/04	3.151	3.462	3.541	3.601	3.666	3.702	0.311	0.390	0.450	0.515	0.551
NDS03/01	2.950	3.086	3.111	3.134	3.168	3.182	0.136	0.161	0.184	0.218	0.232
NDS03/02	2.690	2.762	2.788	2.811	2.912	2.975	0.072	0.098	0.121	0.222	0.285
NDS03/03	2.412	2.730	2.775	2.811	2.913	2.974	0.318	0.363	0.399	0.501	0.562
NDS03/04	2.582	2.715	2.739	2.799	2.909	2.972	0.133	0.157	0.217	0.327	0.390
NDS04/01	10.873	10.894	10.902	10.905	10.910	10.913	0.021	0.029	0.032	0.037	0.040
NDS04/02	9.750	9.876	9.888	9.895	9.908	9.915	0.126	0.138	0.145	0.158	0.165
NDS04/03	6.041	6.158	6.184	6.196	6.214	6.225	0.117	0.143	0.155	0.173	0.184
NDS04/04	3.481	3.624	3.667	3.688	3.714	3.726	0.143	0.186	0.207	0.233	0.245
NDS04/05	2.226	2.747	2.816	2.845	2.907	2.970	0.521	0.590	0.619	0.681	0.744
NDS04/06	0.545	1.500	1.500	2.400	2.400	2.400	0.955	0.955	1.855	1.855	1.855
NDS05/01	10.341	10.517	10.585	10.602	10.627	10.643	0.176	0.244	0.261	0.286	0.302
NDS06/01	4.000	4.000	4.000	4.000	4.000	4.000	0.000	0.000	0.000	0.000	0.000
NDS06/02	3.114	3.307	3.354	3.370	3.391	3.403	0.193	0.240	0.256	0.277	0.289
NDS06/03	2.653	2.931	2.982	2.960	2.990	3.008	0.278	0.329	0.307	0.337	0.355
NDS07/01	2.392	2.633	2.725	2.808	2.912	2.974	0.241	0.333	0.416	0.520	0.582
NDS07/02	2.554	2.628	2.713	2.799	2.909	2.971	0.074	0.159	0.245	0.355	0.417
NDS08/01	2.351	2.479	2.524	2.799	2.909	2.972	0.128	0.173	0.448	0.558	0.621
NDS08/02	2.500	2.500	2.523	2.799	2.908	2.972	0.000	0.023	0.299	0.408	0.472
NDS09/01	2.588	2.741	2.799	2.820	2.908	2.971	0.153	0.211	0.232	0.320	0.383

TABLE C2
Peak Water Levels- Surface Network- 2 Year Event

Node Name	Invert Level (m AHD)	Peak Level (m AHD)	Critical Duration	Maximum Depth (m)
A- Elizabeth Street				
NAS01/01	4.655	4.655	15 Min	0.000
NAS01/02	4.000	4.037	15 Min	0.037
NAS01/03	4.194	4.194	15 Min	0.000
NAS01/04	3.800	3.909	15 Min	0.109
NAS01/05	3.651	3.718	15 Min	0.067
NAS01/06	3.610	3.719	15 Min	0.109
NAS01/07	3.323	3.390	15 Min	0.067
NAS01/08	3.003	3.102	15 Min	0.099
NAS01/09	2.799	2.886	15 Min	0.087
NAS01/10	3.167	3.212	15 Min	0.045
NAS01/11	2.537	2.617	15 Min	0.080
NAS01/12	2.836	2.928	25 Min	0.092
NAS02/01	4.745	4.745	15 Min	0.000
NAS02/02	4.377	4.437	15 Min	0.060
NAS03/01	4.368	4.368	15 Min	0.000
NAS03/02	4.110	4.219	15 Min	0.109
NAS03/03	3.943	4.042	15 Min	0.099
NAS04/01	4.373	4.373	15 Min	0.000
NAS05/01	4.310	4.318	15 Min	0.008
NAS05/02	4.180	4.265	15 Min	0.084
NAS05/03	4.100	4.165	15 Min	0.065
NAS05/04	3.800	3.888	20 Min	0.088
NAS05/05	3.602	3.725	20 Min	0.123
NAS05/06	3.450	3.536	20 Min	0.086
NAS06/01	4.066	4.066	15 Min	0.000
NAS07/01	3.270	3.308	15 Min	0.037
NAS07/02	2.847	3.119	45 Min	0.272
NAS07/03	2.971	3.119	45 Min	0.148
NAS07/04	3.426	3.426	15 Min	0.000
NAS08/01	3.833	3.833	15 Min	0.000
NAS08/02	3.202	3.323	30 Min	0.121
NAS08/03	2.900	2.985	20 Min	0.085
NAS09/01	3.917	3.917	15 Min	0.000
NAS10/01	3.288	3.288	15 Min	0.000
NAS11/01	3.506	3.506	15 Min	0.000
NAS12/01	3.250	3.250	15 Min	0.000
NAS12/02	2.722	3.004	45 Min	0.282
NAS12/03	2.935	3.000	45 Min	0.065
B- Pier Street				
NBS01/03	4.915	4.915	15 Min	0.000
NBS01/04	4.515	4.607	15 Min	0.092
NBS01/04A	4.715	4.715	15 Min	0.000
NBS01/05	4.215	4.315	15 Min	0.100
NBS01/06	4.415	4.415	15 Min	0.000
NBS01/07	3.755	3.838	15 Min	0.083
NBS01/08	3.080	3.162	15 Min	0.082
NBS01/09	3.406	3.496	15 Min	0.090
NBS01/10	2.827	2.911	15 Min	0.084
NBS01/11	2.357	2.454	15 Min	0.097
NBS01/12	2.036	2.154	15 Min	0.118
NBS02/01	4.300	4.329	15 Min	0.029
NBS03/01	4.090	4.090	15 Min	0.000
NBS04/01	3.620	3.620	15 Min	0.000
NBS04/02	3.173	3.362	20 Min	0.189

Node Name	Invert Level (m AHD)	Peak Level (m AHD)	Critical Duration	Maximum Depth (m)
NBS04/03	3.502	3.502	15 Min	0.000
NBS05/01	4.040	4.076	15 Min	0.036
NBS06/01	2.829	2.934	20 Min	0.105
NBS06/02	2.477	2.665	15 Min	0.188
C- Pilot Street				
NCS01/01	6.275	6.450	15 Min	0.175
NCS01/02	5.088	5.269	15 Min	0.181
NCS01/03	4.607	4.740	15 Min	0.133
NCS01/04	3.589	3.829	15 Min	0.240
NCS01/05	3.500	3.704	20 Min	0.204
NCS01/06	3.452	3.539	20 Min	0.087
NCS01/07	3.403	3.441	45 Min	0.038
NCS01/08	3.134	3.220	45 Min	0.086
NCS01/09	3.050	3.112	15 Min	0.062
NCS01/10	3.366	3.448	15 Min	0.082
NCS01/11	3.665	3.665	15 Min	0.000
NCS01/12	0.544	1.500	15 Min	0.956
NCS02/01	5.750	5.750	15 Min	0.000
NCS03/01	5.520	5.558	15 Min	0.038
NCS03/02	3.600	3.891	20 Min	0.291
NCS04/01	3.447	3.447	15 Min	0.000
NCS04/02	3.175	3.370	45 Min	0.195
D- Guard Street				
NDS01/01	5.500	5.541	20 Min	0.041
NDS01/02	5.181	5.336	15 Min	0.155
NDS01/03	3.339	3.396	20 Min	0.057
NDS01/04	2.159	2.265	60 Min	0.106
NDS01/05	1.900	2.040	15 Min	0.140
NDS01/06	2.100	2.198	45 Min	0.098
NDS01/07	2.200	2.300	45 Min	0.100
NDS02/01	3.273	3.455	30 Min	0.182
NDS02/02	3.061	3.454	30 Min	0.393
NDS02/03	3.294	3.452	45 Min	0.158
NDS02/04	3.151	3.462	20 Min	0.311
NDS03/01	2.950	3.086	20 Min	0.136
NDS03/02	2.690	2.762	20 Min	0.072
NDS03/03	2.412	2.730	45 Min	0.318
NDS03/04	2.582	2.715	45 Min	0.133
NDS04/01	10.873	10.894	15 Min	0.021
NDS04/02	9.750	9.876	15 Min	0.126
NDS04/03	6.041	6.158	15 Min	0.117
NDS04/04	3.481	3.624	15 Min	0.143
NDS04/05	2.226	2.747	45 Min	0.521
NDS04/06	0.545	1.500	15 Min	0.955
NDS05/01	10.341	10.517	15 Min	0.176
NDS06/01	4.000	4.000	15 Min	0.000
NDS06/02	3.114	3.307	15 Min	0.193
NDS06/03	2.653	2.931	15 Min	0.278
NDS07/01	2.392	2.633	30 Min	0.241
NDS07/02	2.554	2.628	30 Min	0.074
NDS08/01	2.351	2.479	20 Min	0.128
NDS08/02	2.500	2.500	15 Min	0.000
NDS09/01	2.588	2.741	45 Min	0.153

TABLE C3
Peak Water Levels- Underground Network- 2 Year Event

Node Name	Invert Level (m AHD)	Peak Level m AHD	Critical Duration	Maximum Depth (m)	Surface IL (m AHD)	Surcharge?	Surface Water Lvl (m AHD)
A- Elizabeth Street							
NAU01/02	2.100	2.378	20 Min	0.278	4.000	-	4.037
NAU01/04	1.800	2.351	20 Min	0.551	3.800	-	3.909
NAU01/04A	2.000	2.398	20 Min	0.398	3.800	-	3.909
NAU01/05	1.600	2.318	20 Min	0.718	3.651	-	3.718
NAU01/05A	1.800	2.340	20 Min	0.540	3.651	-	3.718
NAU01/06	1.500	2.159	20 Min	0.659	3.610	-	3.719
NAU01/06A	1.700	2.205	20 Min	0.505	3.610	-	3.719
NAU01/07	1.290	2.038	20 Min	0.748	3.323	-	3.390
NAU01/07A	1.500	2.041	20 Min	0.541	3.323	-	3.390
NAU01/08	1.020	1.962	20 Min	0.942	3.003	-	3.102
NAU01/08A	1.200	2.003	20 Min	0.803	3.003	-	3.102
NAU01/09	0.940	1.805	20 Min	0.865	2.799	-	2.886
NAU01/09A	1.200	1.854	20 Min	0.654	2.799	-	2.886
NAU01/10	0.530	1.634	20 Min	1.104	3.167	-	3.212
NAU01/10A	0.700	1.635	20 Min	0.935	3.167	-	3.212
NAU01/11	0.190	1.541	20 Min	1.351	2.537	-	2.617
NAU01/11A	0.400	1.556	20 Min	1.156	2.537	-	2.617
NAU01/12	0.100	1.531	20 Min	1.431	2.836	-	2.928
NAU01/12A	0.300	1.536	20 Min	1.236	2.836	-	2.928
NAU03/03	2.500	2.684	15 Min	0.184	3.943	-	4.042
NAU05/02	3.032	3.146	15 Min	0.114	4.180	-	4.265
NAU05/04	2.690	3.058	20 Min	0.368	3.800	-	3.888
NAU05/05	2.672	2.780	20 Min	0.108	3.602	-	3.725
NAU05/06	2.680	2.790	20 Min	0.110	3.450	-	3.536
NAU06/01	1.529	2.579	30 Min	1.050	2.829	-	2.934
NAU07/02	2.165	3.118	45 Min	0.953	2.847	SURCH	3.119
NAU07/03	2.021	2.992	30 Min	0.971	2.971	SURCH	3.119
NAU08/02	2.562	2.661	30 Min	0.099	3.202	-	3.323
NAU08/03	2.291	2.450	20 Min	0.159	2.900	-	2.985
NAU12/02	2.113	2.992	45 Min	0.879	2.722	SURCH	3.004
NAUOUT1	0.000	1.500	15 Min	1.500			
NAUOUT2	0.744	1.500	15 Min	0.756			
B- Pier Street							
NBU01/04	2.000	2.866	20 Min	0.866	4.515	-	4.607
NBU01/04A	2.200	2.939	20 Min	0.739	4.515	-	4.607
NBU01/05	1.800	2.522	20 Min	0.722	4.215	-	4.315
NBU01/05A	2.200	2.554	20 Min	0.354	4.215	-	4.315
NBU01/06	1.600	2.269	20 Min	0.669			
NBU01/07	1.500	2.145	20 Min	0.645	3.755	-	3.838
NBU01/07A	2.200	2.292	20 Min	0.091	3.755	-	3.838
NBU01/08	1.000	1.907	20 Min	0.907	3.080	-	3.162
NBU01/08A	1.300	1.918	20 Min	0.618	3.080	-	3.162
NBU01/10	0.700	1.821	20 Min	1.121	2.827	-	2.911
NBU01/10A	1.000	1.827	20 Min	0.827	2.827	-	2.911
NBU01/11	0.500	1.776	20 Min	1.276	2.357	-	2.454
NBU01/11A	0.800	1.779	20 Min	0.979	2.357	-	2.454
NBU01/12	0.190	1.601	20 Min	1.411	2.036	-	2.154
NBU01/12A	0.250	1.634	20 Min	1.384	2.036	-	2.154
NBU04/02	1.853	2.064	20 Min	0.211	3.173	-	3.362
NBU05/01	2.300	2.339	15 Min	0.039	4.040	-	4.076
NBU1	0.378	2.472	30 Min	2.094			
NBUOUT1	0.350	1.500	15 Min	1.150			
NBUOUT2	1.100	1.500	15 Min	0.400			
NBUOUT2A	0.100	1.500	15 Min	1.400			

Node Name	Invert Level (m AHD)	Peak Level m AHD	Critical Duration	Maximum Depth (m)	Surface IL (m AHD)	Surcharge?	Surface Water Lvl (m AHD)
C- Pilot Street							
NCU01/04	2.979	3.452	20 Min	0.473	3.589	<150	3.829
NCU01/05	2.527	3.268	20 Min	0.741	3.500	-	3.704
NCU01/05A	2.554	3.683	20 Min	1.129	3.500	SURCH	3.704
NCU01/06	1.583	2.368	20 Min	0.785	3.452	-	3.539
NCU01/06A	1.613	2.450	20 Min	0.837	3.452	-	3.539
NCU01/07	1.310	2.196	20 Min	0.886	3.403	-	3.441
NCU01/08	1.270	2.020	25 Min	0.750	3.134	-	3.220
NCU01/09	0.850	1.899	25 Min	1.049	3.050	-	3.112
NCU01/09A	2.230	2.264	15 Min	0.034	3.050	-	3.112
NCU01/11	0.270	1.746	25 Min	1.476			
NCU03/02	2.970	3.849	20 Min	0.879	3.600	SURCH	3.891
NCU04/02	2.370	3.367	45 Min	0.997	3.175	SURCH	3.370
NCU1	1.771	1.835	30 Min	0.064			
NCUOUT	0.000	1.500	15 Min	1.500			
NCUOUT2	0.000	1.500	15 Min	1.500			
D- Guard Street							
NDU01/02	4.081	4.372	15 Min	0.291	5.181	-	5.336
NDU01/03	1.800	2.708	15 Min	0.908	3.339	-	3.396
NDU01/04	0.500	1.741	15 Min	1.241	2.159	-	2.265
NDU01/04A	0.551	2.042	45 Min	1.491	2.159	<150	2.265
NDU01/05	0.260	1.677	15 Min	1.417	1.900	-	2.040
NDU01/05A	0.300	1.707	15 Min	1.407	1.900	-	2.040
NDU01/06	0.211	1.643	15 Min	1.432	2.100	-	2.198
NDU01/06A	0.291	1.708	15 Min	1.417	2.100	-	2.198
NDU01/07	0.152	1.597	15 Min	1.445	2.200	-	2.300
NDU01/07A	0.178	1.711	45 Min	1.533	2.200	-	2.300
NDU02/02	2.387	3.454	30 Min	1.067	3.061	SURCH	3.454
NDU02/03	1.954	2.968	45 Min	1.014	3.294	-	3.452
NDU02/04	2.511	3.460	20 Min	0.949	3.151	SURCH	3.462
NDU03/01	2.241	2.286	90 Min	0.045	2.950	-	3.086
NDU03/03	1.602	2.734	45 Min	1.132	2.412	SURCH	2.730
NDU04/01	10.173	10.183	15 Min	0.010	10.873	-	10.894
NDU04/02	8.950	9.118	15 Min	0.168	9.750	-	9.876
NDU04/03	4.533	4.740	15 Min	0.207	6.041	-	6.158
NDU04/03A	4.733	4.860	15 Min	0.127	6.041	-	6.158
NDU04/04	2.821	3.626	15 Min	0.805	3.481	SURCH	3.624
NDU04/04A	2.921	3.625	15 Min	0.704	3.481	SURCH	3.624
NDU04/05	1.845	2.676	30 Min	0.831	2.226	SURCH	2.747
NDU04/05A	1.890	2.749	45 Min	0.859	2.226	SURCH	2.747
NDU05/01	9.571	9.877	15 Min	0.306	10.341	-	10.517
NDU06/03	2.054	2.256	15 Min	0.202	2.653	-	2.931
NDU07/01	1.222	2.607	30 Min	1.385	2.392	SURCH	2.633
NDU08/01	1.160	2.127	25 Min	0.967	2.351	-	2.479
NDU1	1.270	1.748	15 Min	0.478			
NDUOUT1	0.900	1.500	15 Min	0.600			
NDUOUT2	0.050	1.500	15 Min	1.450			
NDUOUT3	0.900	1.500	15 Min	0.600			

TABLE C4
Peak Flows and Velocities- Surface Network- 2 Year Event

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)	U/S Node Depth (m)	U/S Node v.d (m ² /s)	D/S Node Depth (m)	D/S Node v.d (m ² /s)
A- Elizabeth Street							
LAS01/01	0.00	15 Min	0.00	0.00	0.00	0.04	0.00
LAS01/03	0.00	15 Min	0.00	0.00	0.00	0.11	0.00
LAS01/03A1	0.00	15 Min	0.00	0.00	0.00	0.04	0.00
LAS01/04	0.05	15 Min	0.49	0.11	0.05	0.07	0.03
LAS01/05	0.01	15 Min	0.06	0.07	0.00	0.11	0.01
LAS01/06	0.05	15 Min	0.45	0.11	0.05	0.07	0.03
LAS01/07	0.10	15 Min	0.30	0.07	0.02	0.10	0.03
LAS01/08	0.39	15 Min	0.70	0.10	0.07	0.09	0.06
LAS01/10	0.06	15 Min	0.31	0.04	0.01	0.08	0.02
LAS01/10A1	0.05	15 Min	0.18	0.04	0.01	0.09	0.02
LAS01/10A8	0.02	20 Min	0.29	0.04	0.01	0.08	0.02
LAS01/12A1	0.06	20 Min	0.59	0.09	0.05	0.08	0.05
LAS02/01	0.00	15 Min	0.00	0.00	0.00	0.06	0.00
LAS02/01B1	0.00	15 Min	0.00	0.00	0.00	0.09	0.00
LAS02/02	0.13	15 Min	0.47	0.06	0.03	0.11	0.05
LAS03/01	0.00	15 Min	0.00	0.00	0.00	0.11	0.00
LAS03/01B1	0.00	15 Min	0.00	0.00	0.00	0.10	0.00
LAS03/02	0.20	15 Min	0.56	0.11	0.06	0.10	0.05
LAS03/03	0.25	15 Min	0.69	0.10	0.07	0.11	0.07
LAS04/01A3	0.00	15 Min	0.00	0.00	0.00	0.11	0.00
LAS04/01A5	0.00	15 Min	0.00	0.00	0.00	0.06	0.00
LAS05/01	0.00	15 Min	0.01	0.01	0.00	0.08	0.00
LAS05/01B2	0.00	15 Min	-0.05	0.01	0.00	0.03	0.00
LAS05/02	0.05	15 Min	0.36	0.08	0.03	0.06	0.02
LAS05/03	0.09	15 Min	0.29	0.06	0.02	0.09	0.03
LAS05/04	0.15	20 Min	0.36	0.09	0.03	0.12	0.04
LAS05/05	0.16	20 Min	0.65	0.12	0.08	0.09	0.06
LAS05/06	0.18	20 Min	0.48	0.09	0.04	0.10	0.05
LAS06/01A5	0.00	15 Min	0.00	0.00	0.00	0.09	0.00
LAS06/01A7	0.00	15 Min	0.00	0.00	0.00	0.27	0.00
LAS07/01	0.08	15 Min	0.27	0.04	0.01	0.27	0.07
LAS07/03A2	0.17	15 Min	0.41	0.15	0.06	0.27	0.11
LAS07/04	0.00	15 Min	0.00	0.00	0.00	0.04	0.00
LAS07/04A7	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LAS08/01	0.00	15 Min	0.00	0.00	0.00	0.12	0.00
LAS08/02	0.07	30 Min	0.72	0.12	0.09	0.08	0.06
LAS09/01	0.00	15 Min	0.00	0.00	0.00	0.12	0.00
LAS10/01	0.00	15 Min	0.00	0.00	0.00	0.09	0.00
LAS10/01B6	0.00	15 Min	0.00	0.00	0.00	0.10	0.00
LAS11/01	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LAS11/01B6	0.00	15 Min	0.00	0.00	0.00	0.10	0.00
LAS12/01	0.00	15 Min	0.00	0.00	0.00	0.28	0.00
LAS12/03	-0.03	45 Min	-0.05	0.07	0.00	0.28	-0.02
LAS12/03A1	0.03	45 Min	0.30	0.07	0.02	0.09	0.03
B- Pier Street							
LBS01/03	0.00	15 Min	0.00	0.00	0.00	0.09	0.00
LBS01/04	0.00	15 Min	0.00	0.00	0.00	0.10	0.00
LBS01/04B1	0.00	15 Min	0.00	0.00	0.00	0.09	0.00
LBS01/06	0.00	15 Min	0.00	0.00	0.00	0.08	0.00
LBS01/06B1	0.00	15 Min	0.00	0.00	0.00	0.10	0.00
LBS01/07	0.09	15 Min	0.60	0.08	0.05	0.08	0.05
LBS01/09	0.05	15 Min	0.65	0.09	0.06	0.08	0.05
LBS01/09B1	0.07	15 Min	0.52	0.09	0.05	0.08	0.04
LBS01/10	0.07	15 Min	0.53	0.08	0.04	0.10	0.05

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)	U/S Node Depth (m)	U/S Node v.d (m ² /s)	D/S Node Depth (m)	D/S Node v.d (m ² /s)
LBS01/11	0.06	15 Min	0.28	0.10	0.03	0.12	0.03
LBS02/01	0.01	25 Min	0.26	0.03	0.01	0.08	0.02
LBS02/01B5	0.03	15 Min	0.29	0.03	0.01	0.04	0.01
LBS03/01	0.00	15 Min	0.00	0.00	0.00	0.08	0.00
LBS03/01C1	0.00	15 Min	0.00	0.00	0.00	0.04	0.00
LBS04/01	0.00	15 Min	0.00	0.00	0.00	0.19	0.00
LBS04/01A7	0.00	15 Min	0.00	0.00	0.00	0.04	0.00
LBS04/02B1	0.15	20 Min	0.89	0.19	0.17	0.12	0.10
LBS04/03	0.00	15 Min	0.00	0.00	0.00	0.09	0.00
LBS05/01	0.09	15 Min	0.32	0.04	0.01	0.19	0.06
LBS06/01	0.06	20 Min	0.31	0.10	0.03	0.19	0.06
LBS06/02	0.17	15 Min	0.65	0.19	0.12	0.12	0.08
C- Pilot Street							
LCS01/01	0.05	15 Min	0.42	0.18	0.07	0.18	0.08
LCS01/02	0.47	15 Min	0.66	0.18	0.12	0.13	0.09
LCS01/03	0.58	15 Min	0.68	0.13	0.09	0.24	0.16
LCS01/04	0.72	20 Min	0.62	0.24	0.15	0.20	0.13
LCS01/05	0.27	20 Min	0.41	0.20	0.08	0.09	0.04
LCS01/06	0.00	15 Min	0.00	0.09	0.00	0.04	0.00
LCS01/06D2	0.13	20 Min	0.74	0.09	0.06	0.18	0.14
LCS01/07	0.04	45 Min	0.25	0.04	0.01	0.09	0.02
LCS01/07D2	-0.06	30 Min	-0.37	0.04	-0.01	0.18	-0.07
LCS01/08	0.00	15 Min	0.11	0.09	0.01	0.06	0.01
LCS01/09D3	0.15	15 Min	0.20	0.06	0.01	0.14	0.03
LCS01/10C1	0.10	15 Min	0.39	0.08	0.03	0.06	0.02
LCS01/11	0.00	15 Min	0.00	0.00	0.00	0.20	0.00
LCS01/11B1	0.00	15 Min	0.00	0.00	0.00	0.10	0.00
LCS01/11C1	0.00	15 Min	0.00	0.00	0.00	0.08	0.00
LCS01/OUT	0.00	15 Min	0.00	0.00	0.00	0.96	0.00
LCS02/01	0.00	15 Min	0.00	0.00	0.00	0.18	0.00
LCS02/01C3	0.00	15 Min	0.00	0.00	0.00	0.04	0.00
LCS03/01	0.25	15 Min	0.22	0.04	0.01	0.29	0.06
LCS03/01D1	0.01	15 Min	0.18	0.04	0.01	0.04	0.01
LCS03/02	0.22	20 Min	0.32	0.29	0.09	0.24	0.08
LCS04/01	0.00	15 Min	0.00	0.00	0.00	0.20	0.00
LCS04/01D8	0.00	15 Min	0.00	0.00	0.00	0.13	0.00
D- Guard Street							
LDS01/01	0.01	20 Min	0.15	0.04	0.01	0.15	0.02
LDS01/02	0.03	15 Min	1.70	0.15	0.26	0.06	0.10
LDS01/02D6	0.00	15 Min	0.00	0.15	0.00	0.00	0.00
LDS01/03	0.10	20 Min	0.45	0.06	0.03	0.11	0.05
LDS01/03D2	-0.08	20 Min	-0.18	0.06	-0.01	0.31	-0.05
LDS01/04	0.08	45 Min	0.68	0.11	0.07	0.14	0.09
LDS01/06	0.08	45 Min	0.37	0.10	0.04	0.14	0.05
LDS01/07	0.14	45 Min	0.29	0.10	0.03	0.10	0.03
LDS02/01	-0.10	15 Min	0.15	0.18	0.03	0.39	0.06
LDS02/03	-0.08	20 Min	-0.10	0.16	-0.02	0.39	-0.04
LDS02/03D2	-0.13	20 Min	0.30	0.16	0.05	0.31	0.09
LDS03/01	0.13	20 Min	0.48	0.14	0.07	0.07	0.03
LDS03/02	0.06	20 Min	0.18	0.07	0.01	0.32	0.06
LDS03/02D7	0.17	20 Min	0.19	0.07	0.01	0.24	0.05
LDS03/04	-0.20	45 Min	-0.17	0.13	-0.02	0.32	-0.05
LDS03/04D1	0.18	45 Min	0.42	0.13	0.06	0.11	0.04
LDS04/01	0.19	15 Min	0.23	0.02	0.00	0.13	0.03
LDS04/02	0.49	15 Min	1.89	0.13	0.24	0.12	0.22
LDS04/03	0.31	15 Min	1.44	0.12	0.17	0.14	0.21
LDS04/03D6	0.18	15 Min	0.50	0.12	0.06	0.28	0.14
LDS04/04	0.40	15 Min	0.76	0.14	0.11	0.52	0.39

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)	U/S Node Depth (m)	U/S Node v.d (m ² /s)	D/S Node Depth (m)	D/S Node v.d (m ² /s)
LDS04/OUT	0.01	45 Min	0.36	0.52	0.19	0.96	0.34
LDS05/01	0.28	15 Min	1.41	0.18	0.25	0.13	0.18
LDS06/01	0.00	15 Min	0.00	0.00	0.00	0.19	0.00
LDS06/02	0.22	15 Min	0.43	0.19	0.08	0.28	0.12
LDS06/03	0.57	15 Min	0.84	0.28	0.23	0.14	0.12
LDS07/02	-0.07	30 Min	-0.08	0.07	-0.01	0.24	-0.02
LDS07/02D1	0.05	30 Min	0.40	0.07	0.03	0.10	0.04
LDS08/02	0.00	15 Min	0.00	0.00	0.00	0.13	0.00
LDS08/02D1	0.00	15 Min	0.00	0.00	0.00	0.10	0.00
LDS09/01	0.26	45 Min	0.80	0.15	0.12	0.10	0.08
LDS09/01D4	-0.27	45 Min	-0.10	0.15	-0.02	0.52	-0.05

TABLE C5
Peak Flows and Velocities- Underground Network- 2 Year Event

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)
A- Elizabeth Street			
LAU01/02	0.04	20 Min	0.48
LAU01/04	0.16	20 Min	0.84
LAU01/04A	0.15	15 Min	0.82
LAU01/05	0.25	20 Min	0.87
LAU01/05A	0.09	15 Min	0.79
LAU01/06	0.56	20 Min	1.18
LAU01/06A	0.25	15 Min	0.93
LAU01/07	0.58	20 Min	1.03
LAU01/07A	0.01	20 Min	0.11
LAU01/08	0.80	20 Min	1.27
LAU01/08A	0.09	20 Min	0.42
LAU01/09	1.32	20 Min	1.04
LAU01/09A	0.54	15 Min	0.51
LAU01/10	1.48	20 Min	0.77
LAU01/10A	0.03	45 Min	0.08
LAU01/11	1.69	20 Min	0.48
LAU01/11A	0.25	15 Min	0.29
LAU01/12	1.72	20 Min	0.50
LAU01/12A	0.03	25 Min	0.15
LAU03/03	0.08	15 Min	0.99
LAU05/02	0.03	15 Min	0.62
LAU05/04	0.05	20 Min	0.72
LAU05/05	0.05	45 Min	0.88
LAU05/06	0.08	20 Min	1.26
LAU07/02	0.15	15 Min	0.98
LAU07/03	0.16	15 Min	1.15
LAU08/02	0.02	30 Min	0.67
LAU08/03	0.15	20 Min	0.94
LAU12/02	0.14	45 Min	1.91
B- Pier Street			
LBU01/04	0.29	20 Min	1.00
LBU01/04A	0.29	20 Min	0.79
LBU01/05	0.55	20 Min	1.29
LBU01/05A	0.27	20 Min	1.02
LBU01/06	0.55	20 Min	1.40
LBU01/07	0.64	20 Min	1.16
LBU01/07A	0.12	15 Min	1.10
LBU01/08	0.84	20 Min	0.84
LBU01/08A	0.22	15 Min	0.29
LBU01/10	0.87	20 Min	0.77
LBU01/10A	0.03	15 Min	0.19
LBU01/11	0.93	20 Min	0.73
LBU01/11A	0.07	15 Min	0.13
LBU01/12	1.35	20 Min	0.80
LBU01/12A	0.63	15 Min	0.39
LBU01/12LF	0.27	20 Min	0.61
LBU04/02	0.09	20 Min	1.03
LBU05/01	0.00	15 Min	0.42
LBU06/01	0.17	30 Min	1.07
LBU1	0.17	30 Min	2.38
C- Pilot Street			
LCU01/04	0.18	30 Min	1.05
LCU01/05	0.61	30 Min	2.08
LCU01/05A	0.46	15 Min	1.42

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)
LCU01/06	0.78	20 Min	1.10
LCU01/06A	0.19	20 Min	0.59
LCU01/07	0.78	20 Min	1.11
LCU01/08	0.81	25 Min	1.08
LCU01/09	0.85	25 Min	0.98
LCU01/09A	0.07	15 Min	0.43
LCU01/11	0.85	25 Min	0.98
LCU03/02	0.08	45 Min	1.12
LCU04/02	0.07	45 Min	1.64
LCU1	0.07	45 Min	0.53
D- Guard Street			
LDU01/02	0.30	15 Min	1.70
LDU01/03	0.30	15 Min	1.87
LDU01/04	0.49	15 Min	0.91
LDU01/04A	0.27	45 Min	1.22
LDU01/05	1.42	15 Min	0.60
LDU01/05A	0.86	15 Min	0.36
LDU01/06	1.58	15 Min	0.67
LDU01/06A	0.18	45 Min	0.57
LDU01/07	1.72	15 Min	0.73
LDU01/07A	0.24	45 Min	0.75
LDU02/02	0.08	15 Min	1.17
LDU02/03	0.26	30 Min	1.01
LDU02/04	0.08	15 Min	1.18
LDU03/01	0.03	15 Min	0.49
LDU03/03	0.28	15 Min	1.00
LDU04/01	0.00	15 Min	0.33
LDU04/02	0.16	15 Min	3.05
LDU04/03	0.18	15 Min	2.25
LDU04/03A	0.03	15 Min	0.59
LDU04/04	0.18	15 Min	1.63
LDU04/04A	0.06	15 Min	0.57
LDU04/05	0.18	30 Min	1.63
LDU04/05A	0.04	30 Min	1.31
LDU05/01	0.12	15 Min	1.44
LDU06/03	0.07	15 Min	0.90
LDU07/01	0.40	30 Min	1.41
LDU08/01	0.50	25 Min	1.77
LDU1	0.08	15 Min	0.67

TABLE C6
Peak Water Levels- Surface Network- 10 Year Event

Node Name	Invert Level (m AHD)	Peak Level (m AHD)	Critical Duration	Maximum Depth (m)
A- Elizabeth Street				
NAS01/01	4.655	4.655	15 Min	0.000
NAS01/02	4.000	4.048	15 Min	0.048
NAS01/03	4.194	4.194	15 Min	0.000
NAS01/04	3.800	3.937	15 Min	0.137
NAS01/05	3.651	3.741	15 Min	0.090
NAS01/06	3.610	3.746	15 Min	0.136
NAS01/07	3.323	3.414	15 Min	0.091
NAS01/08	3.003	3.133	15 Min	0.130
NAS01/09	2.799	2.917	15 Min	0.118
NAS01/10	3.167	3.227	15 Min	0.060
NAS01/11	2.537	2.644	15 Min	0.107
NAS01/12	2.836	2.986	45 Min	0.150
NAS02/01	4.745	4.745	15 Min	0.000
NAS02/02	4.377	4.458	15 Min	0.081
NAS03/01	4.368	4.368	15 Min	0.000
NAS03/02	4.110	4.252	15 Min	0.142
NAS03/03	3.943	4.072	15 Min	0.129
NAS04/01	4.373	4.373	15 Min	0.000
NAS05/01	4.310	4.325	15 Min	0.015
NAS05/02	4.180	4.293	15 Min	0.113
NAS05/03	4.100	4.187	15 Min	0.087
NAS05/04	3.800	3.916	20 Min	0.116
NAS05/05	3.602	3.748	15 Min	0.146
NAS05/06	3.450	3.570	20 Min	0.120
NAS06/01	4.066	4.066	15 Min	0.000
NAS07/01	3.270	3.323	15 Min	0.052
NAS07/02	2.847	3.260	60 Min	0.413
NAS07/03	2.971	3.260	60 Min	0.289
NAS07/04	3.426	3.426	15 Min	0.000
NAS08/01	3.833	3.833	15 Min	0.000
NAS08/02	3.202	3.350	25 Min	0.148
NAS08/03	2.900	3.020	25 Min	0.120
NAS09/01	3.917	3.917	15 Min	0.000
NAS10/01	3.288	3.288	15 Min	0.000
NAS11/01	3.506	3.506	15 Min	0.000
NAS12/01	3.250	3.250	15 Min	0.000
NAS12/02	2.722	3.093	45 Min	0.371
NAS12/03	2.935	3.073	45 Min	0.138
B- Pier Street				
NBS01/03	4.915	4.915	15 Min	0.000
NBS01/04	4.515	4.638	15 Min	0.123
NBS01/04A	4.715	4.715	15 Min	0.000
NBS01/05	4.215	4.348	20 Min	0.133
NBS01/06	4.415	4.415	15 Min	0.000
NBS01/07	3.755	3.864	15 Min	0.109
NBS01/08	3.080	3.190	15 Min	0.110
NBS01/09	3.406	3.528	15 Min	0.122
NBS01/10	2.827	2.935	15 Min	0.108
NBS01/11	2.357	2.477	15 Min	0.120
NBS01/12	2.036	2.203	20 Min	0.167
NBS02/01	4.300	4.341	15 Min	0.041
NBS03/01	4.090	4.090	15 Min	0.000
NBS04/01	3.620	3.620	15 Min	0.000
NBS04/02	3.173	3.433	25 Min	0.260

Node Name	Invert Level (m AHD)	Peak Level (m AHD)	Critical Duration	Maximum Depth (m)
NBS04/03	3.502	3.521	15 Min	0.018
NBS05/01	4.040	4.089	15 Min	0.049
NBS06/01	2.829	2.963	20 Min	0.134
NBS06/02	2.477	2.697	15 Min	0.220
C- Pilot Street				
NCS01/01	6.275	6.497	15 Min	0.222
NCS01/02	5.088	5.306	15 Min	0.218
NCS01/03	4.607	4.764	15 Min	0.157
NCS01/04	3.589	3.889	15 Min	0.300
NCS01/05	3.500	3.822	20 Min	0.322
NCS01/06	3.452	3.603	20 Min	0.151
NCS01/07	3.403	3.534	30 Min	0.131
NCS01/08	3.134	3.332	30 Min	0.198
NCS01/09	3.050	3.131	15 Min	0.081
NCS01/10	3.366	3.478	15 Min	0.112
NCS01/11	3.665	3.665	15 Min	0.000
NCS01/12	0.544	1.500	15 Min	0.956
NCS02/01	5.750	5.750	15 Min	0.000
NCS03/01	5.520	5.572	15 Min	0.052
NCS03/02	3.600	3.972	20 Min	0.372
NCS04/01	3.447	3.482	60 Min	0.035
NCS04/02	3.175	3.482	60 Min	0.307
D- Guard Street				
NDS01/01	5.500	5.554	20 Min	0.054
NDS01/02	5.181	5.456	15 Min	0.275
NDS01/03	3.339	3.441	15 Min	0.102
NDS01/04	2.159	2.361	30 Min	0.202
NDS01/05	1.900	2.259	30 Min	0.359
NDS01/06	2.100	2.278	30 Min	0.178
NDS01/07	2.200	2.357	25 Min	0.157
NDS02/01	3.273	3.550	30 Min	0.277
NDS02/02	3.061	3.549	30 Min	0.488
NDS02/03	3.294	3.543	30 Min	0.249
NDS02/04	3.151	3.541	30 Min	0.390
NDS03/01	2.950	3.111	15 Min	0.161
NDS03/02	2.690	2.788	20 Min	0.098
NDS03/03	2.412	2.775	30 Min	0.363
NDS03/04	2.582	2.739	30 Min	0.157
NDS04/01	10.873	10.902	15 Min	0.029
NDS04/02	9.750	9.888	15 Min	0.138
NDS04/03	6.041	6.184	15 Min	0.143
NDS04/04	3.481	3.667	15 Min	0.186
NDS04/05	2.226	2.816	25 Min	0.590
NDS04/06	0.545	1.500	15 Min	0.955
NDS05/01	10.341	10.585	15 Min	0.244
NDS06/01	4.000	4.000	15 Min	0.000
NDS06/02	3.114	3.354	15 Min	0.240
NDS06/03	2.653	2.982	15 Min	0.329
NDS07/01	2.392	2.725	30 Min	0.333
NDS07/02	2.554	2.713	30 Min	0.159
NDS08/01	2.351	2.524	20 Min	0.173
NDS08/02	2.500	2.523	20 Min	0.023
NDS09/01	2.588	2.799	25 Min	0.211

TABLE C7
Peak Water Levels- Underground Network- 10 Year Event

Node Name	Invert Level (m AHD)	Peak Level m AHD	Critical Duration	Maximum Depth (m)	Surface IL (m AHD)	Surcharge?	Surface Water Lvl (m AHD)
A- Elizabeth Street							
NAU01/02	2.100	3.500	25 Min	1.400	4.000	-	4.048
NAU01/04	1.800	3.462	25 Min	1.662	3.800	-	3.937
NAU01/04A	2.000	3.557	25 Min	1.557	3.800	-	3.937
NAU01/05	1.600	3.378	25 Min	1.778	3.651	-	3.741
NAU01/05A	1.800	3.435	25 Min	1.635	3.651	-	3.741
NAU01/06	1.500	2.992	20 Min	1.492	3.610	-	3.746
NAU01/06A	1.700	3.071	20 Min	1.371	3.610	-	3.746
NAU01/07	1.290	2.784	20 Min	1.494	3.323	-	3.414
NAU01/07A	1.500	2.793	20 Min	1.293	3.323	-	3.414
NAU01/08	1.020	2.604	20 Min	1.584	3.003	-	3.133
NAU01/08A	1.200	2.712	20 Min	1.512	3.003	-	3.133
NAU01/09	0.940	2.267	20 Min	1.327	2.799	-	2.917
NAU01/09A	1.200	2.364	20 Min	1.164	2.799	-	2.917
NAU01/10	0.530	1.829	20 Min	1.299	3.167	-	3.227
NAU01/10A	0.700	1.829	20 Min	1.129	3.167	-	3.227
NAU01/11	0.190	1.606	20 Min	1.416	2.537	-	2.644
NAU01/11A	0.400	1.654	20 Min	1.254	2.537	-	2.644
NAU01/12	0.100	1.579	20 Min	1.479	2.836	-	2.986
NAU01/12A	0.300	1.601	25 Min	1.301	2.836	-	2.986
NAU03/03	2.500	3.603	20 Min	1.103	3.943	-	4.072
NAU05/02	3.032	3.446	20 Min	0.414	4.180	-	4.293
NAU05/04	2.690	3.410	20 Min	0.720	3.800	-	3.916
NAU05/05	2.672	2.906	20 Min	0.234	3.602	-	3.748
NAU05/06	2.680	3.124	20 Min	0.444	3.450	-	3.570
NAU06/01	1.529	2.752	30 Min	1.223	2.829	<150	2.963
NAU07/02	2.165	3.260	60 Min	1.095	2.847	SURCH	3.260
NAU07/03	2.021	3.156	60 Min	1.135	2.971	SURCH	3.260
NAU08/02	2.562	2.706	25 Min	0.144	3.202	-	3.350
NAU08/03	2.291	2.623	25 Min	0.332	2.900	-	3.020
NAU12/02	2.113	3.082	45 Min	0.969	2.722	SURCH	3.093
NAUOUT1	0.000	1.500	15 Min	1.500			
NAUOUT2	0.744	1.500	15 Min	0.756			
B- Pier Street							
NBU01/04	2.000	4.213	25 Min	2.213	4.515	-	4.638
NBU01/04A	2.200	4.351	25 Min	2.151	4.515	-	4.638
NBU01/05	1.800	3.551	25 Min	1.751	4.215	-	4.348
NBU01/05A	2.200	3.597	25 Min	1.397	4.215	-	4.348
NBU01/06	1.600	3.019	25 Min	1.419			
NBU01/07	1.500	2.767	25 Min	1.267	3.755	-	3.864
NBU01/07A	2.200	2.821	20 Min	0.621	3.755	-	3.864
NBU01/08	1.000	2.368	20 Min	1.368	3.080	-	3.190
NBU01/08A	1.300	2.395	20 Min	1.095	3.080	-	3.190
NBU01/10	0.700	2.196	20 Min	1.496	2.827	-	2.935
NBU01/10A	1.000	2.214	20 Min	1.214	2.827	-	2.935
NBU01/11	0.500	2.087	20 Min	1.587	2.357	-	2.477
NBU01/11A	0.800	2.097	20 Min	1.297	2.357	-	2.477
NBU01/12	0.190	1.706	20 Min	1.516	2.036	-	2.203
NBU01/12A	0.250	1.816	20 Min	1.566	2.036	-	2.203
NBU04/02	1.853	2.120	20 Min	0.267	3.173	-	3.433
NBU05/01	2.300	2.354	15 Min	0.054	4.040	-	4.089
NBU1	0.378	2.627	30 Min	2.249			
NBUOUT1	0.350	1.500	15 Min	1.150			
NBUOUT2	1.100	1.500	15 Min	0.400			
NBUOUT2A	0.100	1.500	15 Min	1.400			

Node Name	Invert Level (m AHD)	Peak Level m AHD	Critical Duration	Maximum Depth (m)	Surface IL (m AHD)	Surcharge?	Surface Water Lvl (m AHD)
C- Pilot Street							
NCU01/04	2.979	3.750	30 Min	0.771	3.589	SURCH	3.889
NCU01/05	2.527	3.600	30 Min	1.073	3.500	SURCH	3.822
NCU01/05A	2.554	3.800	20 Min	1.246	3.500	SURCH	3.822
NCU01/06	1.583	3.099	30 Min	1.516	3.452	-	3.603
NCU01/06A	1.613	3.502	30 Min	1.889	3.452	SURCH	3.603
NCU01/07	1.310	2.940	30 Min	1.630	3.403	-	3.534
NCU01/08	1.270	2.776	30 Min	1.506	3.134	-	3.332
NCU01/09	0.850	2.488	30 Min	1.638	3.050	-	3.131
NCU01/09A	2.230	2.493	30 Min	0.263	3.050	-	3.131
NCU01/11	0.270	2.102	30 Min	1.832			
NCU03/02	2.970	3.922	20 Min	0.952	3.600	SURCH	3.972
NCU04/02	2.370	3.479	60 Min	1.109	3.175	SURCH	3.482
NCU1	1.771	1.840	60 Min	0.069			
NCUOUT	0.000	1.500	15 Min	1.500			
NCUOUT2	0.000	1.500	15 Min	1.500			
D- Guard Street							
NDU01/02	4.081	5.026	20 Min	0.945	5.181	-	5.456
NDU01/03	1.800	3.370	25 Min	1.570	3.339	SURCH	3.441
NDU01/04	0.500	2.080	25 Min	1.580	2.159	<150	2.361
NDU01/04A	0.551	2.353	30 Min	1.802	2.159	SURCH	2.361
NDU01/05	0.260	1.998	30 Min	1.738	1.900	SURCH	2.259
NDU01/05A	0.300	2.094	30 Min	1.794	1.900	SURCH	2.259
NDU01/06	0.211	1.917	30 Min	1.706	2.100	-	2.278
NDU01/06A	0.291	2.254	30 Min	1.963	2.100	SURCH	2.278
NDU01/07	0.152	1.796	25 Min	1.644	2.200	-	2.357
NDU01/07A	0.178	2.312	25 Min	2.134	2.200	SURCH	2.357
NDU02/02	2.387	3.549	30 Min	1.162	3.061	SURCH	3.549
NDU02/03	1.954	3.161	30 Min	1.207	3.294	<150	3.543
NDU02/04	2.511	3.541	30 Min	1.030	3.151	SURCH	3.541
NDU03/01	2.241	2.501	30 Min	0.260	2.950	-	3.111
NDU03/03	1.602	2.790	30 Min	1.188	2.412	SURCH	2.775
NDU04/01	10.173	10.188	15 Min	0.015	10.873	-	10.902
NDU04/02	8.950	9.119	90 Min	0.169	9.750	-	9.888
NDU04/03	4.533	4.774	15 Min	0.241	6.041	-	6.184
NDU04/03A	4.733	4.896	15 Min	0.163	6.041	-	6.184
NDU04/04	2.821	3.700	15 Min	0.879	3.481	SURCH	3.667
NDU04/04A	2.921	3.687	15 Min	0.766	3.481	SURCH	3.667
NDU04/05	1.845	2.736	20 Min	0.891	2.226	SURCH	2.816
NDU04/05A	1.890	2.818	25 Min	0.928	2.226	SURCH	2.816
NDU05/01	9.571	9.890	15 Min	0.319	10.341	-	10.585
NDU06/03	2.054	2.357	30 Min	0.303	2.653	-	2.982
NDU07/01	1.222	2.709	30 Min	1.487	2.392	SURCH	2.725
NDU08/01	1.160	2.305	25 Min	1.145	2.351	<150	2.524
NDU1	1.270	2.068	30 Min	0.798			
NDUOUT1	0.900	1.500	15 Min	0.600			
NDUOUT2	0.050	1.500	15 Min	1.450			
NDUOUT3	0.900	1.500	15 Min	0.600			

TABLE C8
Peak Flows and Velocities- Surface Network- 10 Year Event

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)	U/S Node Depth (m)	U/S Node v.d (m ² /s)	D/S Node Depth (m)	D/S Node v.d (m ² /s)
A- Elizabeth Street							
LAS01/01	0.00	15 Min	0.00	0.00	0.00	0.05	0.00
LAS01/03	0.00	15 Min	0.00	0.00	0.00	0.14	0.00
LAS01/03A1	0.00	15 Min	0.00	0.00	0.00	0.05	0.00
LAS01/04	0.10	15 Min	0.56	0.14	0.08	0.09	0.05
LAS01/05	-0.04	15 Min	-0.08	0.09	-0.01	0.14	-0.01
LAS01/06	0.10	15 Min	0.52	0.14	0.07	0.09	0.05
LAS01/07	0.16	15 Min	0.35	0.09	0.03	0.13	0.04
LAS01/08	0.68	15 Min	0.84	0.13	0.11	0.12	0.10
LAS01/10	0.10	15 Min	0.34	0.06	0.02	0.11	0.04
LAS01/10A1	0.08	15 Min	0.21	0.06	0.01	0.12	0.03
LAS01/10A8	0.02	60 Min	0.29	0.06	0.02	0.12	0.04
LAS01/12A1	0.19	45 Min	0.76	0.15	0.11	0.11	0.08
LAS02/01	0.00	15 Min	0.00	0.00	0.00	0.08	0.00
LAS02/01B1	0.00	15 Min	0.00	0.00	0.00	0.12	0.00
LAS02/02	0.21	15 Min	0.58	0.08	0.05	0.14	0.08
LAS03/01	0.00	15 Min	0.00	0.00	0.00	0.14	0.00
LAS03/01B1	0.00	15 Min	0.00	0.00	0.00	0.13	0.00
LAS03/02	0.35	15 Min	0.63	0.14	0.09	0.13	0.08
LAS03/03	0.41	15 Min	0.79	0.13	0.10	0.14	0.11
LAS04/01A3	0.00	15 Min	0.00	0.00	0.00	0.14	0.00
LAS04/01A5	0.00	15 Min	0.00	0.00	0.00	0.09	0.00
LAS05/01	0.00	15 Min	0.02	0.01	0.00	0.11	0.00
LAS05/01B2	0.00	15 Min	-0.07	0.01	0.00	0.04	0.00
LAS05/02	0.08	15 Min	0.45	0.11	0.05	0.09	0.04
LAS05/03	0.15	15 Min	0.36	0.09	0.03	0.12	0.04
LAS05/04	0.24	20 Min	0.38	0.12	0.04	0.15	0.06
LAS05/05	0.29	20 Min	0.72	0.15	0.10	0.12	0.09
LAS05/06	0.32	20 Min	0.59	0.12	0.07	0.13	0.08
LAS06/01A5	0.00	15 Min	0.00	0.00	0.00	0.12	0.00
LAS06/01A7	0.00	15 Min	0.00	0.00	0.00	0.41	0.00
LAS07/01	0.14	15 Min	0.32	0.05	0.02	0.41	0.13
LAS07/03A2	0.22	15 Min	0.44	0.29	0.13	0.41	0.18
LAS07/04	0.00	15 Min	0.00	0.00	0.00	0.06	0.00
LAS07/04A7	0.00	15 Min	0.00	0.00	0.00	0.29	0.00
LAS08/01	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LAS08/02	0.15	25 Min	0.76	0.15	0.11	0.12	0.09
LAS09/01	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LAS10/01	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LAS10/01B6	0.00	15 Min	0.00	0.00	0.00	0.13	0.00
LAS11/01	0.00	15 Min	0.00	0.00	0.00	0.29	0.00
LAS11/01B6	0.00	15 Min	0.00	0.00	0.00	0.13	0.00
LAS12/01	0.00	15 Min	0.00	0.00	0.00	0.37	0.00
LAS12/03	-0.18	45 Min	-0.14	0.14	-0.02	0.37	-0.05
LAS12/03A1	0.18	45 Min	0.52	0.14	0.07	0.15	0.08
B- Pier Street							
LBS01/03	0.00	15 Min	0.00	0.00	0.00	0.12	0.00
LBS01/04	0.00	15 Min	0.00	0.00	0.00	0.13	0.00
LBS01/04B1	0.00	15 Min	0.00	0.00	0.00	0.12	0.00
LBS01/06	0.00	15 Min	0.00	0.00	0.00	0.11	0.00
LBS01/06B1	0.00	15 Min	0.00	0.00	0.00	0.13	0.00
LBS01/07	0.14	15 Min	0.72	0.11	0.08	0.11	0.08
LBS01/09	0.08	15 Min	0.78	0.12	0.10	0.11	0.08
LBS01/09B1	0.11	15 Min	0.64	0.12	0.08	0.11	0.07
LBS01/10	0.10	15 Min	0.63	0.11	0.07	0.12	0.08

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)	U/S Node Depth (m)	U/S Node v.d (m ² /s)	D/S Node Depth (m)	D/S Node v.d (m ² /s)
LBS01/11	0.09	15 Min	0.29	0.12	0.03	0.17	0.05
LBS02/01	0.01	60 Min	0.27	0.04	0.01	0.11	0.03
LBS02/01B5	0.05	15 Min	0.33	0.04	0.01	0.05	0.02
LBS03/01	0.00	15 Min	0.00	0.00	0.00	0.11	0.00
LBS03/01C1	0.00	15 Min	0.00	0.00	0.00	0.13	0.00
LBS04/01	0.00	15 Min	0.00	0.00	0.00	0.26	0.00
LBS04/01A7	0.00	15 Min	0.00	0.00	0.00	0.05	0.00
LBS04/02B1	0.37	20 Min	1.19	0.26	0.31	0.17	0.20
LBS04/03	-0.02	15 Min	-0.10	0.02	0.00	0.12	-0.01
LBS05/01	0.14	15 Min	0.32	0.05	0.02	0.26	0.08
LBS06/01	0.11	20 Min	0.31	0.13	0.04	0.22	0.07
LBS06/02	0.30	15 Min	0.74	0.22	0.16	0.17	0.12
C- Pilot Street							
LCS01/01	0.08	15 Min	0.48	0.22	0.11	0.22	0.10
LCS01/02	0.79	15 Min	0.78	0.22	0.17	0.16	0.12
LCS01/03	0.98	15 Min	0.78	0.16	0.12	0.30	0.23
LCS01/04	1.41	15 Min	0.67	0.30	0.20	0.32	0.22
LCS01/05	1.06	20 Min	0.62	0.32	0.20	0.15	0.09
LCS01/06	0.00	15 Min	0.00	0.15	0.00	0.13	0.00
LCS01/06D2	0.67	20 Min	1.47	0.15	0.22	0.28	0.41
LCS01/07	0.36	30 Min	0.57	0.13	0.08	0.20	0.11
LCS01/07D2	-0.46	25 Min	-0.81	0.13	-0.11	0.28	-0.22
LCS01/08	0.01	30 Min	0.19	0.20	0.04	0.08	0.02
LCS01/09D3	0.24	15 Min	0.24	0.08	0.02	0.16	0.04
LCS01/10C1	0.16	15 Min	0.48	0.11	0.05	0.08	0.04
LCS01/11	0.00	15 Min	0.00	0.00	0.00	0.31	0.00
LCS01/11B1	0.00	15 Min	0.00	0.00	0.00	0.12	0.00
LCS01/11C1	0.00	15 Min	0.00	0.00	0.00	0.11	0.00
LCS01/OUT	0.00	15 Min	0.00	0.00	0.00	0.96	0.00
LCS02/01	0.00	15 Min	0.00	0.00	0.00	0.22	0.00
LCS02/01C3	0.00	15 Min	0.00	0.00	0.00	0.05	0.00
LCS03/01	0.42	15 Min	0.26	0.05	0.01	0.37	0.10
LCS03/01D1	0.02	15 Min	0.23	0.05	0.01	0.05	0.01
LCS03/02	0.46	20 Min	0.37	0.37	0.14	0.30	0.11
LCS04/01	-0.03	45 Min	-0.03	0.03	0.00	0.31	-0.01
LCS04/01D8	0.02	60 Min	0.17	0.03	0.01	0.17	0.03
D- Guard Street							
LDS01/01	0.01	20 Min	0.18	0.05	0.01	0.27	0.05
LDS01/02	0.15	15 Min	2.58	0.27	0.71	0.10	0.26
LDS01/02D6	0.00	15 Min	0.00	0.27	0.00	0.00	0.00
LDS01/03	0.25	15 Min	0.61	0.10	0.06	0.20	0.12
LDS01/03D2	-0.21	30 Min	-0.25	0.10	-0.03	0.39	-0.10
LDS01/04	0.55	30 Min	0.90	0.20	0.18	0.36	0.32
LDS01/06	0.41	30 Min	0.45	0.18	0.08	0.36	0.16
LDS01/07	0.44	25 Min	0.39	0.16	0.06	0.18	0.07
LDS02/01	-0.18	20 Min	0.14	0.28	0.04	0.49	0.07
LDS02/03	-0.22	25 Min	0.14	0.25	0.04	0.49	0.07
LDS02/03D2	-0.26	15 Min	0.31	0.25	0.08	0.39	0.12
LDS03/01	0.22	15 Min	0.53	0.16	0.09	0.10	0.05
LDS03/02	0.11	20 Min	0.17	0.10	0.02	0.36	0.06
LDS03/02D7	0.29	20 Min	0.21	0.10	0.02	0.33	0.07
LDS03/04	-0.42	30 Min	-0.26	0.16	-0.04	0.36	-0.09
LDS03/04D1	0.41	30 Min	0.46	0.16	0.07	0.20	0.09
LDS04/01	0.31	15 Min	0.31	0.03	0.01	0.14	0.04
LDS04/02	0.90	15 Min	1.95	0.14	0.27	0.14	0.28
LDS04/03	0.58	15 Min	1.55	0.14	0.22	0.19	0.29
LDS04/03D6	0.34	15 Min	0.55	0.14	0.08	0.33	0.18
LDS04/04	0.75	15 Min	0.85	0.19	0.16	0.59	0.50

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)	U/S Node Depth (m)	U/S Node v.d (m ² /s)	D/S Node Depth (m)	D/S Node v.d (m ² /s)
LDS04/OUT	0.08	25 Min	1.19	0.59	0.70	0.96	1.14
LDS05/01	0.55	15 Min	1.69	0.24	0.41	0.14	0.23
LDS06/01	0.00	15 Min	0.00	0.00	0.00	0.24	0.00
LDS06/02	0.35	15 Min	0.57	0.24	0.14	0.33	0.19
LDS06/03	1.13	20 Min	0.94	0.33	0.31	0.36	0.34
LDS07/02	-0.29	30 Min	-0.16	0.16	-0.03	0.33	-0.05
LDS07/02D1	0.26	30 Min	0.65	0.16	0.10	0.18	0.12
LDS08/02	-0.02	15 Min	-0.05	0.02	0.00	0.17	-0.01
LDS08/02D1	0.01	20 Min	0.02	0.02	0.00	0.16	0.00
LDS09/01	0.68	25 Min	0.81	0.21	0.17	0.16	0.13
LDS09/01D4	-0.69	25 Min	-0.20	0.21	-0.04	0.59	-0.12

TABLE C9

Peak Flows and Velocities- Underground Network- 10 Year Event

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)
A- Elizabeth Street			
LAU01/02	0.13	20 Min	0.83
LAU01/04	0.25	25 Min	0.88
LAU01/04A	0.21	25 Min	0.86
LAU01/05	0.39	25 Min	1.38
LAU01/05A	0.15	15 Min	0.85
LAU01/06	0.84	25 Min	1.31
LAU01/06A	0.34	20 Min	0.98
LAU01/07	0.85	25 Min	1.34
LAU01/07A	0.02	20 Min	0.22
LAU01/08	1.19	20 Min	1.87
LAU01/08A	0.15	20 Min	0.67
LAU01/09	2.01	20 Min	1.57
LAU01/09A	0.86	20 Min	0.67
LAU01/10	2.29	20 Min	1.20
LAU01/10A	0.02	90 Min	0.06
LAU01/11	2.67	20 Min	0.76
LAU01/11A	0.41	20 Min	0.47
LAU01/12	2.74	20 Min	0.80
LAU01/12A	0.08	45 Min	0.34
LAU03/03	0.12	20 Min	1.11
LAU05/02	0.04	20 Min	0.68
LAU05/04	0.09	20 Min	0.90
LAU05/05	0.06	20 Min	0.90
LAU05/06	0.14	15 Min	1.40
LAU07/02	0.15	20 Min	0.98
LAU07/03	0.17	15 Min	1.16
LAU08/02	0.02	25 Min	0.66
LAU08/03	0.27	25 Min	1.25
LAU12/02	0.14	45 Min	1.97
B- Pier Street			
LBU01/04	0.41	25 Min	1.41
LBU01/04A	0.40	25 Min	0.83
LBU01/05	0.80	20 Min	1.79
LBU01/05A	0.39	20 Min	1.08
LBU01/06	0.80	20 Min	1.80
LBU01/07	0.93	25 Min	1.46
LBU01/07A	0.17	15 Min	1.18
LBU01/08	1.22	25 Min	1.05
LBU01/08A	0.33	15 Min	0.37
LBU01/10	1.26	25 Min	1.11
LBU01/10A	0.05	15 Min	0.34
LBU01/11	1.37	20 Min	1.07
LBU01/11A	0.12	15 Min	0.22
LBU01/12	2.21	20 Min	1.15
LBU01/12A	1.14	20 Min	0.71
LBU01/12LF	0.39	20 Min	0.87
LBU04/02	0.09	20 Min	1.04
LBU05/01	0.01	15 Min	0.42
LBU06/01	0.19	30 Min	1.15
LBU1	0.19	30 Min	2.56
C- Pilot Street			
LCU01/04	0.18	90 Min	1.04
LCU01/05	0.61	90 Min	2.09
LCU01/05A	0.47	15 Min	1.46

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)
LCU01/06	0.94	15 Min	1.12
LCU01/06A	0.43	25 Min	1.33
LCU01/07	0.94	15 Min	1.11
LCU01/08	1.27	30 Min	1.46
LCU01/09	1.33	30 Min	1.53
LCU01/09A	0.09	15 Min	0.53
LCU01/11	1.33	30 Min	1.53
LCU03/02	0.08	90 Min	1.08
LCU04/02	0.08	60 Min	1.71
LCU1	0.08	60 Min	0.56
D- Guard Street			
LDU01/02	0.33	20 Min	1.74
LDU01/03	0.35	20 Min	2.16
LDU01/04	0.59	30 Min	1.10
LDU01/04A	0.37	30 Min	1.65
LDU01/05	2.12	30 Min	0.94
LDU01/05A	1.58	30 Min	0.70
LDU01/06	2.49	30 Min	1.10
LDU01/06A	0.37	25 Min	1.16
LDU01/07	2.95	25 Min	1.28
LDU01/07A	0.47	25 Min	1.45
LDU02/02	0.08	15 Min	1.14
LDU02/03	0.33	30 Min	1.16
LDU02/04	0.08	15 Min	1.20
LDU03/01	0.03	15 Min	0.50
LDU03/03	0.27	90 Min	0.96
LDU04/01	0.00	15 Min	0.34
LDU04/02	0.17	15 Min	3.05
LDU04/03	0.20	15 Min	2.42
LDU04/03A	0.04	15 Min	0.62
LDU04/04	0.18	15 Min	1.65
LDU04/04A	0.07	15 Min	0.63
LDU04/05	0.19	20 Min	1.67
LDU04/05A	0.04	15 Min	1.32
LDU05/01	0.13	15 Min	1.46
LDU06/03	0.07	60 Min	0.87
LDU07/01	0.42	25 Min	1.49
LDU08/01	0.57	25 Min	2.01
LDU1	0.09	15 Min	0.77

TABLE C10
Peak Water Levels- Surface Network- 20 Year Event

Node Name	Invert Level (m AHD)	Peak Level (m AHD)	Critical Duration	Maximum Depth (m)
A- Elizabeth Street				
NAS01/01	4.655	4.655	15 Min	0.000
NAS01/02	4.000	4.052	15 Min	0.052
NAS01/03	4.194	4.194	15 Min	0.000
NAS01/04	3.800	3.966	20 Min	0.166
NAS01/05	3.651	3.832	20 Min	0.181
NAS01/06	3.610	3.813	20 Min	0.203
NAS01/07	3.323	3.456	20 Min	0.133
NAS01/08	3.003	3.228	25 Min	0.225
NAS01/09	2.799	3.230	25 Min	0.431
NAS01/10	3.167	3.234	15 Min	0.067
NAS01/11	2.537	2.673	30 Min	0.136
NAS01/12	2.836	3.018	45 Min	0.182
NAS02/01	4.745	4.745	15 Min	0.000
NAS02/02	4.377	4.467	15 Min	0.090
NAS03/01	4.368	4.368	15 Min	0.000
NAS03/02	4.110	4.265	15 Min	0.155
NAS03/03	3.943	4.089	15 Min	0.146
NAS04/01	4.373	4.373	15 Min	0.000
NAS05/01	4.310	4.327	15 Min	0.017
NAS05/02	4.180	4.304	15 Min	0.124
NAS05/03	4.100	4.198	15 Min	0.098
NAS05/04	3.800	3.927	20 Min	0.127
NAS05/05	3.602	3.758	20 Min	0.156
NAS05/06	3.450	3.584	20 Min	0.134
NAS06/01	4.066	4.066	15 Min	0.000
NAS07/01	3.270	3.364	90 Min	0.094
NAS07/02	2.847	3.364	90 Min	0.517
NAS07/03	2.971	3.364	90 Min	0.393
NAS07/04	3.426	3.426	15 Min	0.000
NAS08/01	3.833	3.833	15 Min	0.000
NAS08/02	3.202	3.354	25 Min	0.152
NAS08/03	2.900	3.171	30 Min	0.271
NAS09/01	3.917	3.917	15 Min	0.000
NAS10/01	3.288	3.288	15 Min	0.000
NAS11/01	3.506	3.506	15 Min	0.000
NAS12/01	3.250	3.250	15 Min	0.000
NAS12/02	2.722	3.131	45 Min	0.409
NAS12/03	2.935	3.100	45 Min	0.165
B- Pier Street				
NBS01/03	4.915	4.915	15 Min	0.000
NBS01/04	4.515	4.704	25 Min	0.189
NBS01/04A	4.715	4.715	15 Min	0.000
NBS01/05	4.215	4.365	20 Min	0.150
NBS01/06	4.415	4.415	15 Min	0.000
NBS01/07	3.755	3.875	15 Min	0.120
NBS01/08	3.080	3.202	15 Min	0.122
NBS01/09	3.406	3.536	15 Min	0.130
NBS01/10	2.827	2.948	15 Min	0.121
NBS01/11	2.357	2.722	30 Min	0.365
NBS01/12	2.036	2.720	30 Min	0.684
NBS02/01	4.300	4.345	15 Min	0.045
NBS03/01	4.090	4.090	15 Min	0.000
NBS04/01	3.620	3.620	15 Min	0.000
NBS04/02	3.173	3.431	20 Min	0.258

Node Name	Invert Level (m AHD)	Peak Level (m AHD)	Critical Duration	Maximum Depth (m)
NBS04/03	3.502	3.526	15 Min	0.024
NBS05/01	4.040	4.095	15 Min	0.055
NBS06/01	2.829	2.979	20 Min	0.150
NBS06/02	2.477	2.720	25 Min	0.243
C- Pilot Street				
NCS01/01	6.275	6.517	20 Min	0.242
NCS01/02	5.088	5.322	15 Min	0.234
NCS01/03	4.607	4.775	15 Min	0.168
NCS01/04	3.589	3.928	20 Min	0.339
NCS01/05	3.500	3.871	20 Min	0.371
NCS01/06	3.452	3.644	20 Min	0.192
NCS01/07	3.403	3.599	30 Min	0.196
NCS01/08	3.134	3.597	30 Min	0.463
NCS01/09	3.050	3.150	45 Min	0.100
NCS01/10	3.366	3.492	15 Min	0.126
NCS01/11	3.665	3.665	15 Min	0.000
NCS01/12	0.544	2.400	15 Min	1.856
NCS02/01	5.750	5.750	15 Min	0.000
NCS03/01	5.520	5.578	15 Min	0.058
NCS03/02	3.600	4.002	20 Min	0.402
NCS04/01	3.447	3.498	60 Min	0.051
NCS04/02	3.175	3.499	60 Min	0.324
D- Guard Street				
NDS01/01	5.500	5.559	20 Min	0.059
NDS01/02	5.181	5.506	15 Min	0.325
NDS01/03	3.339	3.475	30 Min	0.136
NDS01/04	2.159	2.798	60 Min	0.639
NDS01/05	1.900	2.798	60 Min	0.898
NDS01/06	2.100	2.798	60 Min	0.698
NDS01/07	2.200	2.799	60 Min	0.599
NDS02/01	3.273	3.615	30 Min	0.342
NDS02/02	3.061	3.615	30 Min	0.554
NDS02/03	3.294	3.604	30 Min	0.310
NDS02/04	3.151	3.601	30 Min	0.450
NDS03/01	2.950	3.134	45 Min	0.184
NDS03/02	2.690	2.811	60 Min	0.121
NDS03/03	2.412	2.811	60 Min	0.399
NDS03/04	2.582	2.799	60 Min	0.217
NDS04/01	10.873	10.905	15 Min	0.032
NDS04/02	9.750	9.895	15 Min	0.145
NDS04/03	6.041	6.196	15 Min	0.155
NDS04/04	3.481	3.688	15 Min	0.207
NDS04/05	2.226	2.845	20 Min	0.619
NDS04/06	0.545	2.400	15 Min	1.855
NDS05/01	10.341	10.602	15 Min	0.261
NDS06/01	4.000	4.000	15 Min	0.000
NDS06/02	3.114	3.370	15 Min	0.256
NDS06/03	2.653	2.960	15 Min	0.307
NDS07/01	2.392	2.808	60 Min	0.416
NDS07/02	2.554	2.799	60 Min	0.245
NDS08/01	2.351	2.799	60 Min	0.448
NDS08/02	2.500	2.799	60 Min	0.299
NDS09/01	2.588	2.820	20 Min	0.232

TABLE C11
Peak Water Levels- Underground Network- 20 Year Event

Node Name	Invert Level (m AHD)	Peak Level m AHD	Critical Duration	Maximum Depth (m)	Surface IL (m AHD)	Surcharge?	Surface Water Lvl (m AHD)
A- Elizabeth Street							
NAU01/02	2.100	4.044	20 Min	1.944	4.000	SURCH	4.052
NAU01/04	1.800	3.904	20 Min	2.104	3.800	SURCH	3.966
NAU01/04A	2.000	3.958	20 Min	1.958	3.800	SURCH	3.966
NAU01/05	1.600	3.829	20 Min	2.229	3.651	SURCH	3.832
NAU01/05A	1.800	3.832	20 Min	2.032	3.651	SURCH	3.832
NAU01/06	1.500	3.647	25 Min	2.147	3.610	SURCH	3.813
NAU01/06A	1.700	3.792	20 Min	2.092	3.610	SURCH	3.813
NAU01/07	1.290	3.446	25 Min	2.156	3.323	SURCH	3.456
NAU01/07A	1.500	3.453	25 Min	1.953	3.323	SURCH	3.456
NAU01/08	1.020	3.273	25 Min	2.253	3.003	SURCH	3.228
NAU01/08A	1.200	3.234	25 Min	2.034	3.003	SURCH	3.228
NAU01/09	0.940	3.081	25 Min	2.141	2.799	SURCH	3.230
NAU01/09A	1.200	3.208	25 Min	2.008	2.799	SURCH	3.230
NAU01/10	0.530	2.706	30 Min	2.176	3.167	-	3.234
NAU01/10A	0.700	2.706	30 Min	2.006	3.167	-	3.234
NAU01/11	0.190	2.511	30 Min	2.321	2.537	<150	2.673
NAU01/11A	0.400	2.614	30 Min	2.214	2.537	SURCH	2.673
NAU01/12	0.100	2.483	30 Min	2.383	2.836	-	3.018
NAU01/12A	0.300	2.515	30 Min	2.215	2.836	-	3.018
NAU03/03	2.500	4.078	15 Min	1.578	3.943	SURCH	4.089
NAU05/02	3.032	4.014	20 Min	0.982	4.180	-	4.304
NAU05/04	2.690	3.914	20 Min	1.224	3.800	SURCH	3.927
NAU05/05	2.672	3.485	25 Min	0.813	3.602	<150	3.758
NAU05/06	2.680	3.580	20 Min	0.900	3.450	SURCH	3.584
NAU06/01	1.529	2.974	25 Min	1.445	2.829	SURCH	2.979
NAU07/02	2.165	3.364	90 Min	1.199	2.847	SURCH	3.364
NAU07/03	2.021	3.295	90 Min	1.274	2.971	SURCH	3.364
NAU08/02	2.562	3.196	30 Min	0.634	3.202	<150	3.354
NAU08/03	2.291	3.156	30 Min	0.865	2.900	SURCH	3.171
NAU12/02	2.113	3.126	45 Min	1.013	2.722	SURCH	3.131
NAUOUT1	0.000	2.400	15 Min	2.400			
NAUOUT2	0.744	2.400	15 Min	1.656			
B- Pier Street							
NBU01/04	2.000	4.621	25 Min	2.621	4.515	SURCH	4.704
NBU01/04A	2.200	4.698	25 Min	2.498	4.515	SURCH	4.704
NBU01/05	1.800	4.276	20 Min	2.476	4.215	SURCH	4.365
NBU01/05A	2.200	4.341	20 Min	2.141	4.215	SURCH	4.365
NBU01/06	1.600	3.796	20 Min	2.196			
NBU01/07	1.500	3.566	20 Min	2.066	3.755	-	3.875
NBU01/07A	2.200	3.675	20 Min	1.475	3.755	<150	3.875
NBU01/08	1.000	3.126	20 Min	2.126	3.080	SURCH	3.202
NBU01/08A	1.300	3.171	20 Min	1.871	3.080	SURCH	3.202
NBU01/10	0.700	2.903	25 Min	2.203	2.827	SURCH	2.948
NBU01/10A	1.000	2.929	20 Min	1.929	2.827	SURCH	2.948
NBU01/11	0.500	2.776	25 Min	2.276	2.357	SURCH	2.722
NBU01/11A	0.800	2.741	25 Min	1.941	2.357	SURCH	2.722
NBU01/12	0.190	2.517	30 Min	2.327	2.036	SURCH	2.720
NBU01/12A	0.250	2.640	30 Min	2.390	2.036	SURCH	2.720
NBU04/02	1.853	2.886	25 Min	1.033	3.173	-	3.431
NBU05/01	2.300	2.887	25 Min	0.587	4.040	-	4.095
NBU1	0.378	2.916	25 Min	2.538			
NBUOUT1	0.350	2.400	15 Min	2.050			
NBUOUT2	1.100	2.400	15 Min	1.300			
NBUOUT2A	0.100	2.400	15 Min	2.300			

Node Name	Invert Level (m AHD)	Peak Level m AHD	Critical Duration	Maximum Depth (m)	Surface IL (m AHD)	Surcharge?	Surface Water Lvl (m AHD)
C- Pilot Street							
NCU01/04	2.979	3.875	20 Min	0.896	3.589	SURCH	3.928
NCU01/05	2.527	3.765	25 Min	1.238	3.500	SURCH	3.871
NCU01/05A	2.554	3.860	20 Min	1.306	3.500	SURCH	3.871
NCU01/06	1.583	3.490	30 Min	1.907	3.452	SURCH	3.644
NCU01/06A	1.613	3.635	25 Min	2.022	3.452	SURCH	3.644
NCU01/07	1.310	3.420	30 Min	2.110	3.403	SURCH	3.599
NCU01/08	1.270	3.343	30 Min	2.073	3.134	SURCH	3.597
NCU01/09	0.850	3.132	45 Min	2.282	3.050	SURCH	3.150
NCU01/09A	2.230	3.143	45 Min	0.913	3.050	SURCH	3.150
NCU01/11	0.270	2.846	45 Min	2.576			
NCU03/02	2.970	3.952	20 Min	0.982	3.600	SURCH	4.002
NCU04/02	2.370	3.494	60 Min	1.124	3.175	SURCH	3.499
NCU1	1.771	2.421	60 Min	0.650			
NCUOUT	0.000	2.400	15 Min	2.400			
NCUOUT2	0.000	2.400	15 Min	2.400			
D- Guard Street							
NDU01/02	4.081	5.334	25 Min	1.253	5.181	SURCH	5.506
NDU01/03	1.800	3.593	30 Min	1.793	3.339	SURCH	3.475
NDU01/04	0.500	2.701	60 Min	2.201	2.159	SURCH	2.798
NDU01/04A	0.551	2.791	60 Min	2.240	2.159	SURCH	2.798
NDU01/05	0.260	2.659	60 Min	2.399	1.900	SURCH	2.798
NDU01/05A	0.300	2.712	60 Min	2.412	1.900	SURCH	2.798
NDU01/06	0.211	2.616	60 Min	2.405	2.100	SURCH	2.798
NDU01/06A	0.291	2.771	60 Min	2.480	2.100	SURCH	2.798
NDU01/07	0.152	2.553	60 Min	2.401	2.200	SURCH	2.799
NDU01/07A	0.178	2.773	60 Min	2.595	2.200	SURCH	2.799
NDU02/02	2.387	3.615	30 Min	1.228	3.061	SURCH	3.615
NDU02/03	1.954	3.311	45 Min	1.357	3.294	SURCH	3.604
NDU02/04	2.511	3.602	30 Min	1.091	3.151	SURCH	3.601
NDU03/01	2.241	3.135	30 Min	0.894	2.950	SURCH	3.134
NDU03/03	1.602	2.840	60 Min	1.238	2.412	SURCH	2.811
NDU04/01	10.173	10.190	15 Min	0.016	10.873	-	10.905
NDU04/02	8.950	9.120	60 Min	0.170	9.750	-	9.895
NDU04/03	4.533	4.811	15 Min	0.278	6.041	-	6.196
NDU04/03A	4.733	4.911	15 Min	0.178	6.041	-	6.196
NDU04/04	2.821	3.887	15 Min	1.066	3.481	SURCH	3.688
NDU04/04A	2.921	3.848	15 Min	0.927	3.481	SURCH	3.688
NDU04/05	1.845	3.076	15 Min	1.231	2.226	SURCH	2.845
NDU04/05A	1.890	2.855	20 Min	0.965	2.226	SURCH	2.845
NDU05/01	9.571	9.901	20 Min	0.330	10.341	-	10.602
NDU06/03	2.054	2.873	45 Min	0.819	2.653	SURCH	2.960
NDU07/01	1.222	2.814	60 Min	1.592	2.392	SURCH	2.808
NDU08/01	1.160	2.781	60 Min	1.621	2.351	SURCH	2.799
NDU1	1.270	2.701	60 Min	1.431			
NDUOUT1	0.900	2.400	15 Min	1.500			
NDUOUT2	0.050	2.400	15 Min	2.350			
NDUOUT3	0.900	2.400	15 Min	1.500			

TABLE C12
Peak Flows and Velocities- Surface Network- 20 Year Event

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)	U/S Node Depth (m)	U/S Node v.d (m ² /s)	D/S Node Depth (m)	D/S Node v.d (m ² /s)
A- Elizabeth Street							
LAS01/01	0.00	15 Min	0.00	0.00	0.00	0.05	0.00
LAS01/03	0.00	15 Min	0.00	0.00	0.00	0.17	0.00
LAS01/03A1	0.00	15 Min	0.00	0.00	0.00	0.05	0.00
LAS01/04	0.21	20 Min	0.59	0.17	0.10	0.18	0.11
LAS01/05	0.23	20 Min	0.21	0.18	0.04	0.20	0.04
LAS01/06	0.33	20 Min	0.65	0.20	0.13	0.13	0.09
LAS01/07	0.37	20 Min	0.44	0.13	0.06	0.22	0.10
LAS01/08	1.24	20 Min	0.84	0.22	0.19	0.43	0.36
LAS01/10	0.12	15 Min	0.35	0.07	0.02	0.14	0.05
LAS01/10A1	-0.11	25 Min	0.21	0.07	0.01	0.43	0.09
LAS01/10A8	0.02	15 Min	0.29	0.07	0.02	0.27	0.08
LAS01/12A1	0.37	45 Min	0.87	0.18	0.16	0.14	0.12
LAS02/01	0.00	15 Min	0.00	0.00	0.00	0.09	0.00
LAS02/01B1	0.00	15 Min	0.00	0.00	0.00	0.19	0.00
LAS02/02	0.26	15 Min	0.59	0.09	0.05	0.17	0.10
LAS03/01	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LAS03/01B1	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LAS03/02	0.44	15 Min	0.65	0.15	0.10	0.15	0.09
LAS03/03	0.55	15 Min	0.81	0.15	0.12	0.20	0.16
LAS04/01A3	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LAS04/01A5	0.00	15 Min	0.00	0.00	0.00	0.10	0.00
LAS05/01	0.00	15 Min	0.03	0.02	0.00	0.12	0.00
LAS05/01B2	0.00	15 Min	-0.09	0.02	0.00	0.05	0.00
LAS05/02	0.11	15 Min	0.47	0.12	0.06	0.10	0.05
LAS05/03	0.19	15 Min	0.38	0.10	0.04	0.13	0.05
LAS05/04	0.32	20 Min	0.42	0.13	0.05	0.16	0.07
LAS05/05	0.38	20 Min	0.76	0.16	0.12	0.13	0.10
LAS05/06	0.47	20 Min	0.58	0.13	0.08	0.22	0.13
LAS06/01A5	0.00	15 Min	0.00	0.00	0.00	0.13	0.00
LAS06/01A7	0.00	15 Min	0.00	0.00	0.00	0.52	0.00
LAS07/01	0.16	15 Min	0.31	0.09	0.03	0.52	0.16
LAS07/03A2	0.21	15 Min	0.41	0.39	0.16	0.52	0.21
LAS07/04	0.00	15 Min	0.00	0.00	0.00	0.07	0.00
LAS07/04A7	0.00	15 Min	0.00	0.00	0.00	0.39	0.00
LAS08/01	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LAS08/02	0.21	25 Min	0.77	0.15	0.12	0.27	0.21
LAS09/01	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LAS10/01	0.00	15 Min	0.00	0.00	0.00	0.18	0.00
LAS10/01B6	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LAS11/01	0.00	15 Min	0.00	0.00	0.00	0.39	0.00
LAS11/01B6	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LAS12/01	0.00	15 Min	0.00	0.00	0.00	0.41	0.00
LAS12/03	-0.34	45 Min	-0.20	0.17	-0.03	0.41	-0.08
LAS12/03A1	0.34	45 Min	0.52	0.17	0.09	0.18	0.09
B- Pier Street							
LBS01/03	0.00	15 Min	0.00	0.00	0.00	0.19	0.00
LBS01/04	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LBS01/04B1	0.00	15 Min	0.00	0.00	0.00	0.19	0.00
LBS01/06	0.00	15 Min	0.00	0.00	0.00	0.12	0.00
LBS01/06B1	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LBS01/07	0.16	15 Min	0.77	0.12	0.09	0.12	0.09
LBS01/09	0.10	15 Min	0.81	0.13	0.10	0.12	0.10
LBS01/09B1	0.14	15 Min	0.66	0.13	0.09	0.12	0.08
LBS01/10	0.12	15 Min	0.48	0.12	0.06	0.36	0.18

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)	U/S Node Depth (m)	U/S Node v.d (m ² /s)	D/S Node Depth (m)	D/S Node v.d (m ² /s)
LBS01/11	0.43	20 Min	0.13	0.36	0.05	0.68	0.09
LBS02/01	0.01	15 Min	0.24	0.05	0.01	0.12	0.03
LBS02/01B5	0.06	15 Min	0.35	0.05	0.02	0.05	0.02
LBS03/01	0.00	15 Min	0.00	0.00	0.00	0.12	0.00
LBS03/01C1	0.00	15 Min	0.00	0.00	0.00	0.20	0.00
LBS04/01	0.00	15 Min	0.00	0.00	0.00	0.26	0.00
LBS04/01A7	0.00	15 Min	0.00	0.00	0.00	0.09	0.00
LBS04/02B1	0.53	20 Min	0.67	0.26	0.17	0.68	0.46
LBS04/03	-0.03	15 Min	-0.12	0.02	0.00	0.13	-0.02
LBS05/01	0.17	15 Min	0.35	0.05	0.02	0.26	0.09
LBS06/01	0.15	20 Min	0.35	0.15	0.05	0.24	0.09
LBS06/02	0.35	20 Min	0.14	0.24	0.03	0.68	0.10
C- Pilot Street							
LCS01/01	0.10	15 Min	0.50	0.24	0.12	0.23	0.12
LCS01/02	0.95	15 Min	0.83	0.23	0.20	0.17	0.14
LCS01/03	1.17	15 Min	0.81	0.17	0.14	0.34	0.27
LCS01/04	1.74	20 Min	0.68	0.34	0.23	0.37	0.25
LCS01/05	1.61	20 Min	0.70	0.37	0.26	0.19	0.13
LCS01/06	0.00	15 Min	0.00	0.19	0.00	0.20	0.00
LCS01/06D2	1.39	20 Min	1.73	0.19	0.33	0.34	0.59
LCS01/07	0.71	30 Min	0.63	0.20	0.12	0.46	0.29
LCS01/07D2	-0.87	25 Min	-0.90	0.20	-0.18	0.34	-0.31
LCS01/08	0.15	30 Min	0.45	0.46	0.21	0.10	0.04
LCS01/09D3	0.31	45 Min	0.26	0.10	0.03	0.18	0.05
LCS01/10C1	0.20	15 Min	0.51	0.13	0.06	0.10	0.05
LCS01/11	0.00	15 Min	0.00	0.00	0.00	0.32	0.00
LCS01/11B1	0.00	15 Min	0.00	0.00	0.00	0.36	0.00
LCS01/11C1	0.00	15 Min	0.00	0.00	0.00	0.13	0.00
LCS01/OUT	0.00	15 Min	0.00	0.00	0.00	1.86	0.00
LCS02/01	0.00	15 Min	0.00	0.00	0.00	0.23	0.00
LCS02/01C3	0.00	15 Min	0.00	0.00	0.00	0.06	0.00
LCS03/01	0.50	15 Min	0.28	0.06	0.02	0.40	0.11
LCS03/01D1	0.02	15 Min	0.26	0.06	0.01	0.06	0.02
LCS03/02	0.59	20 Min	0.37	0.40	0.15	0.34	0.12
LCS04/01	-0.05	30 Min	-0.05	0.05	0.00	0.32	-0.02
LCS04/01D8	0.04	60 Min	0.02	0.05	0.00	0.45	0.01
D- Guard Street							
LDS01/01	0.01	20 Min	0.19	0.06	0.01	0.32	0.06
LDS01/02	0.23	15 Min	2.94	0.32	0.96	0.14	0.40
LDS01/02D6	0.00	15 Min	0.00	0.32	0.00	0.00	0.00
LDS01/03	0.49	30 Min	0.17	0.14	0.02	0.64	0.11
LDS01/03D2	-0.37	30 Min	-0.31	0.14	-0.04	0.45	-0.14
LDS01/04	0.83	30 Min	0.10	0.64	0.06	0.90	0.09
LDS01/06	-0.64	20 Min	-0.12	0.70	-0.08	0.90	-0.11
LDS01/07	-0.64	30 Min	-0.11	0.60	-0.06	0.70	-0.07
LDS02/01	0.32	30 Min	0.14	0.34	0.05	0.55	0.08
LDS02/03	-0.42	30 Min	-0.16	0.31	-0.05	0.55	-0.09
LDS02/03D2	-0.33	15 Min	-0.32	0.31	-0.10	0.45	-0.14
LDS03/01	0.30	45 Min	0.58	0.18	0.11	0.12	0.07
LDS03/02	0.13	20 Min	0.16	0.12	0.02	0.40	0.06
LDS03/02D7	0.34	20 Min	0.19	0.12	0.02	0.42	0.08
LDS03/04	-0.56	30 Min	-0.30	0.22	-0.07	0.40	-0.12
LDS03/04D1	0.53	30 Min	0.13	0.22	0.03	0.64	0.08
LDS04/01	0.38	15 Min	0.34	0.03	0.01	0.15	0.05
LDS04/02	1.12	15 Min	1.95	0.15	0.28	0.15	0.30
LDS04/03	0.71	15 Min	1.47	0.15	0.23	0.21	0.30
LDS04/03D6	0.44	15 Min	0.90	0.15	0.14	0.31	0.28
LDS04/04	0.94	15 Min	0.44	0.21	0.09	0.62	0.27

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)	U/S Node Depth (m)	U/S Node v.d (m ² /s)	D/S Node Depth (m)	D/S Node v.d (m ² /s)
LDS04/OUT	0.14	20 Min	1.37	0.62	0.85	1.86	2.55
LDS05/01	0.69	15 Min	1.76	0.26	0.46	0.15	0.26
LDS06/01	0.00	15 Min	0.00	0.00	0.00	0.26	0.00
LDS06/02	0.44	15 Min	0.59	0.26	0.15	0.31	0.18
LDS06/03	1.30	15 Min	0.34	0.31	0.10	0.90	0.30
LDS07/02	-0.55	45 Min	-0.23	0.25	-0.06	0.42	-0.09
LDS07/02D1	0.51	45 Min	0.13	0.25	0.03	0.70	0.09
LDS08/02	0.31	30 Min	-0.16	0.30	-0.05	0.45	-0.07
LDS08/02D1	-0.80	30 Min	-0.12	0.30	-0.03	0.60	-0.07
LDS09/01	0.89	20 Min	0.26	0.23	0.06	0.60	0.16
LDS09/01D4	-0.94	20 Min	-0.25	0.23	-0.06	0.62	-0.16

TABLE C13

Peak Flows and Velocities- Underground Network- 20 Year Event

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)
A- Elizabeth Street			
LAU01/02	0.12	60 Min	0.73
LAU01/04	0.24	45 Min	0.84
LAU01/04A	0.22	15 Min	0.68
LAU01/05	0.33	60 Min	1.13
LAU01/05A	0.15	15 Min	0.53
LAU01/06	0.83	15 Min	1.30
LAU01/06A	0.47	15 Min	0.81
LAU01/07	0.87	15 Min	1.35
LAU01/07A	0.04	15 Min	0.35
LAU01/08	0.95	15 Min	1.49
LAU01/08A	0.13	25 Min	0.56
LAU01/09	1.87	25 Min	1.46
LAU01/09A	1.18	25 Min	0.92
LAU01/10	2.16	25 Min	1.12
LAU01/10A	0.03	15 Min	0.08
LAU01/11	2.72	30 Min	0.80
LAU01/11A	0.58	30 Min	0.67
LAU01/12	2.81	30 Min	0.82
LAU01/12A	0.08	60 Min	0.37
LAU03/03	0.11	30 Min	1.01
LAU05/02	0.05	20 Min	0.70
LAU05/04	0.09	20 Min	0.84
LAU05/05	0.06	20 Min	0.58
LAU05/06	0.12	30 Min	1.04
LAU07/02	0.08	45 Min	0.50
LAU07/03	0.14	90 Min	0.84
LAU08/02	0.03	15 Min	0.44
LAU08/03	0.28	25 Min	0.97
LAU12/02	0.10	45 Min	1.33
B- Pier Street			
LBU01/04	0.50	25 Min	1.73
LBU01/04A	0.50	25 Min	0.86
LBU01/05	0.80	20 Min	1.78
LBU01/05A	0.48	20 Min	0.56
LBU01/06	0.80	20 Min	1.79
LBU01/07	0.96	20 Min	1.50
LBU01/07A	0.22	20 Min	0.65
LBU01/08	1.37	20 Min	1.20
LBU01/08A	0.42	15 Min	0.47
LBU01/10	1.43	20 Min	1.25
LBU01/10A	0.08	15 Min	0.46
LBU01/11	1.14	25 Min	0.89
LBU01/11A	-0.32	15 Min	-0.56
LBU01/12	2.12	30 Min	0.84
LBU01/12A	1.22	30 Min	0.75
LBU01/12LF	0.29	30 Min	0.65
LBU04/02	0.09	20 Min	0.83
LBU05/01	0.01	15 Min	0.08
LBU06/01	0.13	25 Min	0.79
LBU1	0.13	25 Min	1.73
C- Pilot Street			
LCU01/04	0.16	90 Min	1.05
LCU01/05	0.47	90 Min	1.64
LCU01/05A	0.34	30 Min	1.05

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)
LCU01/06	0.76	30 Min	0.88
LCU01/06A	0.37	30 Min	1.13
LCU01/07	0.76	30 Min	0.88
LCU01/08	1.09	30 Min	1.25
LCU01/09	1.15	45 Min	1.31
LCU01/09A	0.11	15 Min	0.32
LCU01/11	1.15	45 Min	1.31
LCU03/02	0.07	90 Min	1.01
LCU04/02	0.10	60 Min	1.35
LCU1	0.10	60 Min	0.33
D- Guard Street			
LDU01/02	0.34	20 Min	1.52
LDU01/03	0.30	20 Min	1.85
LDU01/04	0.41	30 Min	0.75
LDU01/04A	0.16	60 Min	0.72
LDU01/05	1.55	60 Min	0.68
LDU01/05A	1.21	60 Min	0.53
LDU01/06	1.80	60 Min	0.79
LDU01/06A	0.26	60 Min	0.79
LDU01/07	2.11	60 Min	0.93
LDU01/07A	0.32	60 Min	0.97
LDU02/02	0.07	90 Min	0.92
LDU02/03	0.38	30 Min	1.33
LDU02/04	0.07	15 Min	1.00
LDU03/01	0.02	15 Min	0.20
LDU03/03	0.14	90 Min	0.48
LDU04/01	0.00	15 Min	0.31
LDU04/02	0.17	15 Min	3.05
LDU04/03	0.21	15 Min	2.53
LDU04/03A	0.04	15 Min	0.63
LDU04/04	0.16	15 Min	1.44
LDU04/04A	0.07	15 Min	0.63
LDU04/05	0.14	15 Min	1.23
LDU04/05A	-0.03	15 Min	-0.93
LDU05/01	0.13	15 Min	1.50
LDU06/03	0.06	15 Min	0.58
LDU07/01	0.19	90 Min	0.68
LDU08/01	0.39	60 Min	1.38
LDU1	0.06	15 Min	0.57

TABLE C14
Peak Water Levels- Surface Network- 50 Year Event

Node Name	Invert Level (m AHD)	Peak Level (m AHD)	Critical Duration	Maximum Depth (m)
A- Elizabeth Street				
NAS01/01	4.655	4.655	15 Min	0.000
NAS01/02	4.000	4.072	25 Min	0.072
NAS01/03	4.194	4.194	15 Min	0.000
NAS01/04	3.800	4.002	20 Min	0.202
NAS01/05	3.651	3.888	20 Min	0.237
NAS01/06	3.610	3.870	20 Min	0.260
NAS01/07	3.323	3.495	20 Min	0.172
NAS01/08	3.003	3.332	25 Min	0.329
NAS01/09	2.799	3.333	25 Min	0.534
NAS01/10	3.167	3.315	30 Min	0.148
NAS01/11	2.537	2.921	30 Min	0.384
NAS01/12	2.836	3.027	30 Min	0.191
NAS02/01	4.745	4.773	30 Min	0.028
NAS02/02	4.377	4.483	15 Min	0.106
NAS03/01	4.368	4.424	25 Min	0.056
NAS03/02	4.110	4.283	15 Min	0.173
NAS03/03	3.943	4.116	15 Min	0.173
NAS04/01	4.373	4.373	15 Min	0.000
NAS05/01	4.310	4.331	15 Min	0.021
NAS05/02	4.180	4.314	15 Min	0.134
NAS05/03	4.100	4.217	15 Min	0.117
NAS05/04	3.800	3.940	20 Min	0.140
NAS05/05	3.602	3.778	20 Min	0.176
NAS05/06	3.450	3.604	20 Min	0.154
NAS06/01	4.066	4.066	15 Min	0.000
NAS07/01	3.270	3.458	90 Min	0.188
NAS07/02	2.847	3.456	90 Min	0.609
NAS07/03	2.971	3.457	90 Min	0.486
NAS07/04	3.426	3.457	90 Min	0.031
NAS08/01	3.833	3.833	15 Min	0.000
NAS08/02	3.202	3.369	20 Min	0.167
NAS08/03	2.900	3.323	30 Min	0.423
NAS09/01	3.917	3.917	15 Min	0.000
NAS10/01	3.288	3.288	15 Min	0.000
NAS11/01	3.506	3.506	15 Min	0.000
NAS12/01	3.250	3.250	15 Min	0.000
NAS12/02	2.722	3.166	30 Min	0.444
NAS12/03	2.935	3.126	30 Min	0.191
B- Pier Street				
NBS01/03	4.915	4.915	15 Min	0.000
NBS01/04	4.515	4.773	30 Min	0.258
NBS01/04A	4.715	4.765	30 Min	0.050
NBS01/05	4.215	4.434	25 Min	0.219
NBS01/06	4.415	4.433	25 Min	0.018
NBS01/07	3.755	3.886	15 Min	0.131
NBS01/08	3.080	3.289	20 Min	0.209
NBS01/09	3.406	3.543	15 Min	0.137
NBS01/10	2.827	2.966	20 Min	0.139
NBS01/11	2.357	2.818	30 Min	0.461
NBS01/12	2.036	2.816	30 Min	0.780
NBS02/01	4.300	4.352	15 Min	0.052
NBS03/01	4.090	4.090	15 Min	0.000
NBS04/01	3.620	3.620	15 Min	0.000
NBS04/02	3.173	3.462	20 Min	0.289

Node Name	Invert Level (m AHD)	Peak Level (m AHD)	Critical Duration	Maximum Depth (m)
NBS04/03	3.502	3.531	15 Min	0.029
NBS05/01	4.040	4.104	15 Min	0.064
NBS06/01	2.829	2.996	20 Min	0.167
NBS06/02	2.477	2.816	30 Min	0.339
C- Pilot Street				
NCS01/01	6.275	6.533	20 Min	0.258
NCS01/02	5.088	5.345	15 Min	0.257
NCS01/03	4.607	4.795	15 Min	0.188
NCS01/04	3.589	3.976	15 Min	0.387
NCS01/05	3.500	3.921	20 Min	0.421
NCS01/06	3.452	3.698	30 Min	0.246
NCS01/07	3.403	3.673	30 Min	0.270
NCS01/08	3.134	3.673	30 Min	0.539
NCS01/09	3.050	3.185	30 Min	0.135
NCS01/10	3.366	3.508	15 Min	0.142
NCS01/11	3.665	3.665	15 Min	0.000
NCS01/12	0.544	2.400	15 Min	1.856
NCS02/01	5.750	5.750	15 Min	0.000
NCS03/01	5.520	5.588	15 Min	0.068
NCS03/02	3.600	4.042	20 Min	0.442
NCS04/01	3.447	3.539	60 Min	0.092
NCS04/02	3.175	3.541	60 Min	0.366
D- Guard Street				
NDS01/01	5.500	5.581	15 Min	0.081
NDS01/02	5.181	5.571	15 Min	0.390
NDS01/03	3.339	3.502	30 Min	0.163
NDS01/04	2.159	2.906	60 Min	0.747
NDS01/05	1.900	2.906	60 Min	1.006
NDS01/06	2.100	2.907	60 Min	0.807
NDS01/07	2.200	2.908	60 Min	0.708
NDS02/01	3.273	3.684	30 Min	0.411
NDS02/02	3.061	3.683	30 Min	0.622
NDS02/03	3.294	3.671	45 Min	0.377
NDS02/04	3.151	3.666	45 Min	0.515
NDS03/01	2.950	3.168	30 Min	0.218
NDS03/02	2.690	2.912	60 Min	0.222
NDS03/03	2.412	2.913	60 Min	0.501
NDS03/04	2.582	2.909	60 Min	0.327
NDS04/01	10.873	10.910	15 Min	0.037
NDS04/02	9.750	9.908	15 Min	0.158
NDS04/03	6.041	6.214	15 Min	0.173
NDS04/04	3.481	3.714	15 Min	0.233
NDS04/05	2.226	2.907	60 Min	0.681
NDS04/06	0.545	2.400	15 Min	1.855
NDS05/01	10.341	10.627	15 Min	0.286
NDS06/01	4.000	4.000	15 Min	0.000
NDS06/02	3.114	3.391	15 Min	0.277
NDS06/03	2.653	2.990	15 Min	0.337
NDS07/01	2.392	2.912	60 Min	0.520
NDS07/02	2.554	2.909	60 Min	0.355
NDS08/01	2.351	2.909	60 Min	0.558
NDS08/02	2.500	2.908	60 Min	0.408
NDS09/01	2.588	2.908	60 Min	0.320

TABLE C15
Peak Water Levels- Underground Network- 50 Year Event

Node Name	Invert Level (m AHD)	Peak Level m AHD	Critical Duration	Maximum Depth (m)	Surface IL (m AHD)	Surcharge?	Surface Water Lvl (m AHD)
A- Elizabeth Street							
NAU01/02	2.100	4.070	25 Min	1.970	4.000	SURCH	4.072
NAU01/04	1.800	3.949	20 Min	2.149	3.800	SURCH	4.002
NAU01/04A	2.000	3.997	20 Min	1.997	3.800	SURCH	4.002
NAU01/05	1.600	3.883	20 Min	2.283	3.651	SURCH	3.888
NAU01/05A	1.800	3.888	20 Min	2.088	3.651	SURCH	3.888
NAU01/06	1.500	3.704	25 Min	2.204	3.610	SURCH	3.870
NAU01/06A	1.700	3.847	20 Min	2.147	3.610	SURCH	3.870
NAU01/07	1.290	3.507	25 Min	2.217	3.323	SURCH	3.495
NAU01/07A	1.500	3.496	25 Min	1.996	3.323	SURCH	3.495
NAU01/08	1.020	3.354	25 Min	2.334	3.003	SURCH	3.332
NAU01/08A	1.200	3.336	25 Min	2.136	3.003	SURCH	3.332
NAU01/09	0.940	3.171	30 Min	2.231	2.799	SURCH	3.333
NAU01/09A	1.200	3.307	25 Min	2.107	2.799	SURCH	3.333
NAU01/10	0.530	2.797	30 Min	2.267	3.167	-	3.315
NAU01/10A	0.700	2.897	30 Min	2.197	3.167	-	3.315
NAU01/11	0.190	2.564	30 Min	2.374	2.537	SURCH	2.921
NAU01/11A	0.400	2.857	30 Min	2.457	2.537	SURCH	2.921
NAU01/12	0.100	2.522	30 Min	2.422	2.836	-	3.027
NAU01/12A	0.300	2.555	30 Min	2.255	2.836	-	3.027
NAU03/03	2.500	4.109	15 Min	1.609	3.943	SURCH	4.116
NAU05/02	3.032	4.050	15 Min	1.018	4.180	<150	4.314
NAU05/04	2.690	3.931	20 Min	1.241	3.800	SURCH	3.940
NAU05/05	2.672	3.594	25 Min	0.922	3.602	<150	3.778
NAU05/06	2.680	3.603	20 Min	0.923	3.450	SURCH	3.604
NAU06/01	1.529	2.993	25 Min	1.464	2.829	SURCH	2.996
NAU07/02	2.165	3.457	90 Min	1.292	2.847	SURCH	3.456
NAU07/03	2.021	3.369	90 Min	1.348	2.971	SURCH	3.457
NAU08/02	2.562	3.333	30 Min	0.771	3.202	SURCH	3.369
NAU08/03	2.291	3.302	30 Min	1.011	2.900	SURCH	3.323
NAU12/02	2.113	3.161	30 Min	1.048	2.722	SURCH	3.166
NAUOUT1	0.000	2.400	15 Min	2.400			
NAUOUT2	0.744	2.400	15 Min	1.656			
B- Pier Street							
NBU01/04	2.000	4.694	30 Min	2.694	4.515	SURCH	4.773
NBU01/04A	2.200	4.766	30 Min	2.566	4.515	SURCH	4.773
NBU01/05	1.800	4.354	25 Min	2.554	4.215	SURCH	4.434
NBU01/05A	2.200	4.416	25 Min	2.216	4.215	SURCH	4.434
NBU01/06	1.600	3.889	20 Min	2.289			
NBU01/07	1.500	3.669	20 Min	2.169	3.755	<150	3.886
NBU01/07A	2.200	3.818	20 Min	1.618	3.755	SURCH	3.886
NBU01/08	1.000	3.217	20 Min	2.217	3.080	SURCH	3.289
NBU01/08A	1.300	3.275	20 Min	1.975	3.080	SURCH	3.289
NBU01/10	0.700	2.970	25 Min	2.270	2.827	SURCH	2.966
NBU01/10A	1.000	2.968	25 Min	1.968	2.827	SURCH	2.966
NBU01/11	0.500	2.841	30 Min	2.341	2.357	SURCH	2.818
NBU01/11A	0.800	2.829	30 Min	2.029	2.357	SURCH	2.818
NBU01/12	0.190	2.547	30 Min	2.357	2.036	SURCH	2.816
NBU01/12A	0.250	2.709	30 Min	2.459	2.036	SURCH	2.816
NBU04/02	1.853	2.949	20 Min	1.096	3.173	-	3.462
NBU05/01	2.300	2.952	20 Min	0.652	4.040	-	4.104
NBU1	0.378	2.933	25 Min	2.555			
NBUOUT1	0.350	2.400	15 Min	2.050			
NBUOUT2A	0.100	2.400	15 Min	2.300			
NBUOUT2	1.100	2.400	15 Min	1.300			

Node Name	Invert Level (m AHD)	Peak Level m AHD	Critical Duration	Maximum Depth (m)	Surface IL (m AHD)	Surcharge?	Surface Water Lvl (m AHD)
C- Pilot Street							
NCU01/04	2.979	3.923	20 Min	0.944	3.589	SURCH	3.976
NCU01/05	2.527	3.813	20 Min	1.286	3.500	SURCH	3.921
NCU01/05A	2.554	3.910	20 Min	1.356	3.500	SURCH	3.921
NCU01/06	1.583	3.537	30 Min	1.954	3.452	SURCH	3.698
NCU01/06A	1.613	3.688	30 Min	2.075	3.452	SURCH	3.698
NCU01/07	1.310	3.465	30 Min	2.155	3.403	SURCH	3.673
NCU01/08	1.270	3.386	30 Min	2.116	3.134	SURCH	3.673
NCU01/09	0.850	3.167	30 Min	2.317	3.050	SURCH	3.185
NCU01/09A	2.230	3.180	30 Min	0.950	3.050	SURCH	3.185
NCU01/11	0.270	2.868	30 Min	2.598			
NCU03/02	2.970	3.992	20 Min	1.022	3.600	SURCH	4.042
NCU04/02	2.370	3.537	60 Min	1.167	3.175	SURCH	3.541
NCU1	1.771	2.422	45 Min	0.651			
NCUOUT	0.000	2.400	15 Min	2.400			
NCUOUT2	0.000	2.400	15 Min	2.400			
D- Guard Street							
NDU01/02	4.081	5.413	25 Min	1.332	5.181	SURCH	5.571
NDU01/03	1.800	3.657	30 Min	1.857	3.339	SURCH	3.502
NDU01/04	0.500	2.773	60 Min	2.273	2.159	SURCH	2.906
NDU01/04A	0.551	2.898	60 Min	2.347	2.159	SURCH	2.906
NDU01/05	0.260	2.726	60 Min	2.466	1.900	SURCH	2.906
NDU01/05A	0.300	2.794	60 Min	2.494	1.900	SURCH	2.906
NDU01/06	0.211	2.672	60 Min	2.461	2.100	SURCH	2.907
NDU01/06A	0.291	2.871	60 Min	2.580	2.100	SURCH	2.907
NDU01/07	0.152	2.593	60 Min	2.441	2.200	SURCH	2.908
NDU01/07A	0.178	2.874	60 Min	2.696	2.200	SURCH	2.908
NDU02/02	2.387	3.684	30 Min	1.297	3.061	SURCH	3.683
NDU02/03	1.954	3.448	45 Min	1.494	3.294	SURCH	3.671
NDU02/04	2.511	3.667	45 Min	1.156	3.151	SURCH	3.666
NDU03/01	2.241	3.168	30 Min	0.927	2.950	SURCH	3.168
NDU03/03	1.602	2.945	60 Min	1.343	2.412	SURCH	2.913
NDU04/01	10.173	10.192	15 Min	0.019	10.873	-	10.910
NDU04/02	8.950	9.121	15 Min	0.171	9.750	-	9.908
NDU04/03	4.533	4.845	15 Min	0.312	6.041	-	6.214
NDU04/03A	4.733	4.931	15 Min	0.198	6.041	-	6.214
NDU04/04	2.821	3.981	15 Min	1.160	3.481	SURCH	3.714
NDU04/04A	2.921	3.937	15 Min	1.016	3.481	SURCH	3.714
NDU04/05	1.845	3.122	15 Min	1.277	2.226	SURCH	2.907
NDU04/05A	1.890	2.916	60 Min	1.026	2.226	SURCH	2.907
NDU05/01	9.571	9.903	30 Min	0.332	10.341	-	10.627
NDU06/03	2.054	2.906	30 Min	0.852	2.653	SURCH	2.990
NDU07/01	1.222	2.919	60 Min	1.697	2.392	SURCH	2.912
NDU08/01	1.160	2.886	60 Min	1.726	2.351	SURCH	2.909
NDU1	1.270	2.758	60 Min	1.488			
NDUOUT1	0.900	2.400	15 Min	1.500			
NDUOUT2	0.050	2.400	15 Min	2.350			
NDUOUT3	0.900	2.400	15 Min	1.500			

TABLE C16
Peak Flows and Velocities- Surface Network- 50 Year Event

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)	U/S Node Depth (m)	U/S Node v.d (m ² /s)	D/S Node Depth (m)	D/S Node v.d (m ² /s)
A- Elizabeth Street							
LAS01/01	0.00	15 Min	0.00	0.00	0.00	0.07	0.00
LAS01/03	0.00	15 Min	0.00	0.00	0.00	0.20	0.00
LAS01/03A1	0.00	15 Min	0.00	0.00	0.00	0.07	0.00
LAS01/04	0.32	20 Min	0.61	0.20	0.12	0.24	0.14
LAS01/05	0.36	20 Min	0.23	0.24	0.05	0.26	0.06
LAS01/06	0.71	20 Min	0.76	0.26	0.20	0.17	0.13
LAS01/07	0.81	20 Min	0.48	0.17	0.08	0.33	0.16
LAS01/08	1.99	20 Min	0.86	0.33	0.28	0.53	0.46
LAS01/10	0.59	30 Min	0.37	0.15	0.05	0.38	0.14
LAS01/10A1	-0.79	25 Min	0.22	0.15	0.03	0.53	0.12
LAS01/10A8	-0.09	30 Min	0.23	0.15	0.03	0.42	0.10
LAS01/12A1	0.55	30 Min	0.89	0.19	0.17	0.38	0.34
LAS02/01	0.03	30 Min	0.17	0.03	0.00	0.11	0.02
LAS02/01B1	-0.05	30 Min	-0.06	0.03	0.00	0.26	-0.02
LAS02/02	0.32	15 Min	0.61	0.11	0.06	0.20	0.12
LAS03/01	0.07	25 Min	0.22	0.06	0.01	0.17	0.04
LAS03/01B1	-0.09	25 Min	-0.14	0.06	-0.01	0.22	-0.03
LAS03/02	0.58	15 Min	0.66	0.17	0.11	0.17	0.11
LAS03/03	0.77	15 Min	0.85	0.17	0.15	0.26	0.22
LAS04/01A3	0.00	15 Min	0.00	0.00	0.00	0.17	0.00
LAS04/01A5	0.00	15 Min	0.00	0.00	0.00	0.12	0.00
LAS05/01	0.01	15 Min	0.03	0.02	0.00	0.13	0.00
LAS05/01B2	-0.01	15 Min	-0.10	0.02	0.00	0.05	-0.01
LAS05/02	0.15	15 Min	0.48	0.13	0.06	0.12	0.06
LAS05/03	0.25	15 Min	0.40	0.12	0.05	0.14	0.06
LAS05/04	0.45	20 Min	0.47	0.14	0.07	0.18	0.08
LAS05/05	0.56	20 Min	0.82	0.18	0.14	0.15	0.13
LAS05/06	0.68	20 Min	0.63	0.15	0.10	0.33	0.21
LAS06/01A5	0.00	15 Min	0.00	0.00	0.00	0.14	0.00
LAS06/01A7	0.00	15 Min	0.00	0.00	0.00	0.61	0.00
LAS07/01	0.21	15 Min	0.34	0.19	0.06	0.61	0.20
LAS07/03A2	0.23	15 Min	0.43	0.49	0.21	0.61	0.26
LAS07/04	0.03	90 Min	0.26	0.03	0.01	0.15	0.04
LAS07/04A7	-0.05	90 Min	-0.02	0.03	0.00	0.49	-0.01
LAS08/01	0.00	15 Min	0.00	0.00	0.00	0.17	0.00
LAS08/02	0.27	20 Min	0.77	0.17	0.13	0.42	0.33
LAS09/01	0.00	15 Min	0.00	0.00	0.00	0.17	0.00
LAS10/01	0.00	15 Min	0.00	0.00	0.00	0.19	0.00
LAS10/01B6	0.00	15 Min	0.00	0.00	0.00	0.17	0.00
LAS11/01	0.00	15 Min	0.00	0.00	0.00	0.49	0.00
LAS11/01B6	0.00	15 Min	0.00	0.00	0.00	0.17	0.00
LAS12/01	0.00	15 Min	0.00	0.00	0.00	0.44	0.00
LAS12/03	-0.50	30 Min	-0.24	0.19	-0.05	0.44	-0.11
LAS12/03A1	0.49	30 Min	0.59	0.19	0.11	0.19	0.11
B- Pier Street							
LBS01/03	0.00	15 Min	0.00	0.00	0.00	0.26	0.00
LBS01/04	0.10	30 Min	0.20	0.05	0.01	0.22	0.04
LBS01/04B1	-0.10	30 Min	-0.12	0.05	-0.01	0.26	-0.03
LBS01/06	0.02	25 Min	0.12	0.02	0.00	0.13	0.02
LBS01/06B1	-0.03	25 Min	-0.06	0.02	0.00	0.22	-0.01
LBS01/07	0.26	15 Min	0.77	0.13	0.10	0.21	0.16
LBS01/09	0.13	15 Min	0.82	0.14	0.11	0.14	0.11
LBS01/09B1	0.18	15 Min	0.69	0.14	0.09	0.21	0.14
LBS01/10	0.19	20 Min	0.50	0.14	0.07	0.46	0.23

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)	U/S Node Depth (m)	U/S Node v.d (m ² /s)	D/S Node Depth (m)	D/S Node v.d (m ² /s)
LBS01/11	0.59	20 Min	0.13	0.46	0.06	0.78	0.10
LBS02/01	0.02	15 Min	0.24	0.05	0.01	0.13	0.03
LBS02/01B5	0.07	15 Min	0.39	0.05	0.02	0.06	0.02
LBS03/01	0.00	15 Min	0.00	0.00	0.00	0.13	0.00
LBS03/01C1	0.00	15 Min	0.00	0.00	0.00	0.27	0.00
LBS04/01	0.00	15 Min	0.00	0.00	0.00	0.29	0.00
LBS04/01A7	0.00	15 Min	0.00	0.00	0.00	0.19	0.00
LBS04/02B1	0.71	20 Min	0.78	0.29	0.22	0.78	0.61
LBS04/03	-0.04	15 Min	-0.13	0.03	0.00	0.14	-0.02
LBS05/01	0.22	15 Min	0.38	0.06	0.02	0.29	0.11
LBS06/01	0.20	20 Min	0.36	0.17	0.06	0.34	0.12
LBS06/02	0.50	15 Min	0.15	0.34	0.05	0.78	0.12
C- Pilot Street							
LCS01/01	0.14	15 Min	0.54	0.26	0.14	0.26	0.14
LCS01/02	1.23	15 Min	0.89	0.26	0.23	0.19	0.17
LCS01/03	1.52	15 Min	0.83	0.19	0.16	0.39	0.32
LCS01/04	2.44	15 Min	0.69	0.39	0.27	0.42	0.29
LCS01/05	2.33	20 Min	0.79	0.42	0.33	0.25	0.20
LCS01/06	0.00	15 Min	0.00	0.25	0.00	0.27	0.00
LCS01/06D2	2.13	20 Min	1.84	0.25	0.45	0.41	0.76
LCS01/07	0.92	20 Min	0.63	0.27	0.17	0.54	0.34
LCS01/07D2	-1.30	20 Min	-1.00	0.27	-0.27	0.41	-0.41
LCS01/08	0.24	30 Min	0.52	0.54	0.28	0.13	0.07
LCS01/09D3	0.48	30 Min	0.33	0.13	0.04	0.22	0.07
LCS01/10C1	0.26	15 Min	0.55	0.14	0.08	0.13	0.07
LCS01/11	0.00	15 Min	0.00	0.00	0.00	0.37	0.00
LCS01/11B1	0.00	15 Min	0.00	0.00	0.00	0.46	0.00
LCS01/11C1	0.00	15 Min	0.00	0.00	0.00	0.14	0.00
LCS01/OUT	0.00	15 Min	0.00	0.00	0.00	1.86	0.00
LCS02/01	0.00	15 Min	0.00	0.00	0.00	0.26	0.00
LCS02/01C3	0.00	15 Min	0.00	0.00	0.00	0.07	0.00
LCS03/01	0.64	15 Min	0.30	0.07	0.02	0.44	0.13
LCS03/01D1	0.03	15 Min	0.30	0.07	0.02	0.08	0.02
LCS03/02	0.84	20 Min	0.39	0.44	0.17	0.39	0.15
LCS04/01	-0.11	45 Min	-0.07	0.09	-0.01	0.37	-0.03
LCS04/01D8	0.09	60 Min	0.04	0.09	0.00	0.56	0.02
D- Guard Street							
LDS01/01	-0.04	15 Min	0.18	0.08	0.01	0.39	0.07
LDS01/02	0.37	15 Min	3.34	0.39	1.30	0.16	0.54
LDS01/02D6	0.00	15 Min	0.00	0.39	0.00	0.00	0.00
LDS01/03	0.86	30 Min	0.21	0.16	0.03	0.75	0.16
LDS01/03D2	-0.67	45 Min	-0.38	0.16	-0.06	0.52	-0.20
LDS01/04	1.16	30 Min	0.11	0.75	0.08	1.01	0.11
LDS01/06	-1.09	25 Min	-0.13	0.81	-0.10	1.01	-0.13
LDS01/07	-1.06	30 Min	-0.11	0.71	-0.08	0.81	-0.09
LDS02/01	0.58	30 Min	0.13	0.41	0.05	0.62	0.08
LDS02/03	-0.70	30 Min	-0.19	0.38	-0.07	0.62	-0.12
LDS02/03D2	0.47	30 Min	-0.34	0.38	-0.13	0.52	-0.17
LDS03/01	0.53	45 Min	0.64	0.22	0.14	0.22	0.14
LDS03/02	0.18	20 Min	0.16	0.22	0.04	0.50	0.08
LDS03/02D7	0.45	25 Min	0.20	0.22	0.04	0.52	0.10
LDS03/04	-0.71	25 Min	-0.34	0.33	-0.11	0.50	-0.17
LDS03/04D1	0.63	30 Min	0.14	0.33	0.04	0.75	0.10
LDS04/01	0.49	15 Min	0.37	0.04	0.01	0.16	0.06
LDS04/02	1.47	15 Min	1.95	0.16	0.31	0.17	0.34
LDS04/03	0.92	15 Min	1.55	0.17	0.27	0.23	0.36
LDS04/03D6	0.59	15 Min	0.96	0.17	0.17	0.34	0.32
LDS04/04	1.21	15 Min	0.49	0.23	0.11	0.68	0.33

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)	U/S Node Depth (m)	U/S Node v.d (m ² /s)	D/S Node Depth (m)	D/S Node v.d (m ² /s)
LDS04/OUT	0.37	60 Min	1.66	0.68	1.13	1.86	3.08
LDS05/01	0.91	15 Min	1.86	0.29	0.53	0.16	0.29
LDS06/01	0.00	15 Min	0.00	0.00	0.00	0.28	0.00
LDS06/02	0.58	15 Min	0.64	0.28	0.18	0.34	0.21
LDS06/03	1.72	15 Min	0.39	0.34	0.13	1.01	0.39
LDS07/02	-0.75	30 Min	-0.26	0.36	-0.09	0.52	-0.14
LDS07/02D1	0.65	30 Min	0.14	0.36	0.05	0.81	0.12
LDS08/02	0.46	30 Min	-0.17	0.41	-0.07	0.56	-0.09
LDS08/02D1	-1.39	25 Min	-0.15	0.41	-0.06	0.71	-0.11
LDS09/01	1.24	20 Min	0.28	0.32	0.09	0.71	0.20
LDS09/01D4	-1.26	20 Min	-0.29	0.32	-0.09	0.68	-0.20

TABLE C17

Peak Flows and Velocities- Underground Network- 50 Year Event

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)
A- Elizabeth Street			
LAU01/02	0.12	90 Min	0.73
LAU01/04	0.24	60 Min	0.84
LAU01/04A	0.22	20 Min	0.69
LAU01/05	0.33	90 Min	1.13
LAU01/05A	0.15	15 Min	0.54
LAU01/06	0.83	15 Min	1.30
LAU01/06A	0.47	15 Min	0.82
LAU01/07	0.87	15 Min	1.35
LAU01/07A	0.04	15 Min	0.39
LAU01/08	0.96	20 Min	1.49
LAU01/08A	0.17	20 Min	0.73
LAU01/09	1.89	20 Min	1.47
LAU01/09A	1.22	20 Min	0.95
LAU01/10	2.37	25 Min	1.23
LAU01/10A	0.21	30 Min	0.64
LAU01/11	3.32	30 Min	0.97
LAU01/11A	0.99	30 Min	1.15
LAU01/12	3.41	30 Min	1.00
LAU01/12A	0.08	90 Min	0.37
LAU03/03	0.11	45 Min	1.01
LAU05/02	0.06	15 Min	0.73
LAU05/04	0.09	25 Min	0.84
LAU05/05	0.06	20 Min	0.57
LAU05/06	0.12	60 Min	1.05
LAU07/02	0.08	60 Min	0.50
LAU07/03	0.15	90 Min	0.90
LAU08/02	0.03	20 Min	0.51
LAU08/03	0.30	20 Min	1.04
LAU12/02	0.10	30 Min	1.37
B- Pier Street			
LBU01/04	0.53	30 Min	1.84
LBU01/04A	0.53	30 Min	0.91
LBU01/05	0.84	15 Min	1.88
LBU01/05A	0.53	15 Min	0.61
LBU01/06	0.84	15 Min	1.89
LBU01/07	0.99	20 Min	1.53
LBU01/07A	0.26	15 Min	0.77
LBU01/08	1.45	15 Min	1.27
LBU01/08A	0.49	15 Min	0.55
LBU01/10	1.49	15 Min	1.31
LBU01/10A	0.08	15 Min	0.49
LBU01/11	1.22	25 Min	0.95
LBU01/11A	-0.35	15 Min	-0.61
LBU01/12	2.38	30 Min	0.94
LBU01/12A	1.40	30 Min	0.86
LBU01/12LF	0.33	30 Min	0.73
LBU04/02	0.10	20 Min	0.88
LBU05/01	0.01	15 Min	0.10
LBU06/01	0.13	45 Min	0.80
LBU1	0.13	25 Min	1.76
C- Pilot Street			
LCU01/04	0.16	60 Min	1.05
LCU01/05	0.46	90 Min	1.61
LCU01/05A	0.34	60 Min	1.05

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)
LCU01/06	0.76	60 Min	0.88
LCU01/06A	0.37	60 Min	1.13
LCU01/07	0.76	60 Min	0.88
LCU01/08	1.12	45 Min	1.28
LCU01/09	1.18	30 Min	1.35
LCU01/09A	0.14	15 Min	0.40
LCU01/11	1.18	30 Min	1.35
LCU03/02	0.07	90 Min	1.01
LCU04/02	0.10	60 Min	1.37
LCU1	0.10	60 Min	0.34
D- Guard Street			
LDU01/02	0.35	20 Min	1.53
LDU01/03	0.30	15 Min	1.86
LDU01/04	0.44	45 Min	0.81
LDU01/04A	0.19	45 Min	0.81
LDU01/05	1.74	60 Min	0.76
LDU01/05A	1.37	60 Min	0.60
LDU01/06	2.02	60 Min	0.89
LDU01/06A	0.29	60 Min	0.90
LDU01/07	2.37	60 Min	1.04
LDU01/07A	0.36	60 Min	1.09
LDU02/02	0.06	15 Min	0.91
LDU02/03	0.40	25 Min	1.40
LDU02/04	0.07	15 Min	1.00
LDU03/01	0.02	25 Min	0.20
LDU03/03	0.14	90 Min	0.49
LDU04/01	0.00	15 Min	0.28
LDU04/02	0.18	15 Min	3.05
LDU04/03	0.21	15 Min	2.56
LDU04/03A	0.04	15 Min	0.65
LDU04/04	0.17	15 Min	1.48
LDU04/04A	0.07	15 Min	0.64
LDU04/05	0.14	15 Min	1.27
LDU04/05A	0.03	60 Min	0.96
LDU05/01	0.13	15 Min	1.51
LDU06/03	0.06	15 Min	0.58
LDU07/01	0.19	90 Min	0.68
LDU08/01	0.45	60 Min	1.56
LDU1	0.06	15 Min	0.57

TABLE C18
Peak Water Levels- Surface Network- 100 Year Event

Node Name	Invert Level (m AHD)	Peak Level (m AHD)	Critical Duration	Maximum Depth (m)
A- Elizabeth Street				
NAS01/01	4.655	4.655	15 Min	0.000
NAS01/02	4.000	4.086	30 Min	0.086
NAS01/03	4.194	4.194	15 Min	0.000
NAS01/04	3.800	4.021	20 Min	0.221
NAS01/05	3.651	3.910	20 Min	0.259
NAS01/06	3.610	3.892	20 Min	0.282
NAS01/07	3.323	3.517	20 Min	0.194
NAS01/08	3.003	3.377	25 Min	0.374
NAS01/09	2.799	3.378	25 Min	0.579
NAS01/10	3.167	3.353	30 Min	0.186
NAS01/11	2.537	3.116	30 Min	0.579
NAS01/12	2.836	3.114	30 Min	0.278
NAS02/01	4.745	4.795	30 Min	0.050
NAS02/02	4.377	4.492	15 Min	0.115
NAS03/01	4.368	4.454	30 Min	0.086
NAS03/02	4.110	4.295	15 Min	0.185
NAS03/03	3.943	4.131	15 Min	0.188
NAS04/01	4.373	4.373	15 Min	0.000
NAS05/01	4.310	4.334	15 Min	0.024
NAS05/02	4.180	4.320	15 Min	0.140
NAS05/03	4.100	4.225	15 Min	0.125
NAS05/04	3.800	3.949	15 Min	0.149
NAS05/05	3.602	3.790	15 Min	0.188
NAS05/06	3.450	3.615	20 Min	0.165
NAS06/01	4.066	4.066	15 Min	0.000
NAS07/01	3.270	3.498	90 Min	0.228
NAS07/02	2.847	3.497	90 Min	0.650
NAS07/03	2.971	3.497	90 Min	0.526
NAS07/04	3.426	3.496	90 Min	0.070
NAS08/01	3.833	3.833	15 Min	0.000
NAS08/02	3.202	3.379	25 Min	0.177
NAS08/03	2.900	3.357	25 Min	0.457
NAS09/01	3.917	3.917	15 Min	0.000
NAS10/01	3.288	3.288	15 Min	0.000
NAS11/01	3.506	3.506	15 Min	0.000
NAS12/01	3.250	3.250	15 Min	0.000
NAS12/02	2.722	3.189	30 Min	0.467
NAS12/03	2.935	3.144	30 Min	0.209
B- Pier Street				
NBS01/03	4.915	4.915	15 Min	0.000
NBS01/04	4.515	4.798	30 Min	0.283
NBS01/04A	4.715	4.786	30 Min	0.071
NBS01/05	4.215	4.472	30 Min	0.257
NBS01/06	4.415	4.458	30 Min	0.043
NBS01/07	3.755	3.892	15 Min	0.137
NBS01/08	3.080	3.358	25 Min	0.278
NBS01/09	3.406	3.546	15 Min	0.140
NBS01/10	2.827	2.980	25 Min	0.153
NBS01/11	2.357	2.877	30 Min	0.520
NBS01/12	2.036	2.874	30 Min	0.838
NBS02/01	4.300	4.356	15 Min	0.056
NBS03/01	4.090	4.090	15 Min	0.000
NBS04/01	3.620	3.620	15 Min	0.000
NBS04/02	3.173	3.479	20 Min	0.306

Node Name	Invert Level (m AHD)	Peak Level (m AHD)	Critical Duration	Maximum Depth (m)
NBS04/03	3.502	3.534	15 Min	0.032
NBS05/01	4.040	4.109	15 Min	0.069
NBS06/01	2.829	3.006	20 Min	0.177
NBS06/02	2.477	2.874	30 Min	0.397
C- Pilot Street				
NCS01/01	6.275	6.539	15 Min	0.264
NCS01/02	5.088	5.358	15 Min	0.269
NCS01/03	4.607	4.806	15 Min	0.199
NCS01/04	3.589	4.004	15 Min	0.415
NCS01/05	3.500	3.950	15 Min	0.450
NCS01/06	3.452	3.733	30 Min	0.281
NCS01/07	3.403	3.711	45 Min	0.308
NCS01/08	3.134	3.711	45 Min	0.577
NCS01/09	3.050	3.200	30 Min	0.150
NCS01/10	3.366	3.518	15 Min	0.152
NCS01/11	3.665	3.665	15 Min	0.000
NCS01/12	0.544	2.400	15 Min	1.856
NCS02/01	5.750	5.750	15 Min	0.000
NCS03/01	5.520	5.595	15 Min	0.075
NCS03/02	3.600	4.066	15 Min	0.466
NCS04/01	3.447	3.563	60 Min	0.116
NCS04/02	3.175	3.566	60 Min	0.391
D- Guard Street				
NDS01/01	5.500	5.601	15 Min	0.101
NDS01/02	5.181	5.598	15 Min	0.417
NDS01/03	3.339	3.520	30 Min	0.181
NDS01/04	2.159	2.970	60 Min	0.811
NDS01/05	1.900	2.970	60 Min	1.070
NDS01/06	2.100	2.971	60 Min	0.871
NDS01/07	2.200	2.971	60 Min	0.771
NDS02/01	3.273	3.721	45 Min	0.448
NDS02/02	3.061	3.720	45 Min	0.659
NDS02/03	3.294	3.708	45 Min	0.414
NDS02/04	3.151	3.702	45 Min	0.551
NDS03/01	2.950	3.182	30 Min	0.232
NDS03/02	2.690	2.975	60 Min	0.285
NDS03/03	2.412	2.974	60 Min	0.562
NDS03/04	2.582	2.972	60 Min	0.390
NDS04/01	10.873	10.913	15 Min	0.040
NDS04/02	9.750	9.915	15 Min	0.165
NDS04/03	6.041	6.225	15 Min	0.184
NDS04/04	3.481	3.726	15 Min	0.245
NDS04/05	2.226	2.970	60 Min	0.744
NDS04/06	0.545	2.400	15 Min	1.855
NDS05/01	10.341	10.643	15 Min	0.302
NDS06/01	4.000	4.000	15 Min	0.000
NDS06/02	3.114	3.403	15 Min	0.289
NDS06/03	2.653	3.008	15 Min	0.355
NDS07/01	2.392	2.974	60 Min	0.582
NDS07/02	2.554	2.971	60 Min	0.417
NDS08/01	2.351	2.972	60 Min	0.621
NDS08/02	2.500	2.972	60 Min	0.472
NDS09/01	2.588	2.971	60 Min	0.383

TABLE C19
Peak Water Levels- Underground Network- 100 Year Event

Node Name	Invert Level (m AHD)	Peak Level m AHD	Critical Duration	Maximum Depth (m)	Surface IL (m AHD)	Surcharge?	Surface Water Lvl (m AHD)
A- Elizabeth Street							
NAU01/02	2.100	4.084	30 Min	1.984	4.000	SURCH	4.086
NAU01/04	1.800	3.969	20 Min	2.169	3.800	SURCH	4.021
NAU01/04A	2.000	4.016	20 Min	2.016	3.800	SURCH	4.021
NAU01/05	1.600	3.905	20 Min	2.305	3.651	SURCH	3.910
NAU01/05A	1.800	3.910	20 Min	2.110	3.651	SURCH	3.910
NAU01/06	1.500	3.729	25 Min	2.229	3.610	SURCH	3.892
NAU01/06A	1.700	3.868	20 Min	2.168	3.610	SURCH	3.892
NAU01/07	1.290	3.537	25 Min	2.247	3.323	SURCH	3.517
NAU01/07A	1.500	3.518	20 Min	2.018	3.323	SURCH	3.517
NAU01/08	1.020	3.391	25 Min	2.371	3.003	SURCH	3.377
NAU01/08A	1.200	3.379	25 Min	2.179	3.003	SURCH	3.377
NAU01/09	0.940	3.214	30 Min	2.274	2.799	SURCH	3.378
NAU01/09A	1.200	3.350	30 Min	2.150	2.799	SURCH	3.378
NAU01/10	0.530	2.845	30 Min	2.315	3.167	-	3.353
NAU01/10A	0.700	3.087	30 Min	2.387	3.167	<150	3.353
NAU01/11	0.190	2.590	30 Min	2.400	2.537	SURCH	3.116
NAU01/11A	0.400	2.983	30 Min	2.583	2.537	SURCH	3.116
NAU01/12	0.100	2.542	30 Min	2.442	2.836	-	3.114
NAU01/12A	0.300	2.578	30 Min	2.278	2.836	-	3.114
NAU03/03	2.500	4.124	15 Min	1.624	3.943	SURCH	4.131
NAU05/02	3.032	4.065	15 Min	1.033	4.180	<150	4.320
NAU05/04	2.690	3.940	15 Min	1.250	3.800	SURCH	3.949
NAU05/05	2.672	3.643	25 Min	0.971	3.602	SURCH	3.790
NAU05/06	2.680	3.617	20 Min	0.937	3.450	SURCH	3.615
NAU06/01	1.529	3.004	25 Min	1.475	2.829	SURCH	3.006
NAU07/02	2.165	3.497	90 Min	1.332	2.847	SURCH	3.497
NAU07/03	2.021	3.402	90 Min	1.381	2.971	SURCH	3.497
NAU08/02	2.562	3.356	25 Min	0.794	3.202	SURCH	3.379
NAU08/03	2.291	3.335	25 Min	1.044	2.900	SURCH	3.357
NAU12/02	2.113	3.184	30 Min	1.071	2.722	SURCH	3.189
NAUOUT1	0.000	2.400	15 Min	2.400			
NAUOUT2	0.744	2.400	15 Min	1.656			
B- Pier Street							
NBU01/04	2.000	4.722	30 Min	2.722	4.515	SURCH	4.798
NBU01/04A	2.200	4.791	30 Min	2.591	4.515	SURCH	4.798
NBU01/05	1.800	4.393	30 Min	2.593	4.215	SURCH	4.472
NBU01/05A	2.200	4.454	30 Min	2.254	4.215	SURCH	4.472
NBU01/06	1.600	3.938	25 Min	2.338			
NBU01/07	1.500	3.720	25 Min	2.220	3.755	<150	3.892
NBU01/07A	2.200	3.864	25 Min	1.664	3.755	SURCH	3.892
NBU01/08	1.000	3.272	25 Min	2.272	3.080	SURCH	3.358
NBU01/08A	1.300	3.340	25 Min	2.040	3.080	SURCH	3.358
NBU01/10	0.700	3.016	30 Min	2.316	2.827	SURCH	2.980
NBU01/10A	1.000	2.986	25 Min	1.986	2.827	SURCH	2.980
NBU01/11	0.500	2.888	30 Min	2.388	2.357	SURCH	2.877
NBU01/11A	0.800	2.882	30 Min	2.082	2.357	SURCH	2.877
NBU01/12	0.190	2.564	30 Min	2.374	2.036	SURCH	2.874
NBU01/12A	0.250	2.751	30 Min	2.501	2.036	SURCH	2.874
NBU04/02	1.853	2.989	20 Min	1.136	3.173	-	3.479
NBU05/01	2.300	2.994	20 Min	0.694	4.040	-	4.109
NBU1	0.378	2.943	25 Min	2.565			
NBUOUT1	0.350	2.400	15 Min	2.050			
NBUOUT2	1.100	2.400	15 Min	1.300			
NBUOUT2A	0.100	2.400	15 Min	2.300			

Node Name	Invert Level (m AHD)	Peak Level m AHD	Critical Duration	Maximum Depth (m)	Surface IL (m AHD)	Surcharge?	Surface Water Lvl (m AHD)
C- Pilot Street							
NCU01/04	2.979	3.947	20 Min	0.968	3.589	SURCH	4.004
NCU01/05	2.527	3.839	20 Min	1.312	3.500	SURCH	3.950
NCU01/05A	2.554	3.938	15 Min	1.384	3.500	SURCH	3.950
NCU01/06	1.583	3.560	30 Min	1.977	3.452	SURCH	3.733
NCU01/06A	1.613	3.720	30 Min	2.107	3.452	SURCH	3.733
NCU01/07	1.310	3.486	30 Min	2.176	3.403	SURCH	3.711
NCU01/08	1.270	3.405	30 Min	2.135	3.134	SURCH	3.711
NCU01/09	0.850	3.183	30 Min	2.333	3.050	SURCH	3.200
NCU01/09A	2.230	3.196	30 Min	0.966	3.050	SURCH	3.200
NCU01/11	0.270	2.877	30 Min	2.607			
NCU03/02	2.970	4.013	20 Min	1.043	3.600	SURCH	4.066
NCU04/02	2.370	3.561	60 Min	1.191	3.175	SURCH	3.566
NCU1	1.771	2.423	45 Min	0.652			
NCUOUT	0.000	2.400	15 Min	2.400			
NCUOUT2	0.000	2.400	15 Min	2.400			
D- Guard Street							
NDU01/02	4.081	5.454	25 Min	1.373	5.181	SURCH	5.598
NDU01/03	1.800	3.691	45 Min	1.891	3.339	SURCH	3.520
NDU01/04	0.500	2.817	60 Min	2.317	2.159	SURCH	2.970
NDU01/04A	0.551	2.959	60 Min	2.408	2.159	SURCH	2.970
NDU01/05	0.260	2.765	60 Min	2.505	1.900	SURCH	2.970
NDU01/05A	0.300	2.843	60 Min	2.543	1.900	SURCH	2.970
NDU01/06	0.211	2.705	60 Min	2.494	2.100	SURCH	2.971
NDU01/06A	0.291	2.929	60 Min	2.638	2.100	SURCH	2.971
NDU01/07	0.152	2.616	60 Min	2.464	2.200	SURCH	2.971
NDU01/07A	0.178	2.933	60 Min	2.755	2.200	SURCH	2.971
NDU02/02	2.387	3.721	45 Min	1.334	3.061	SURCH	3.720
NDU02/03	1.954	3.513	45 Min	1.559	3.294	SURCH	3.708
NDU02/04	2.511	3.703	45 Min	1.192	3.151	SURCH	3.702
NDU03/01	2.241	3.182	30 Min	0.941	2.950	SURCH	3.182
NDU03/03	1.602	3.005	60 Min	1.403	2.412	SURCH	2.974
NDU04/01	10.173	10.194	15 Min	0.020	10.873	-	10.913
NDU04/02	8.950	9.123	15 Min	0.173	9.750	-	9.915
NDU04/03	4.533	4.865	15 Min	0.332	6.041	-	6.225
NDU04/03A	4.733	4.942	15 Min	0.209	6.041	-	6.225
NDU04/04	2.821	4.024	15 Min	1.203	3.481	SURCH	3.726
NDU04/04A	2.921	3.978	15 Min	1.057	3.481	SURCH	3.726
NDU04/05	1.845	3.143	15 Min	1.298	2.226	SURCH	2.970
NDU04/05A	1.890	2.978	60 Min	1.088	2.226	SURCH	2.970
NDU05/01	9.571	9.900	15 Min	0.329	10.341	-	10.643
NDU06/03	2.054	2.931	60 Min	0.877	2.653	SURCH	3.008
NDU07/01	1.222	2.980	60 Min	1.758	2.392	SURCH	2.974
NDU08/01	1.160	2.947	60 Min	1.787	2.351	SURCH	2.972
NDU1	1.270	2.799	60 Min	1.529			
NDUOUT1	0.900	2.400	15 Min	1.500			
NDUOUT2	0.050	2.400	15 Min	2.350			
NDUOUT3	0.900	2.400	15 Min	1.500			

TABLE C20

Peak Flows and Velocities- Surface Network- 100 Year Event

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)	U/S Node Depth (m)	U/S Node v.d (m ² /s)	D/S Node Depth (m)	D/S Node v.d (m ² /s)
A- Elizabeth Street							
LAS01/01	0.00	15 Min	0.00	0.00	0.00	0.09	0.00
LAS01/03	0.00	15 Min	0.00	0.00	0.00	0.22	0.00
LAS01/03A1	0.00	15 Min	0.00	0.00	0.00	0.09	0.00
LAS01/04	0.39	20 Min	0.62	0.22	0.14	0.26	0.16
LAS01/05	0.44	20 Min	0.23	0.26	0.06	0.28	0.07
LAS01/06	0.96	20 Min	0.81	0.28	0.23	0.19	0.16
LAS01/07	1.09	20 Min	0.49	0.19	0.09	0.37	0.18
LAS01/08	2.43	20 Min	0.86	0.37	0.32	0.58	0.50
LAS01/10	1.05	30 Min	0.38	0.19	0.07	0.58	0.22
LAS01/10A1	-1.27	25 Min	-0.29	0.19	-0.05	0.58	-0.17
LAS01/10A8	-0.14	25 Min	0.23	0.19	0.04	0.46	0.11
LAS01/12A1	0.99	30 Min	0.90	0.28	0.25	0.58	0.52
LAS02/01	0.07	30 Min	0.30	0.05	0.01	0.11	0.03
LAS02/01B1	-0.08	30 Min	-0.09	0.05	0.00	0.28	-0.02
LAS02/02	0.36	15 Min	0.62	0.11	0.07	0.22	0.14
LAS03/01	0.14	30 Min	0.34	0.09	0.03	0.18	0.06
LAS03/01B1	-0.17	30 Min	-0.19	0.09	-0.02	0.26	-0.05
LAS03/02	0.66	15 Min	0.67	0.18	0.12	0.19	0.13
LAS03/03	0.90	15 Min	0.86	0.19	0.16	0.28	0.24
LAS04/01A3	0.00	15 Min	0.00	0.00	0.00	0.18	0.00
LAS04/01A5	0.00	15 Min	0.00	0.00	0.00	0.13	0.00
LAS05/01	0.01	15 Min	0.04	0.02	0.00	0.14	0.01
LAS05/01B2	-0.01	15 Min	-0.11	0.02	0.00	0.06	-0.01
LAS05/02	0.19	15 Min	0.48	0.14	0.07	0.13	0.06
LAS05/03	0.31	15 Min	0.43	0.13	0.05	0.15	0.06
LAS05/04	0.54	15 Min	0.49	0.15	0.07	0.19	0.09
LAS05/05	0.67	15 Min	0.85	0.19	0.16	0.16	0.14
LAS05/06	0.81	20 Min	0.65	0.16	0.11	0.37	0.24
LAS06/01A5	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LAS06/01A7	0.00	15 Min	0.00	0.00	0.00	0.65	0.00
LAS07/01	0.23	15 Min	0.35	0.23	0.08	0.65	0.23
LAS07/03A2	0.24	15 Min	0.44	0.53	0.23	0.65	0.28
LAS07/04	0.12	90 Min	0.51	0.07	0.04	0.19	0.09
LAS07/04A7	-0.14	90 Min	-0.05	0.07	0.00	0.53	-0.02
LAS08/01	0.00	15 Min	0.00	0.00	0.00	0.18	0.00
LAS08/02	0.31	20 Min	0.76	0.18	0.14	0.46	0.35
LAS09/01	0.00	15 Min	0.00	0.00	0.00	0.18	0.00
LAS10/01	0.00	15 Min	0.00	0.00	0.00	0.28	0.00
LAS10/01B6	0.00	15 Min	0.00	0.00	0.00	0.18	0.00
LAS11/01	0.00	15 Min	0.00	0.00	0.00	0.53	0.00
LAS11/01B6	0.00	15 Min	0.00	0.00	0.00	0.18	0.00
LAS12/01	0.00	15 Min	0.00	0.00	0.00	0.47	0.00
LAS12/03	-0.61	30 Min	-0.26	0.21	-0.05	0.47	-0.12
LAS12/03A1	0.60	30 Min	0.60	0.21	0.13	0.28	0.17
B- Pier Street							
LBS01/03	0.00	15 Min	0.00	0.00	0.00	0.28	0.00
LBS01/04	0.17	30 Min	0.24	0.07	0.02	0.26	0.06
LBS01/04B1	-0.18	30 Min	-0.17	0.07	-0.01	0.28	-0.05
LBS01/06	0.11	30 Min	0.38	0.04	0.02	0.14	0.05
LBS01/06B1	-0.11	30 Min	-0.13	0.04	-0.01	0.26	-0.03
LBS01/07	0.31	15 Min	0.76	0.14	0.10	0.28	0.21
LBS01/09	0.15	15 Min	0.83	0.14	0.12	0.15	0.13
LBS01/09B1	0.19	15 Min	0.70	0.14	0.10	0.28	0.19
LBS01/10	0.25	25 Min	0.52	0.15	0.08	0.52	0.27

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)	U/S Node Depth (m)	U/S Node v.d (m ² /s)	D/S Node Depth (m)	D/S Node v.d (m ² /s)
LBS01/11	0.65	20 Min	0.13	0.52	0.07	0.84	0.11
LBS02/01	0.02	15 Min	0.23	0.06	0.01	0.14	0.03
LBS02/01B5	0.08	15 Min	0.41	0.06	0.02	0.07	0.03
LBS03/01	0.00	15 Min	0.00	0.00	0.00	0.14	0.00
LBS03/01C1	0.00	15 Min	0.00	0.00	0.00	0.31	0.00
LBS04/01	0.00	15 Min	0.00	0.00	0.00	0.31	0.00
LBS04/01A7	0.00	15 Min	0.00	0.00	0.00	0.23	0.00
LBS04/02B1	0.81	20 Min	0.83	0.31	0.25	0.84	0.70
LBS04/03	-0.04	15 Min	-0.13	0.03	0.00	0.14	-0.02
LBS05/01	0.24	15 Min	0.40	0.07	0.03	0.31	0.12
LBS06/01	0.23	20 Min	0.35	0.18	0.06	0.40	0.14
LBS06/02	0.54	25 Min	0.16	0.40	0.06	0.84	0.14
C- Pilot Street							
LCS01/01	0.16	15 Min	0.55	0.26	0.15	0.27	0.15
LCS01/02	1.40	15 Min	0.91	0.27	0.25	0.20	0.18
LCS01/03	1.72	15 Min	0.84	0.20	0.17	0.41	0.35
LCS01/04	2.88	15 Min	0.70	0.41	0.29	0.45	0.32
LCS01/05	2.79	15 Min	0.84	0.45	0.38	0.28	0.24
LCS01/06	0.00	15 Min	0.00	0.28	0.00	0.31	0.00
LCS01/06D2	2.60	15 Min	1.88	0.28	0.53	0.45	0.84
LCS01/07	1.00	15 Min	0.61	0.31	0.19	0.58	0.35
LCS01/07D2	-1.56	20 Min	-1.05	0.31	-0.32	0.45	-0.47
LCS01/08	0.29	30 Min	0.56	0.58	0.32	0.15	0.08
LCS01/09D3	0.59	30 Min	0.36	0.15	0.05	0.23	0.08
LCS01/10C1	0.30	15 Min	0.57	0.15	0.09	0.15	0.09
LCS01/11	0.00	15 Min	0.00	0.00	0.00	0.39	0.00
LCS01/11B1	0.00	15 Min	0.00	0.00	0.00	0.52	0.00
LCS01/11C1	0.00	15 Min	0.00	0.00	0.00	0.15	0.00
LCS01/OUT	0.00	15 Min	0.00	0.00	0.00	1.86	0.00
LCS02/01	0.00	15 Min	0.00	0.00	0.00	0.27	0.00
LCS02/01C3	0.00	15 Min	0.00	0.00	0.00	0.07	0.00
LCS03/01	0.75	15 Min	0.32	0.07	0.02	0.47	0.15
LCS03/01D1	0.03	15 Min	0.30	0.07	0.02	0.10	0.03
LCS03/02	1.01	15 Min	0.42	0.47	0.19	0.41	0.17
LCS04/01	-0.16	45 Min	-0.08	0.12	-0.01	0.39	-0.03
LCS04/01D8	0.13	60 Min	0.04	0.12	0.01	0.62	0.03
D- Guard Street							
LDS01/01	-0.09	15 Min	0.18	0.10	0.02	0.42	0.08
LDS01/02	0.45	15 Min	3.51	0.42	1.46	0.18	0.64
LDS01/02D6	0.00	15 Min	0.00	0.42	0.00	0.00	0.00
LDS01/03	1.13	30 Min	0.23	0.18	0.04	0.81	0.18
LDS01/03D2	-0.89	45 Min	-0.42	0.18	-0.08	0.55	-0.23
LDS01/04	1.56	30 Min	0.12	0.81	0.10	1.07	0.13
LDS01/06	-1.36	20 Min	-0.13	0.87	-0.11	1.07	-0.14
LDS01/07	-1.37	20 Min	-0.13	0.77	-0.10	0.87	-0.12
LDS02/01	0.73	30 Min	-0.13	0.45	-0.06	0.66	-0.09
LDS02/03	-0.89	30 Min	-0.21	0.41	-0.09	0.66	-0.14
LDS02/03D2	0.65	30 Min	-0.34	0.41	-0.14	0.55	-0.19
LDS03/01	0.65	30 Min	0.67	0.23	0.15	0.29	0.19
LDS03/02	0.22	20 Min	0.14	0.29	0.04	0.56	0.08
LDS03/02D7	0.52	20 Min	0.21	0.29	0.06	0.58	0.12
LDS03/04	-0.79	20 Min	-0.36	0.39	-0.14	0.56	-0.20
LDS03/04D1	0.72	25 Min	0.14	0.39	0.05	0.81	0.11
LDS04/01	0.55	15 Min	0.39	0.04	0.02	0.17	0.06
LDS04/02	1.70	15 Min	1.96	0.17	0.32	0.18	0.36
LDS04/03	1.06	15 Min	1.57	0.18	0.29	0.25	0.39
LDS04/03D6	0.69	15 Min	0.98	0.18	0.18	0.35	0.35
LDS04/04	1.41	15 Min	0.52	0.25	0.13	0.74	0.39

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)	U/S Node Depth (m)	U/S Node v.d (m ² /s)	D/S Node Depth (m)	D/S Node v.d (m ² /s)
LDS04/OUT	0.75	60 Min	1.88	0.74	1.40	1.86	3.50
LDS05/01	1.07	15 Min	1.92	0.30	0.58	0.17	0.32
LDS06/01	0.00	15 Min	0.00	0.00	0.00	0.29	0.00
LDS06/02	0.67	15 Min	0.66	0.29	0.19	0.35	0.23
LDS06/03	1.98	15 Min	0.41	0.35	0.15	1.07	0.44
LDS07/02	-0.86	25 Min	-0.28	0.42	-0.12	0.58	-0.16
LDS07/02D1	0.71	45 Min	0.16	0.42	0.07	0.87	0.14
LDS08/02	0.58	20 Min	-0.17	0.47	-0.08	0.62	-0.11
LDS08/02D1	-1.76	20 Min	-0.18	0.47	-0.08	0.77	-0.14
LDS09/01	1.45	20 Min	0.30	0.38	0.12	0.77	0.23
LDS09/01D4	-1.46	20 Min	-0.32	0.38	-0.12	0.74	-0.24

TABLE C21

Peak Flows and Velocities- Underground Network- 100 Year Event

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)
A- Elizabeth Street			
LAU01/02	0.12	30 Min	0.75
LAU01/04	0.24	90 Min	0.84
LAU01/04A	0.22	25 Min	0.69
LAU01/05	0.32	90 Min	1.10
LAU01/05A	0.14	60 Min	0.50
LAU01/06	0.83	15 Min	1.30
LAU01/06A	0.47	15 Min	0.82
LAU01/07	0.87	15 Min	1.35
LAU01/07A	0.05	15 Min	0.40
LAU01/08	0.96	25 Min	1.49
LAU01/08A	0.17	15 Min	0.73
LAU01/09	1.90	20 Min	1.48
LAU01/09A	1.22	15 Min	0.95
LAU01/10	2.47	30 Min	1.28
LAU01/10A	0.33	30 Min	1.00
LAU01/11	3.58	30 Min	1.05
LAU01/11A	1.16	30 Min	1.34
LAU01/12	3.66	30 Min	1.07
LAU01/12A	0.09	30 Min	0.39
LAU03/03	0.11	60 Min	1.01
LAU05/02	0.06	15 Min	0.75
LAU05/04	0.09	45 Min	0.84
LAU05/05	0.07	25 Min	0.58
LAU05/06	0.12	60 Min	1.04
LAU07/02	0.08	60 Min	0.50
LAU07/03	0.15	90 Min	0.92
LAU08/02	0.03	15 Min	0.51
LAU08/03	0.30	15 Min	1.04
LAU12/02	0.10	30 Min	1.39
B- Pier Street			
LBU01/04	0.53	60 Min	1.84
LBU01/04A	0.53	60 Min	0.91
LBU01/05	0.86	25 Min	1.91
LBU01/05A	0.54	25 Min	0.62
LBU01/06	0.86	25 Min	1.92
LBU01/07	0.99	15 Min	1.54
LBU01/07A	0.27	15 Min	0.79
LBU01/08	1.49	20 Min	1.31
LBU01/08A	0.51	20 Min	0.57
LBU01/10	1.51	15 Min	1.32
LBU01/10A	0.08	15 Min	0.50
LBU01/11	1.27	30 Min	0.99
LBU01/11A	-0.36	15 Min	-0.62
LBU01/12	2.52	30 Min	0.99
LBU01/12A	1.50	30 Min	0.92
LBU01/12LF	0.35	30 Min	0.77
LBU04/02	0.10	20 Min	0.91
LBU05/01	0.01	15 Min	0.13
LBU06/01	0.13	25 Min	0.81
LBU1	0.13	25 Min	1.78
C- Pilot Street			
LCU01/04	0.16	90 Min	1.05
LCU01/05	0.45	90 Min	1.60
LCU01/05A	0.34	60 Min	1.05

Link Name	Maximum Flow (m ³ /s)	Critical Duration	Maximum Velocity (m/s)
LCU01/06	0.76	60 Min	0.88
LCU01/06A	0.37	60 Min	1.13
LCU01/07	0.76	60 Min	0.88
LCU01/08	1.13	45 Min	1.30
LCU01/09	1.19	30 Min	1.36
LCU01/09A	0.15	15 Min	0.43
LCU01/11	1.19	30 Min	1.36
LCU03/02	0.07	90 Min	1.01
LCU04/02	0.10	60 Min	1.39
LCU1	0.10	60 Min	0.34
D- Guard Street			
LDU01/02	0.35	20 Min	1.54
LDU01/03	0.30	20 Min	1.86
LDU01/04	0.45	60 Min	0.83
LDU01/04A	0.20	60 Min	0.87
LDU01/05	1.84	60 Min	0.81
LDU01/05A	1.46	60 Min	0.64
LDU01/06	2.14	60 Min	0.94
LDU01/06A	0.31	60 Min	0.95
LDU01/07	2.51	60 Min	1.10
LDU01/07A	0.38	60 Min	1.16
LDU02/02	0.06	15 Min	0.91
LDU02/03	0.40	25 Min	1.41
LDU02/04	0.07	15 Min	1.01
LDU03/01	0.02	30 Min	0.20
LDU03/03	0.14	90 Min	0.48
LDU04/01	0.00	15 Min	0.26
LDU04/02	0.18	15 Min	3.06
LDU04/03	0.22	15 Min	2.59
LDU04/03A	0.04	15 Min	0.65
LDU04/04	0.17	15 Min	1.50
LDU04/04A	0.07	15 Min	0.65
LDU04/05	0.15	15 Min	1.29
LDU04/05A	0.03	60 Min	1.02
LDU05/01	0.14	15 Min	1.50
LDU06/03	0.06	15 Min	0.58
LDU07/01	0.19	90 Min	0.68
LDU08/01	0.47	60 Min	1.65
LDU1	0.06	15 Min	0.57

Estimated Construction Costs**List of Tables**

Table D1 Typical Construction Costs

TABLE D1
Typical Construction Costs

Item	Unit	Cost (\$)
Pipes		
300 dia RCP	/m	90
375 dia RCP	/m	110
450 dia RCP	/m	135
525 dia RCP	/m	155
600 dia RCP	/m	180
675 dia RCP	/m	215
750 dia RCP	/m	250
825 dia RCP	/m	270
900 dia RCP	/m	320
1050 dia RCP	/m	405
1200 dia RCP	/m	520
1350 dia RCP	/m	635
Manholes		
1,050 dia manhole	item	1800
Multi pipe manhole	item	10000
Gully Pits		
1 Bay SEP	item	1500
2 Bay SEP	item	1600
3 Bay SEP	item	1900
4 Bay SEP	item	2200
Roadworks		
Road replacement	/m ²	65

Note Figures for 2003